

# **SKRIPSI**

## **STUDI ALTERNATIF PONDASI TIANG BOR (STRAUSS) PEMBANGUNAN RUSUNAWA UNIVERSITAS NEGERI MALANG**



**Disusun Oleh :**

**BAYU ENDIK PRASETYO      05.21.074**

**JURUSAN TEKNIK SIPIL S-1  
FAKULTAS TEKNIK SIPIL DAN PERENCANAAN  
INSTITUT TEKNOLOGI NASIONAL MALANG  
2011**

REPORT

ANALYSIS OF THE DATA FROM THE SURVEY OF THE  
EFFECTS OF THE APPLICATION OF THE  
LAW

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**LEMBAR PERSETUJUAN  
SKRIPSI**

**STUDI ALTERNATIF PONDASI TIANG BOR (STRAUSS)  
PEMBANGUNAN RUSUNAWA UNIVERSITAS NEGERI MALANG**

*Diajukan Sebagai Salah Satu Syarat Memperoleh Gelar Sarjana  
Teknik Sipil (S-1) Institut Teknologi Nasional Malang*

**Disusun Oleh :  
BAYU ENDIK PRASETYO**

**05. 21. 074**

**Menyetujui:**

**Dosen Pembimbing I**

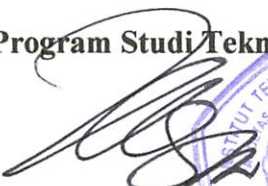
  
**(Ir. Eding Iskak Imananto, MT.)**


**Dosen Pembimbing II**

  
**(Ir.A. Agus Santosa, MT.)**

**Mengetahui,**

**Ketua Program Studi Teknik Sipil S-1**

  
**(Ir. H. Hirijanto, MT)**



**PROGRAM STUDI TEKNIK SIPIL  
FAKULTAS TEKNIK SIPIL DAN PERENCANAAN  
INSTITUT TEKNOLOGI NASIONAL MALANG**

**2011**

## LEMBAR PENGESAHAN

### STUDI ALTERNATIF PONDASI TIANG BOR (STRAUSS) PEMBANGUNAN RUSUNAWA UNIVERSITAS NEGERI MALANG

#### SKRIPSI

*Dipertahankan Dihadapan Majelis Penguji Sidang Skripsi*

*Jenjang Strata Satu (S-1)*

*Pada hari : Kamis*

*Tanggal : 25 Agustus 2011*

*Dan Diterima Untuk Memenuhi Salah Satu Persyaratan*

*Guna Memperoleh Gelar Sarjana Teknik*

**Disusun Oleh :**

**BAYU ENDIK PRASETYO**

**05. 21. 074**

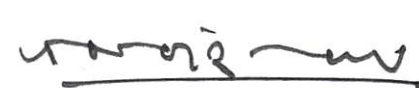
**Disahkan Oleh:**

**Ketua**  
  
**( Ir. H. Hirijanto, MT. )**

**Sekretaris**  
  
**( Lila Ayu Ratna Winanda, ST, MT. )**

**Anggota Penguji :**

**Dosen Penguji I**  
  
**( Ir. Bambang Wedyantadji, MT. )**

**Dosen Penguji II**  
  
**( Ir. H. Sudirman Indra, MSc. )**

**PROGRAM STUDI TEKNIK SIPIL  
FAKULTAS TEKNIK SIPIL DAN PERENCANAAN  
INSTITUT TEKNOLOGI NASIONAL MALANG**

**2011**

## KATA PENGANTAR

Puji syukur penyusun panjatkan kehadirat Allah SWT, karena berkat-Nya penyusun dapat menyelesaikan **Laporan skripsi yang berjudul ” Studi Alternatif Pondasi Tiang Bor ( Strauss ) Pada Pembangunan Rusunawa Di Universitas Negeri Malang ”** yang merupakan salah satu syarat untuk memperoleh gelar Sarjana Teknik Sipil S-1 Institut Teknologi Nasional Malang. Dalam kesempatan ini penyusun juga ingin menyampaikan rasa terima kasih kepada berbagai pihak yang telah membantu dalam menyelesaikan laporan ini, diantaranya :

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Penyusun berharap laporan ini dapat menambah wawasan bagi penyusun, khususnya untuk pembaca pada umumnya. Dan penyusun menyadari bahwa laporan skripsi ini masih kurang sempurna, oleh karena itu penyusun sangat mengharapkan kritik dan saran yang bersifat membangun untuk perbaikan di masa mendatang.

**Malang, 2011**

**Penyusun**

## ABSTRAKSI

Bayu Endik Prasetyo, **Studi Alternatif Pondasi Tiang Bor (Strauss) Pembangunan Rusunawa Universitas Negeri Malang**. Pembimbing I : Ir. Eding Iskak Imananto, MT. Pembimbing II : Ir. Agus Santosa, MT.

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Kata Kunci : Daya dukung, Biaya pengerjaan pondasi

Perkembangan ilmu pengetahuan dan teknologi dalam bidang analisa dan perencanaan konstruksi semakin meningkat terutama pada bagian pondasi. Hal ini mendorong para perencana, pelaksana dan pengawas pembangunan untuk menindak lanjuti seberapa jauh konsep teknologi itu diterapkan dalam pembangunan pondasi.

Pondasi berfungsi untuk memikul dan menahan beban yang bekerja di atasnya yaitu beban konstruksi di atasnya ke lapisan tanah keras. Dalam perencanaan pondasi tiang harus dilakukan dengan teliti dan sebaik mungkin karena setiap pondasi harus mampu mendukung beban sampai batas keamanan yang ditentukan termasuk memikul beban maksimum yang mungkin terjadi.

Tujuan dari penulis adalah memberikan alternatif pondasi tiang bor, yang sebelumnya pondasi yang digunakan adalah pondasi sumuran. Disamping itu penulis juga membandingkan daya dukung pondasi tiang bor (strauss) dan pondasi sumuran dari beberapa beban yang bekerja serta biaya pengerjaan pondasi tersebut. Data yang digunakan adalah data sondir, nilai sondir yang digunakan adalah nilai sondir pada titik S2 : 150 kg / cm<sup>2</sup> pada kedalaman 6,40 m.

Hasil dari perhitungan analisa struktur diperoleh beban aksi tipe I = 306000 kg, tipe II = 283000 kg dan tipe III = 57000 kg, daya dukung antara pondasi tiang bor (strauss) dan pondasi sumuran (caisson) mampu menahan beban struktur dan dari beberapa tipe beban yang bekerja diperoleh hasil daya dukung sebagai berikut : a. pondasi tiang bor : tipe I dan II = 321539 kg, tipe III = 124794 kg; b. pondasi sumuran : tipe I, II, dan III = 392500 kg. Sedangkan untuk biaya pengerjaan pondasi tiang bor : Rp 5.484.062.761 dan pondasi sumuran : Rp 2.049.522.395, sehingga pondasi yang digunakan adalah pondasi sumuran karena lebih murah, aman dan efisien.

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# **BAB I**

## **PENDAHULUAN**

### **1.1 Latar Belakang**

Perkembangan ilmu pengetahuan dan teknologi dalam bidang analisa dan perencanaan konstruksi semakin meningkat terutama pada bagian pondasi. Hal ini mendorong para perencana, pelaksana dan pengawas pembangunan untuk menindak lanjuti seberapa jauh konsep teknologi itu diterapkan dalam pembangunan pondasi.

Pondasi merupakan bagian terpenting dalam sebuah konstruksi bangunan karena bertugas meneruskan beban bangunan di atas ke dasar tanah yang cukup kuat untuk mendukungnya. Untuk itu pondasi harus diperhitungkan agar dapat mendukung kestabilan bangunan, dengan cara memperhatikan kekuatan pondasi itu sendiri.

Apabila tanah mampu memikul beban yang bekerja di atasnya maka pondasi dapat dibangun diatas tanah tersebut, apabila tidak maka diperlukan konstruksi seperti pondasi tiang bor (strauss) untuk meneruskan beban ke lapisan tanah yang mampu memikul beban itu sepenuhnya.

Secara umum ada dua jenis pondasi yaitu pondasi langsung dan pondasi tidak langsung. Pondasi langsung seperti pondasi batu kali, pondasi plat, pondasi menerus, karena lapisan tanah keras yang tidak begitu dalam dari permukaan tanah. Sedangkan pondasi tidak langsung adalah yang tidak langsung diletakkan pada lapisan tanah keras yang

letaknya sangat dalam tetapi dibantu dengan struktur pembantu. Pemilihan pondasi tergantung dari kondisi tanah dimana tanah merupakan dasar pondasi pendukung suatu bangunan yang dibedakan atas tanah lunak dan tanah keras. Dimana untuk menentukan kedalaman pondasi tiang bor dicoba dahulu dengan menggunakan  $q_c$  minimum =  $40 \text{ kg/cm}^2$  begitu seterusnya sampai mendapatkan kedalaman pondasi yang sesuai dengan beban yang ditopangnya sehingga lebih aman dan ekonomis. Pada perencanaan awal pondasi yang di gunakan adalah pondasi sumuran, kemudian penyusun merencanakan jenis pondasi lain yang diharapkan akan menghasilkan daya dukung yang lebih besar yaitu dengan menggunakan pondasi tiang bor. Keuntungan yang di dapat dari pondasi tiang bor adalah kedalaman dan diameter dapat divariasikan sesuai dengan beban struktur yang akan ditopangnya selain itu juga pembangunan pondasi ini dalam pengerjaannya tidak mengganggu lingkungan sekitarnya. Dalam hal ini penyusun akan mengangkat

**pembahasan mengenai STUDI PERENCANAAN PONDASI TIANG BOR (STRAUSS) PADA PAMBANGUNAN RUSUNAWA DI UNIVERSITAS NEGERI MALANG.**

## **1.2 Identifikasi Masalah**

Pondasi merupakan bagian struktur bangunan yang sangat penting dimana pondasi akan menerima beban dari bangunan untuk diteruskan ke lapisan tanah yang mempunyai daya dukung yang cukup kuat dan penurunan yang terjadi sekecil mungkin. Sehingga dalam merencanakan

pondasi harus didukung dengan data - data yang dapat dipertanggung jawabkan secara teknis, agar hasil yang didapatkan sesuai dengan yang diinginkan.

Pembangunan Gedung Rusunawa di Universitas Negeri Malang yang terdiri dari 4 lantai. Dengan menggunakan data tanah dengan test SONDIR didapat data sebagai berikut :

**Tabel 1.1 : Pelawanan Konus dan JHP Untuk Berbagai Macam Kedalaman**

<b>Titik Uji</b>	<b>Kedalaman Tanah Keras (m)</b>	<b>Perlawanan Konus (Kg/cm<sup>2</sup>)</b>	<b>JHP (Kg/cm)</b>
S1	6.00	150	710.00
S2	6.40	150	1080.00
S3	6.60	155	880.00

\*Data lengkap dapat dilihat pada lampiran

Pada pelaksanaan di lapangan pondasi yang digunakan adalah pondasi sumuran (Caison), tetapi penyusun akan mencoba menggunakan pondasi Strauss untuk mendapatkan hasil yang optimum dari data analisa.

### **1.3 Rumusan Masalah**

Berdasarkan dari uraian diatas maka dapat dirumuskan masalah yang dapat dibahas yaitu:

1. Bagaimana menentukan pondasi yang tepat ditinjau dari data yang diperoleh dari penyelidikan maupun dari lokasi di lapangan.



2. Bagaimana merencanakan pondasi strauss yang aman, efisien dan ekonomis.
3. Bagaimana hasil volume dan biaya antara pondasi strauss dan pondasi sumuran.

#### **1.4 Maksud Dan Tujuan**

Maksud penyusun disini adalah untuk memberikan alternatif perencanaan pondasi strauss, sedangkan tujuannya adalah untuk mendapatkan struktur pondasi yang mempunyai daya dukung yang cukup dan perbedaan biaya yang dikeluarkan antara pondasi strauss dan sumuran.

#### **1.5 Lingkup Pembahasan**

Dengan memperhatikan maksud dan tujuan maka ruang lingkup pembahasan tugas akhir ini adalah meliputi:

1. Analisa pembebanan dan analisa statika.

Sebagai pedoman perhitungan didasarkan pada:

- a. Peraturan Pembebanan Indonesia untuk gedung ( PPIUG )  
1983
- b. SNI 30-2847-2002 (Tata Cara Perhitungan Beton Bertulang)
- c. SNI 29-1726-2002 (Tata Cara Perencanaan Struktur Baja Untuk Bangunan)
- d. Analisa statika dengan menggunakan program bantu komputer (STAAD PRO).

2. Perhitungan daya dukung pondasi strauss.
3. Perhitungan penulangan pondasi strauss.
4. Perhitungan daya dukung pondasi sumuran.
5. Perhitungan penulangan pondasi sumuran.
6. Perbandingan volume dan biaya pondasi strauss dan sumuran.

## **BAB II**

### **LANDASAN TEORI**

#### **2.1 Pengertian Pondasi Secara Umum**

Pondasi adalah suatu konstruksi bagian bawah struktur yang fungsinya untuk meneruskan beban bagian atas bangunan ke lapisan tanah yang mempunyai daya dukung yang cukup, tanpa mengakibatkan penurunan (settlement) tanah yang berlebihan.

Dalam merencanakan suatu pondasi adapun faktor-faktor yang harus dipertimbangkan adalah:

1. Fungsi bangunan yang akan dipikul oleh pondasi.
2. Beban yang harus dipikul.
3. Keadaan tanah dasar dimana pondasi akan didirikan.
4. Segi ekonomi pembuatan pondasi.

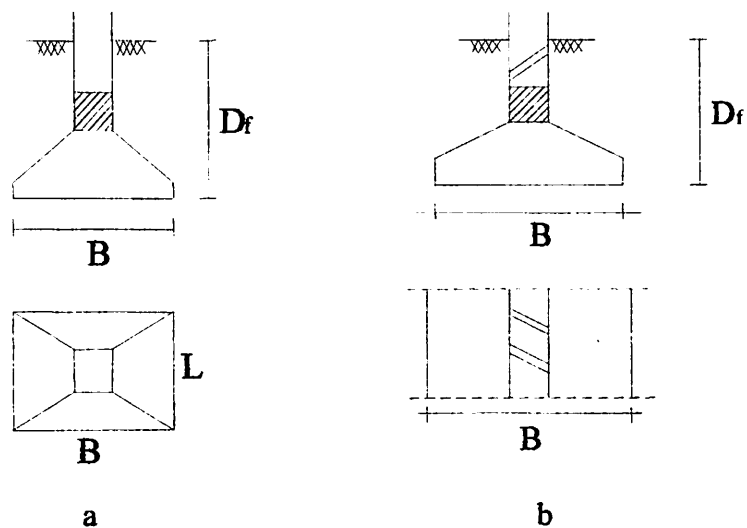
##### **2.1.1 Pondasi Langsung**

Pondasi langsung digunakan bila lapisan tanah yang mampu memikul pondasi relatif dalam dari permukaan tanah, dimana kedalaman maksimum antara 2-3 m, dan pondasi langsung diletakkan diatas tanah keras. Seperti pondasi setempat, pondasi telapak dan pondasi menerus.

Ada beberapa contoh dari pondasi langsung yaitu:

1. Pondasi telapak, adalah pondasi beton bertulang dan pondasi batu kali yang dibuat dibawah kolom struktur.
2. Pondasi menerus, adalah pondasi yang dibuat sepanjang arah melintang dan arah memanjang bangunan dibawah deretan kolom struktur.

Jenis pondasi ini sering digunakan pada bangunan – bangunan dengan pondasi yang langsung berada pada lapisan tanah keras yang mempunyai daya dukung yang cukup untuk menahan struktur bangunan tersebut.



**Gambar 2.1 Pondasi Langsung : a. Pondasi Telapak; b. Pondasi Menerus**

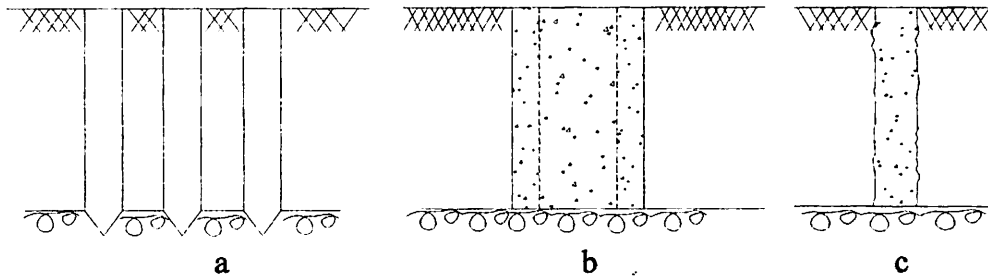
### **2.1.2 Pondasi Tidak Langsung**

Pondasi tidak langsung didefinisikan sebagai pondasi yang meneruskan beban bangunan ke tanah keras yang terletak relatif jauh dari permukaan dengan menggunakan perantara. Hal ini dimaksudkan untuk mendapatkan suatu kedalaman tanah keras yang mempunyai daya dukung yang cukup dengan penurunan sekecil mungkin.

Pada kondisi tertentu pondasi dapat juga diletakkan tidak pada lapisan tanah kerasnya, contohnya pondasi rakit. Dimana pondasi rakit tidak perlu mencapai lapisan tanah keras, karena perhitungan telah mampu untuk menahan semua gaya yang bekerja. Pondasi ini dibuat dari plat beton penuh seluas bangunan dan diletakkan pada lapisan tanah yang mempunyai daya dukung rendah atau berbeda-beda di beberapa tempat letak kolom atau dinding pendukung struktur berdekatan satu sama lainnya pada arah melintang dan memanjang. Untuk pondasi jenis ini termasuk pondasi tidak langsung, karena masih menggunakan perantara dalam meningkatkan daya dukungnya.

Sehingga dapat ditarik kesimpulan bahwa pondasi tidak langsung merupakan jenis pondasi yang digunakan pada struktur yang masih menggunakan struktur perantara untuk mencapai lapisan tanah keras yang diinginkan dan mempunyai daya dukung yang cukup.

Pondasi tidak langsung antara lain: Pondasi tiang Pancang, Pondasi Sumuran (kaison), dan Pondasi Strauss.



**Gambar 2.2 Pondasi Dalam : a. Pondasi tiang pancang; b. pondasi sumuran;  
c. Pondasi strauss**

## **2.2 Pondasi Tiang Bor (Strauss)**

Pondasi tiang bor adalah pilar pancang yang dibor dibuat dengan cara membuat sebuah lubang silindris hingga pada tanah keras yang cukup untuk memikul beban-beban dari struktur di atasnya dan sesudah itu diisi dengan adukan beton.

Untuk cara tersebut diatas dapat di laksanakan dengan dua cara yaitu :

1. Dengan pipa baja yang dipancangkan ke dalam tanah, kemudian diisi dengan beton dan ditumbuk sambil pipa baja tersebut di tarik keatas.
2. Dengan pipa baja yang dipancangkan kedalam tanah, kemudian diisi dengan beton sedangkan pipa baja tersebut tetap tinggal didalam tanah.

Tiang bor yang dicor di tempat mempunyai keuntungan dan kerugian.

Keuntungan yang di peroleh dari pemakaian pondasi tiang bor ini adalah :

1. Karena getaran pada saat melaksanakan sangat kecil sehingga cocok untuk pekerjaan pada daerah yang padat penduduk.
2. Karena tanpa sambungan, dapat dibuat tiang yang lurus dengan diameter yang besar dan juga untuk tiang yang panjang dapat dilakukan dengan mudah.
3. Diameter biasanya lebih besar dari pada tiang pracetak dan daya dukung tiap tiang juga lebih besar sehingga tumpuan dapat di buat lebih kecil.
4. Selain cara pengeboran dalam arah yang berlawanan jarum jam tanah galian dapat di amati secara langsung dan sifat-sifat tanah pada lapisan antara tanah pendukung pondasi dapat langsung di ketahui.
5. Pengaruh yang di timbulkan terhadap bangunan di dekatnya cukup kecil.
6. Kedalaman pipa dapat divariasikan.
7. Tanah dapat diperiksa dan dicocokkan dengan data laboratorium.

8. Tiang dapat dipasang sampai kedalaman yang direncanakan, dengan diameter besar dan dapat dilakukan pembesaran pada ujung bawahnya jika tanah dasar berupa lempung atau batu lunak.

Kerugian yang dapat di peroleh dari pemakaian pondasi tiang bor adalah :

1. Dalam banyak hal, beton dari tubuh tiang yang berada dibawah air kualitasnya setelah selesai lebih rendah dari tiang pracetak. Disamping itu, pemeriksaan kualitas hanya dapat dilakukan secara tidak langsung.
2. Ketika beton dituangkan, di khawatirkan adukan beton akan bercampur dengan runtunan tanah. Oleh karena itu beton harus di tuangkan dengan seksama setelah penggalian di lakukan.
3. Walaupun penetrasi sampai tanah pendukung pondasi dianggap telah terpenuhi kadang-kadang terjadi bahwa tiang pendukung kurang sempurna karena adanya lumpur yang tertimbun di dasar.
4. Karena diameter tiang cukup besar dan memerlukan banyak beton sehingga biaya yang di butuhkan juga cukup besar.
5. Karena pada cara pemasangan tiang yang di putar berlawanan jarum jam dipakai air maka, lapangan akan menjadi kotor, lagi



pula untuk setiap cara perlu diperhatikan bagaimana cara menangani tanah yang telah digali.

6. Pengecoran beton sulit bila dipengaruhi air tanah karena dapat mengurangi mutu beton tersebut
7. Air yang mengalir kelubang bor dapat mengakibatkan gangguan tanah, gangguan pengecoran sehingga mengurangi kapasitas daya dukung tanah terhadap tiang.
8. Pembesaran ujung bawah tiang tidak dapat dilakukan bila tanah berupa pasir.

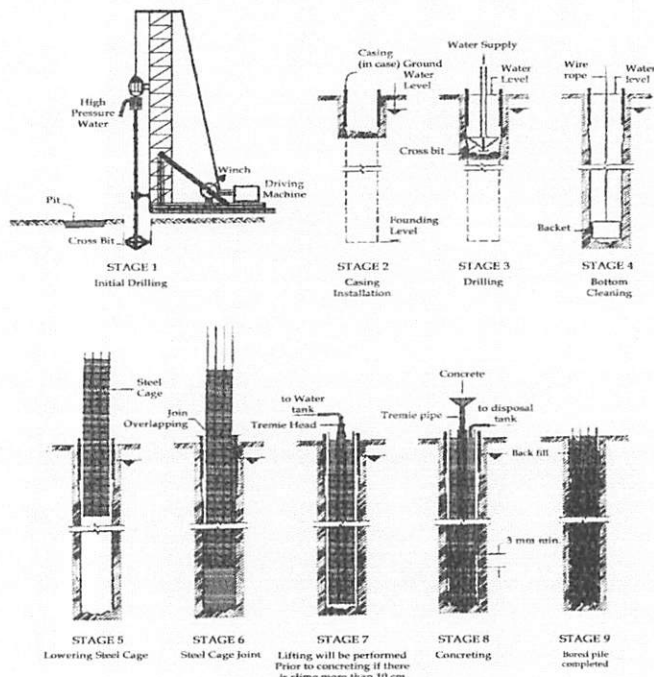
### **2.3 Dasar Perencanaan Pondasi Tiang Bor (Strauss)**

Pondasi tiang digunakan bilamana lapisan-lapisan bagian atas dari tanah tidak cukup kuat, sehingga tidak cukup kuat untuk memikul bangunan dengan memakai pondasi langsung.

Ada beberapa cara yang dipergunakan untuk memasang tiang, salah satunya dengan membuat lubang terlebih dahulu kemudian dimasukan besi tulangan yang sudah dirangkaikan lalu dicor beton. Tiang semacam ini biasanya disebut tiang strauss.

Cara pengerjaan pondasi tiang strauss adalah sebagai berikut:

1. Pada tempat tiang strauss yang akan didirikan dibuat lubang vertikal dengan cara mengebor dengan alat bor sampai dengan kedalaman yang direncanakan.
2. Setelah pengeboran selesai, lubang tersebut dimasukan pipa baja. Ujung pipa tersebut dimasukan sampai menempel pada dasar lubang bor.
3. Kemudian campuran beton dimasukan ke dalam pipa.
4. Campuran beton kering pada dasar pipa tersebut ditumbuk beberapa kali sambil pipa tersebut secara pelahan diangkat keatas dengan ketinggian tertentu. Setelah permukaan atas beton kering yang ditumbuk itu mencapai ketinggian yang sama dengan ujung pipa maka penumbukan dihentikan.
5. Rangkaian tulangan bulat dimasukan kedalam pipa dan kemudian dilakukan pengecoran dengan beton cair yang sudah diaduk dalam truck ready mix. Bersamaan dengan pengecoran beton cair tersebut dipadatkan dengan vibrator dan pipa diangkat perlahan sampai seluruh lubang terisi dengan beton.



**Gambar 2.3 : 1. Pemasangan pengeboran**

**2. Pemasangan casing**

**3. Proses pengeboran**

**4. Pembersihan dasar pengeboran**

**5. Menurunkan tulangan**

**6. Penempatan tulangan**

**7. Angkat tulangan bila ada lumpur setinggi 10 cm**

**8. Pengecoran tulangan**

Untuk perencanaan (*design*), tiang dapat dibagi menjadi dua golongan :

1. Tiang yang tertahan pada ujung

Tiang semacam ini dimasukan sampai lapisan yang keras sehingga beban bangunan dipikul pasa lapisan in. Bila lapisan ini

merupakan batu keras maka penentuan data dukung tiang tidak menjadi soal. Daya dukung dalam hal ini tergantung pada kekuatan tiang sendiri dan dapat dihitung dari tegangan yang diperbolehkan bahan tiang.

Apabila lapisan keras terdiri dari pasir maka daya dukung tiang tergantung pada sifat-sifat pasir tersebut dan kita harus dapat mengetahui besarnya gaya melawan lapisan tersebut terhadap ujung tiang.

## 2. Tiang yang tertahan oleh perletakan antara tiang dan tanah.

Bila ujung tiang tidak mencapai tanah keras, maka yang tertahan adalah perletakan antara ting dengan tanah. Tiang semacam ini juga disebut tiang terapung atau *floating pile*. Bila tiang semacam ini tidak dimasukan dalam pasir maka sebagian besar daya dukungnya masih tergantung pada ujungnya dan dapat dihitung dari hasil sondir dan bilamana tiang ini dimasukan dalam lapisan lempung maka perlawanan ujung akan lebih kecil dari perlawanan akibat pelekatan antara tiang dengan tanah, karena itu untuk menghitung daya dukung tiang ini dalam lempung kita harus dapat menentukan besarnya gaya pelekatan antara tiang dengan tanah.

## 2.4 Daya Dukung Pondasi Tiang Tunggal

Daya dukung ( bearing Capacity) adalah kemampuan tanah untuk mendukung beban baik dari segi struktur pondasi maupun bangunan di atasnya tanpa terjadi keruntuhan. Apabila beban yang bekerja pada tanah pondasi telah melampaui batas daya dukung dan tegangan geser maka akan berakibat keruntuhan pada pondasi.

Persamaan daya dukung tiang secara umum dirumuskan sebagai berikut :

$$Q = \frac{Q_p}{SF_1} + \frac{Q_f}{SF_2}$$

Dimana :

Q = daya dukung ijin tiang (kg)

$Q_p$  = daya dukung ujung tiang (kg)

$Q_f$  = daya dukung friksi (kg)

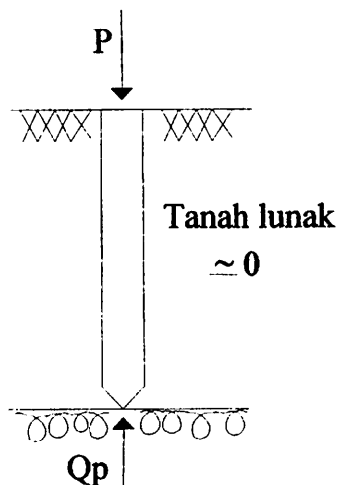
$SF_1$  = angka keamanan ujung tiang (2,5)

$SF_2$  = angka keamana selimut tiang (5)

### 2.4.1 Daya Dukung Berdasarkan Tahanan Ujung

End bearing tiang pancang yang tertahan ujungnya (*daya dukung ujung*). Tiang ini meneruskan beban melalui tahanan ujung ke lapisan tanah keras yang mampu memikul beban yang diterima oleh tiang tersebut.

Untuk menaksir gaya perlawanan lapisan tanah keras tersebut terhadap ujung tiang digunakan alat sondir. Dengan alat sondir kita dapat menentukan sampai berapa dalam tiang yang harus dipancangkan dan berapa daya dukung lapisan keras tersebut terhadap ujung tiang (Sardjono HS, 1998, Hal 32).



**Gambar 2.4 : skema end bearing pile**

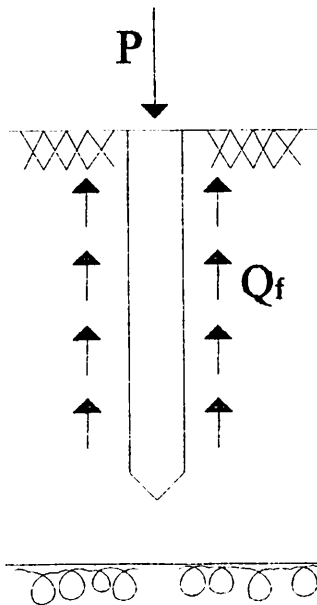
Rumus yang di gunakan :

$$Q_p = (A_{\text{tiang}} \cdot q_c) / SF_1$$

#### 2.4.2 Daya Dukung Berdasarkan Gesekan Tiang Dengan Tanah

Friction pile adalah tiang yang daya dukungnya berdasarkan hambatan lekat antara tiang dengan tanah. Bila dilihat dari cara fungsinya dapat dibedakan menjadi :

1. Tiang gesekan (*friction*) pada tanah berbutir kasar yang sangat permeable. Tiang ini memindahkan sebagian besar bebannya ke tanah melalui gesekan antara permukaan keliling tiang dengan tanah. Bila jarak tiang berdekatan antara satu dengan lainnya bisa menyebabkan menurunnya porositas dan menyebabkan pemadatan tanah sekelilingnya. Pada keadaan ini tiang termasuk kategori “*compaction pile*”.
2. Tiang gesekan (*friction pile*) pada tanah berbutir sangat halus dengan permeabilitas rendah. Tiang ini juga memindahkan bebannya ke tanah melalui gesekan antara muka keliling tiang dengan tanah, tapi tidak memadatkan tanah ditempat tersebut. Pondasi yang didukung oleh tipe ini biasa disebut tiang terapung (*floating pile foundation*).



**Gambar 2.5 : skema friction pile**

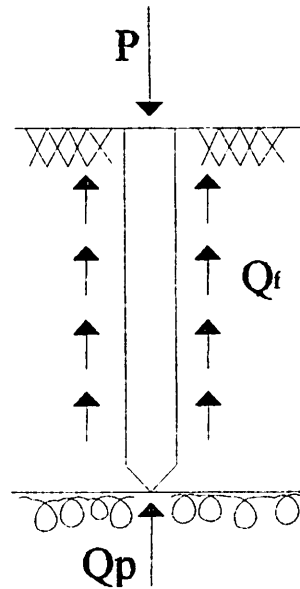
Rumus yang digunakan pada kondisi ini yaitu:

$$Q_f = (\Theta \cdot JHL) / SF_2$$

### **2.4.3 Daya Dukung Berdasarkan Kombinasi Ujung Dengan Gesekan**

Kombinasi antara daya dukung ujung dan tahanan kulit digunakan apabila tekanan konus dan hambatan lekat pada tanah tidak sama dengan nol. Untuk mengetahui besarnya perlawanan lapisan tanah tersebut digunakan alat sondir.





**Gambar 2.6 : Skema combined bearing pile**

◆ **Kemampuan daya dukung kombinasi**

Rumus :

$$Q_u = [(A_p \cdot q_c) / SF_1] + [(\odot \cdot JHL) / SF_2]$$

Dimana :

$Q_p$  = Daya dukung tiang (kg)

$Q_f$  = Daya dukung friksi (kg)

$A_p$  = Luas penampang tiang (cm<sup>2</sup>)

$\odot$  = Keliling tiang (cm)

JHL = Jumlah Hambatan Lekat (kg/cm)

SF<sub>1</sub> = Faktor keamanan ujung tiang

SF<sub>2</sub> = Faktor keamanan dinding tiang

## **2.5 Daya Dukung Pondasi Tiang Bor (Strauss)**

Pondasi tiang bor di pasang kedalam tanah dengan cara mengebor terlebih dahulu, oleh karena itu dalam menghitung daya dukung hanya menggunakan daya dukung ujung tiang saja. Tiang dengan tertahan ujung (End Bearing Pile) adalah tiang yang meneruskan beban melalui tahanan ujung ke lapisan tanah keras.

Tetapi perlu diperhatikan juga adanya tekanan tanah lateral pada waktu pemasangan tiang bor, sehingga memberikan kontribusi daya dukung yang berupa lekatan atau daya dukung Adhesi. Penelitian pengaruh pekerjaan pemasangan tiang bor pada adhesi antara dinding tiang dan tanah di sekitarnya menunjukkan bahwa adhesi lebih kecil dari pada nilai tak terdrainase tanah sebelum pemasangan tiang. Hal ini akibat dari pelunakan lempung di sekitar lubang dinding. Pelunakan tersebut adalah pengaruh dari bertambahnya kadar air lempung oleh pengaruh-pengaruh air pada pengecoran beton, pangalihan air tanah ke zona yang bertekanan lebih rendah di sekitar lubang bor, dan air yang dipakai untuk pelaksanaan pembuatan lubang bor. Pelunakan pada tanah lempung dapat di kurangi jika pengeboran dan pengecoran di laksanakan dalam waktu 1 atau 2 jam (Palmer Dan Holland, 1966).

♦ Untuk tiang bor menggunakan rumus sebagai berikut :

$$Q_{all} = \frac{q_c \cdot A_p}{SF_1} + 0.5 \frac{JHL \cdot O}{SF_2}$$

$$Q_u = q_c \cdot A_p + 0.5 \cdot JHP$$

Dimana :

$Q_u$  = daya dukung ultimit

$q_c$  = nilai konus dari hasil sondir

$A_p$  = luas penampang tiang

JHL = jumlah hambatan lekat

$SF_1, SF_2$  = angka keamanan

$\odot$  = keliling tiang

Mayerhof (1976) menyarankan bahwa  $q_c$  adalah rata-rata di hitung dari 8d diatas dasar tiang sampai 4d dibawah dasar tiang.

Untuk diameter tiang ( $d$ ) lebih dari 2 m. Kapasitas tiang ijin perlu dievaluasi dari pertimbangan penurunan tiang. Selanjutnya, penurunan struktur harus pula dicek terhadap persyaratan besar penurunan toleransi yang masih diijinkan.

Maksud penggunaan faktor-faktor aman adalah untuk menyakinkan keamanan tiang terhadap keruntuhan tiang dengan mempertimbangkan penurunan tiang pada beban kerja yang diterapkan. Untuk menentukan faktor keaman dapat digunakan struktur bangunan menurut Pugsley (1966) sebagai berikut :

**Tabel 2.1 : Faktor Keamanan Untuk Pondasi Tiang**

Klasifikasi Struktur	Faktor Aman			
	Kontrol Baik	Kontrol Normal	Kontrol Jelek	Kontrol Sangat Jelek
Monumental	2.3	3	3.5	4
Permanen	2	2.5	2.8	3.4
Sementara	1.4	2.0	2.3	2.8

\*(Sumber : Reese & O'Neil, 1989; Pugsley, 1966)

1. Bangunan monumental, umumnya memiliki umur rencana melebihi 100 tahun, seperti Tugu Monas, Monumen Garuda Wisnu Kencana, jembatan-jembatan besar, dan lain-lain.
2. Bangunan permanen, umumnya adalah bangunan gedung, jembatan, jalan raya dan jalan kereta api, dan memiliki umur rencana 50 tahun.

3. Bangunan sementara, umur rencana bangunan kurang dari 25 tahun, bahkan mungkin hanya beberapa saat saja selama masa konstruksi.

Faktor-faktor lain kemudian ditentukan berdasarkan tingkat pengendaliannya pada saat konstruksi.

1. Pengendalian Baik : kondisi tanah cukup homogen dan konstruksi di dasarkan pada program penyelidikan geoteknik yang tepat dan profesional, terdapat informasi uji pembebanan di atau dekat proyek dan pengawasan konstruksi di dilaksanakan secara ketat.
2. Pengendalian Normal : Situasi yang paling umum, hampir serupa dengan kondisi diatas, tetapi kondisi tanah bervariasi dan tidak tersedia data pengujian tiang.
3. Pengendalian Kurang : Tidak ada uji pembebanan, kondisi tanah sulit dan bervariasi, pengawasan pekerjaan kurang, tetapi pengujian geoteknik dilakukan dengan baik.
4. Pengendalian Buruk : Kondisi tanah amat buruk dan sukar ditentukan, penyelidikan geoteknik tidak memadai.

## 2.6 Daya Dukung Kelompok Tiang

Penentuan daya dukung vertikal sebagai tiang dalam kelompok perlu dihitung dulu efisiensi dari tiang tersebut didalam kelompok, karena daya dukung vertikal sebuah tiang yang berdiri adalah tidak sama besarnya yang berada dalam suatu kelompok.

Efisiensi  $\eta$  adalah perbandingan hambatan kulit pada garis keliling kelompok terhadap jumlah tahanan kulit masing-masing tiang pancang. Misalkan banyaknya baris adalah (n) dan banyaknya kolom (m) dan jarak masing-masing tiang (s), maka banyaknya tiang  $K = m.n$ .

$$\eta = \frac{\text{Daya dukung kelompok tiang}}{\text{Jumlah tiang} \times \text{Daya dukung tiang tunggal}} = \frac{Q_{pg}}{n \times Q_s}$$

Penentuan daya dukung kelompok tiang dihitung berdasarkan faktor efisiensi seperti rumus dibawah ini :

$$Q_{\text{tiang}} = \eta \cdot k \cdot Q_{1 \text{ tiang}}$$

Dimana :

$Q_{\text{tiang}}$  = daya dukung yang diijinkan untuk sebuah tiang dalam kelompok

$Q_{1 \text{ tiang}}$  = daya dukung yang diijinkan untuk tiang tunggal

k = jumlah tiang

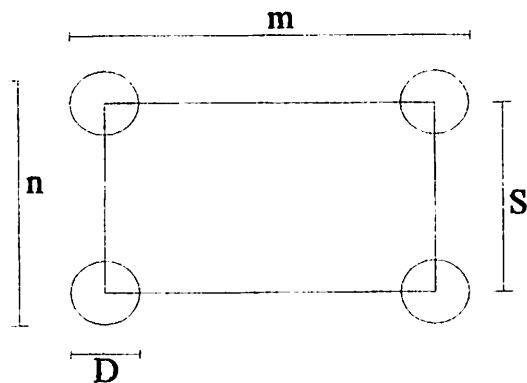
$\eta$  = Efisiensi kelompok tiang

Untuk menghitung daya dukung kelompok digunakan perhitungan

seperti :

a) Jarak antara tiang dalam kelompok

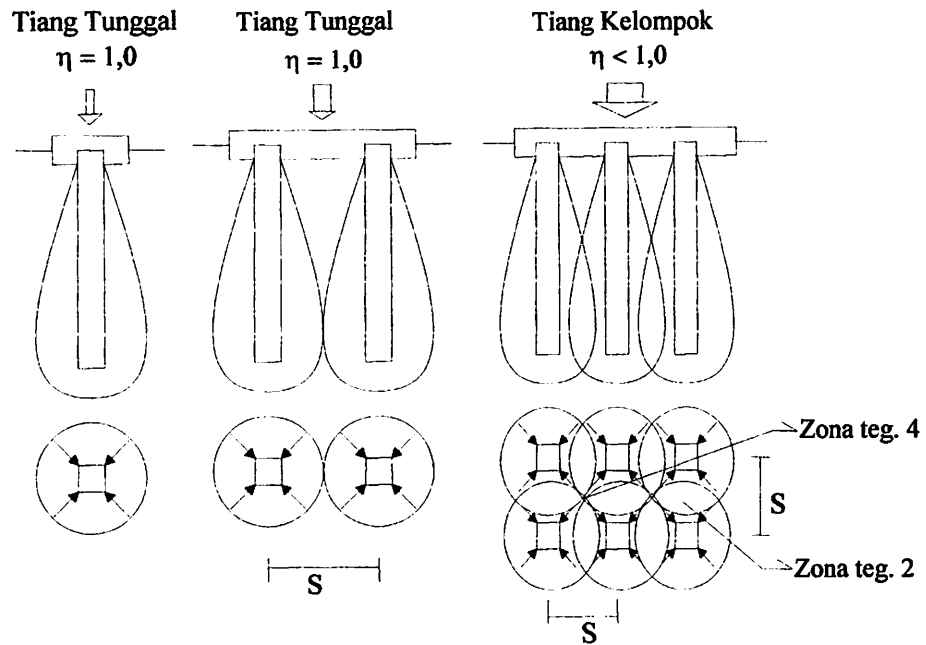
syarat jarak tiang :



Gambar 2.7 : Skema Jarak Antar Tiang

- \*  $S \geq 2,5D$  → Jika terlalu rapat, kemungkinan tiang berdekatan akan terangkat pada saat pemancangan.
- \*  $S \leq \frac{1,57 \cdot D \cdot m \cdot n - 2D}{(m + n) - 2}$  → Syarat agar efisiensi,  $\eta < 1$  dan konstruksi akan aman.
- \*  $S \leq 2,00 \text{ m}$  → Jika terlalu renggang, konstruksi poer akan mahal.
- \*  $S \leq \frac{1,57 \cdot D \cdot m \cdot n}{(m + n) - 2}$  → Konstruksi akan lebih ekonomis tetapi kurang aman.

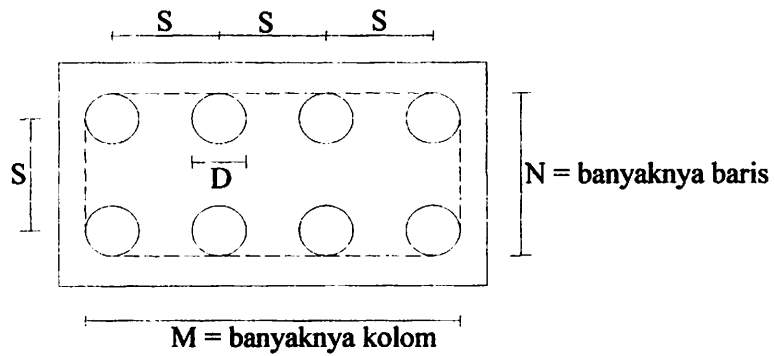
Kontribusi daya dukung tiang yang dihasilkan dari lekatan atau friksi kulit tiang dengan tanah di sekeliling tiang (lihat sketsa).



**Gambar 2.8 : Skema Efisiensi Kelompok Tiang**

Efisiensi kelompok tiang dengan rumus sederhana:

$$\eta = \frac{2 \cdot (m + n - 2) \cdot S + 4 \cdot D}{p \cdot m \cdot n}$$





Dimana :

m = jumlah baris tiang

n = jumlah tiang dalam baris

D = diameter tiang (cm)

S = jarak antara as ke as tiang (cm)

p = keliling dari penampang tiang

Rumus efisiensi kelompok banyak sekali ragamnya, di bawah ini disajikan beberapa rumus efisiensi yang lazim digunakan dalam hitungan. Apabila hitungan dilakukan dengan lebih dari satu macam rumus, maka angka efisiensi diambil yang terkecil karena akan diperoleh safety factor yang paling aman.

Adapun rumus-rumus tersebut antara lain :

a. Rumus Converse-labarre

$$\eta = 1 - \frac{\phi}{90} \left[ \frac{(n-1).m + (m-1).n}{m.n} \right]$$

Dimana :

$$\phi = \text{arc tg } \frac{D}{S}$$

**b. Rumus Los Angeles Group**

$$\eta = 1 - \frac{D}{\pi \cdot m \cdot S \cdot n} \left[ m \cdot (n-1) + n \cdot (m-1) + \dots + (m-1)(n-1) \cdot \sqrt{2} \right]$$

**c. Rumus Seiler Keeny**

$$\eta = \left[ 1 - \frac{(36 \cdot S) \cdot (m + n - 2)}{75 \cdot (S^2 - 7) \cdot (m + n - 1)} \right] + \left[ \frac{0,3}{(m + n)} \right]$$

**Dimana :**

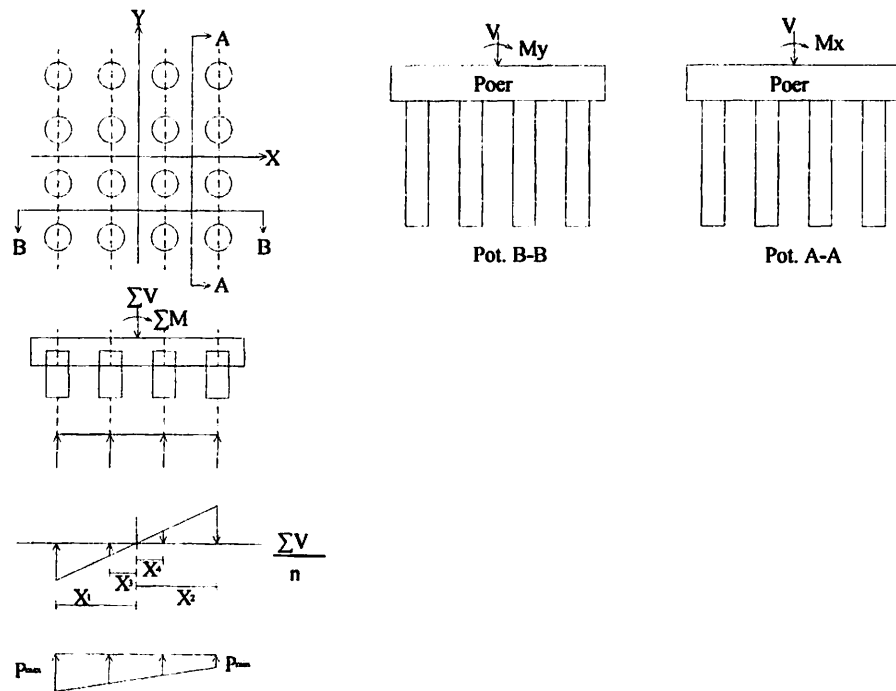
**m** = jumlah baris tiang

**n** = jumlah tiang dalam baris

**D** = diameter tiang

**S** = jarak antara as ke as tiang

b) Kelompok tiang yang menerima beban normal sentries dan momen yang bekerja pada dua arah.



**Gambar 2.9 : Skema Pondasi Tiang Kelompok**

Dari gambar diatas dapat dirumuskan :

$$P_{\max} = \frac{P_{\text{total}}}{n} \pm \frac{My \cdot X_{\max}}{ny \cdot \Sigma x^2} \pm \frac{Mx \cdot Y_{\max}}{nx \cdot \Sigma y^2}$$

Dimana :

$P_{\max}$  = Beban maksimum yang diterima oleh tiang (kg)

$P_{\text{total}}$  = Beban vertikal yang diterima oleh kelompok tiang (kg)

$n$  = Banyaknya jumlah tiang (buah)

$X_{\max}$  = Jarak terjauh tiang ke pusat berat kelompok tiang searah sumbu X (m)

$Y_{\max}$  = Jarak terjauh tiang ke pusat berat kelompok tiang searah sumbu Y (m)

$M_x$  = Momen yang bekerja pada bidang yang tegak lurus sumbu x (kgm)

$M_y$  = Momen yang bekerja pada bidang yang tegak lurus sumbu y (kgm)

$n_x$  = Banyak tiang dalam satu baris searah sumbu x (buah)

$n_y$  = Banyak tiang dalam satu baris searah sumbu y (buah)

$\sum X^2$  = Jumlah kuadrat absis tiang ( $m^2$ )

$\sum Y^2$  = Jumlah kuadrat ordinat tiang ( $m^2$ )

Apabila dalam merencanakan pondasi tiang bor kontrol daya dukung tidak memenuhi, maka dalam perencanaan kita dapat menambah daya dukung dengan cara menyesuaikan kedalaman dan diameter tiang.

## **BAB III**

### **ANALISA PEMBEBANAN DAN STATIKA**

#### **3.1. Data perencanaan**

Dalam merencanakan suatu pondasi harus didukung dengan data-data yang dapat dipertanggung jawabkan secara teknis, agar hasil yang di dapatkan sesuai dengan yang di inginkan. Pada perencanaan pondasi strauss data yang di perlukan adalah sebagai berikut :

##### **1. Spesifikasi Umum**

- a. Fungsi Bangunan = Rusunawa ( Rumah Susun Mahasiswa )
- b. Jumlah Lantai = 4 Lantai
- c. Struktur Lantai = Plat Beton Bertulang
- d. Bentang Memanjang = 59.7 m
- e. Bentang Melintang = 16.8 m
- f. Data Tanah = Uji Sondir
- g. Struktur bawah = Pondasi Strauss
- h. Zona Gempa = 4 (Malang)

## 3.2 Pembebanan

### 3.2.1 Pembebanan Plat Lantai

#### Lantai 2-4

◆ Beban Mati (qd)

- Berat sendiri plat lantai =  $0.12 \times 2400 \text{ kg/m}^2 = 288 \text{ kg/m}^2$
- Berat plafon + penggantung =  $(11 + 7) \text{ kg/m}^2 = 18 \text{ kg/m}^2$
- Berat spesi =  $2 \times 21 \text{ kg/m}^2 = 42 \text{ kg/m}^2$
- Berat pasir =  $0.03 \times 1800 = 54 \text{ kg/m}^2$
- Berat tegel =  $2 \times 24 = \underline{48 \text{ kg/m}^2}$

$$q_d = 450 \text{ kg/m}^2$$

◆ Beban Hidup (ql)

- Gedung asrama =  $250 \text{ kg/m}^2$

#### Lantai Atap

◆ Beban mati (qd)

- Berat sendiri plat lantai =  $0.12 \times 2400 \text{ kg/m}^2 = 288 \text{ kg/m}^2$
- Berat plafon + penggantung =  $(11 + 7) \text{ kg/m}^2 = 18 \text{ kg/m}^2$
- Berat pasir =  $0.03 \times 1800 = 54 \text{ kg/m}^2$
- Berat spesi =  $2 \times 21 \text{ kg/m}^2 = \underline{42 \text{ kg/m}^2}$

$$q_d = 438 \text{ kg/m}^2$$

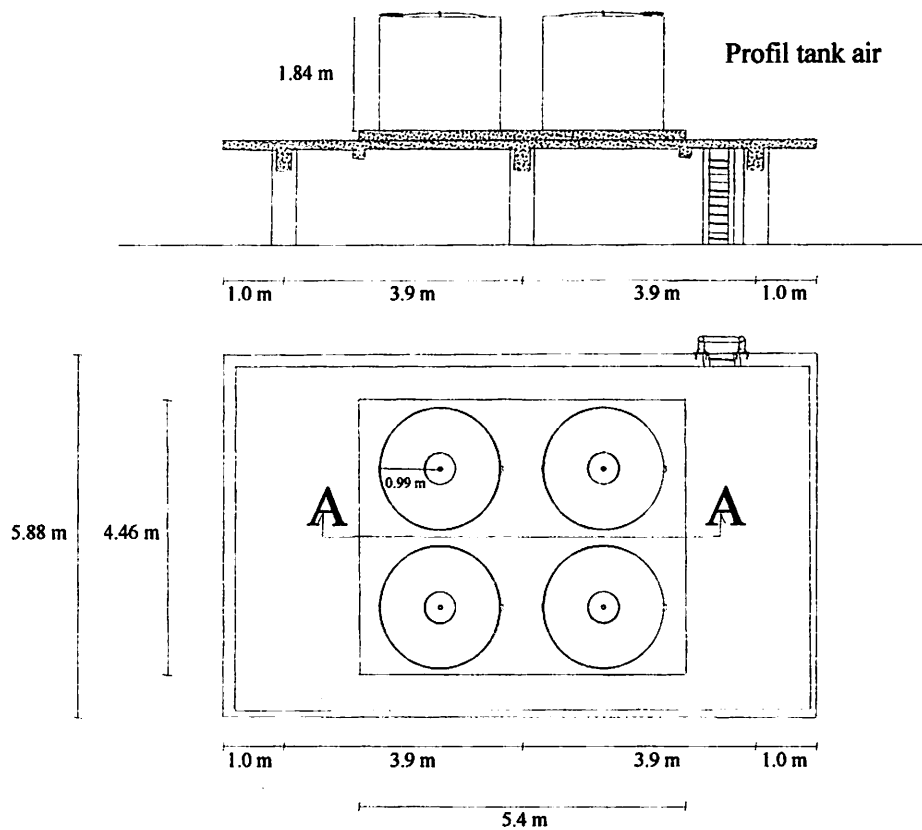
◆ **Beban hidup**

- **Beban orang** =  $100 \text{ kg/m}^2$

- **Beban air hujan** =  $0.05 \times 1000$  =  $50 \text{ kg/m}^2$

$$q_l = 150 \text{ kg/m}^2$$

**AIR DALAM TANDON**



**Gambar 3.1 Detail profil tank air**

Diketahui :

$$r = 0.99 \text{ m}$$

$$t = 1.84 \text{ m}$$

$$I = L.t$$

Dimana =

I = Isi tabung

L = luas alas

t = tinggi

$$L = \pi.r^2$$

$$= 3.14.0.99^2$$

$$= 3.08 \text{ m}^2$$

$$I = L.t$$

$$= 3.08.1.84$$

$$= 5.67 \text{ m}^3$$

Kapasitas satu buah tendon  $5 \text{ m}^3$ , jadi untuk 4 tandon  $4 \times 5 = 20 \text{ m}^3$ .

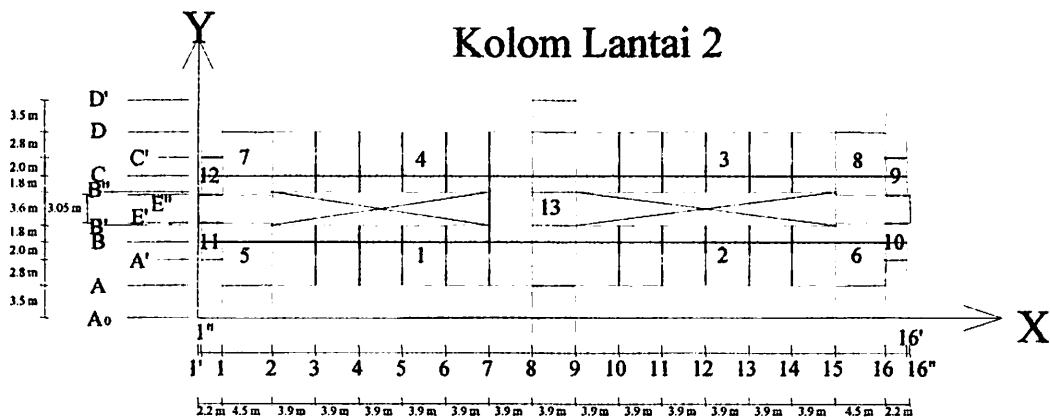


### 3.2.2 Perhitungan Pusat Massa

#### Perhitungan Pusat Massa Lantai 2

a). Kolom

Garis patokan arah memanjang (X) adalah line A<sup>0</sup>, sedangkan untuk arah melintang (Y) garis patokannya adalah line 0.



Gambar 3.2 : Denah Pusat Massa Kolom Lantai 2

Berat sendiri = luas x ( $\frac{1}{2}$  x tinggi lantai bawah +  $\frac{1}{2}$  x lantai atas) x 2400

Jarak = jarak patokan kolom ke garis patokan A<sup>0</sup>

➤ Titik A-2

◆ Berat sendiri =  $0.3 \times 0.45 \times \frac{1}{2} \times (3.4+3.2) \times 2400 = 1069.20$  Kg.

◆ Jarak (X) pusat massa element ke pusat sumbu X-Y = 6.70 m.

◆ Jarak (Y) pusat massa element ke pusat sumbu X-Y = 3.50 m.

◆ Berat x Jarak (X) =  $1069.20 \times 6.70 = 7163.64$  Kgm.

◆ Berat x Jarak (Y) =  $1069.20 \times 3.50 = 3742.20$  Kgm.

➤ Titik A-3

◆ Berat sendiri =  $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400 = 1069.20 \text{ Kg}$ .

◆ Jarak (X) pusat massa element ke pusat sumbu XY = 10.60 m.

◆ Jarak (Y) pusat massa element ke pusat sumbu XY = 3.50 m.

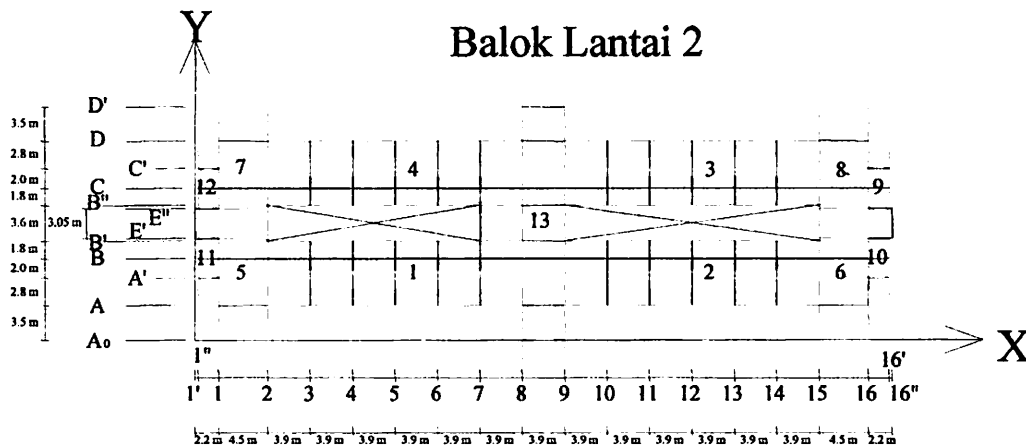
◆ Berat x Jarak (X) =  $1069.20 \times 10.60 = 11503.40 \text{ Kgm}$ .

◆ Berat x Jarak (Y) =  $1069.20 \times 3.50 = 3742.20 \text{ Kgm}$ .

Untuk perhitungan selanjutnya dapat dilihat pada tabel 3.1

b). Balok

Garis patokan arah memanjang (X) adalah line A<sup>0</sup>, sedangkan untuk arah melintang (Y) garis patokannya adalah line 0.



**Gambar 3.3 : Denah Pusat Massa Balok Lantai 2**

Berat sendiri = luas x panjang x 2400

Jarak = jarak patokan kolom ke garis patokan A<sup>0</sup>

➤ Balok line A (2-8)

- ◆ Berat sendiri =  $0.2 \times 0.4 \times 23.40 \times 2400$  = 4492.8 Kg.
- ◆ Jarak (X) pusat massa element ke pusat sumbu XY = 18.40 m.
- ◆ Jarak (Y) pusat massa element ke pusat sumbu XY = 3.50 m.
- ◆ Berat x Jarak (X) =  $4492.8 \times 18.40$  = 82667.52 Kgm.
- ◆ Berat x Jarak (Y) =  $4492.8 \times 3.50$  = 15724.80 Kgm.

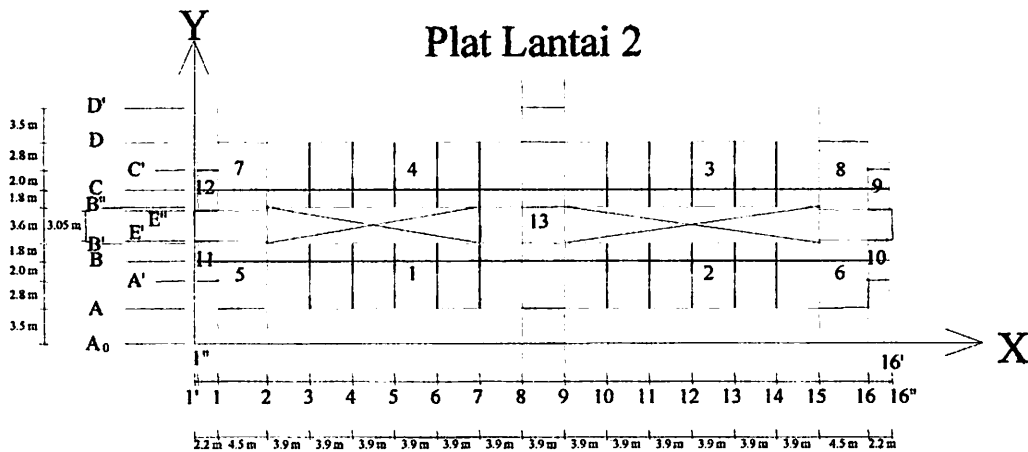
➤ Balok line B' (2-8)

- ◆ Berat sendiri =  $0.2 \times 0.4 \times 23.40 \times 2400$  = 4492.8 Kg.
- ◆ Jarak (X) pusat massa element ke pusat sumbu XY = 18.40 m.
- ◆ Jarak (Y) pusat massa element ke pusat sumbu XY = 10.10 m.
- ◆ Berat x Jarak (X) =  $4492.8 \times 18.40$  = 82667.52 Kgm.
- ◆ Berat x Jarak (Y) =  $4492.8 \times 10.10$  = 45377.28 Kgm.

Untuk perhitungan selanjutnya dapat dilihat pada tabel 3.2

c). Plat

Garis patokan arah memanjang (X) adalah line A<sup>0</sup>, sedangkan untuk arah melintang (Y) garis patokannya adalah line 0.



**Gambar 3.4 : Denah Pusat Massa Plat Lantai 2**

Berat sendiri = luas x tebal x 2400

Jarak = jarak patokan kolom ke garis patokan A<sup>0</sup>

➤ Line A-B' (2-8)

◆ Berat sendiri =  $23.40 \times 6.80 \times 0.12 \times 2400 = 44478.72 \text{ Kg.}$

◆ Jarak (X) pusat massa element ke pusat sumbu XY = 18.40 m.

◆ Jarak (Y) pusat massa element ke pusat sumbu XY = 6.80 m.

◆ Berat x Jarak (X) =  $44478.72 \times 18.40 = 818408.45 \text{ Kgm.}$

◆ Berat x Jarak (Y) =  $44478.72 \times 6.80 = 302455.30 \text{ Kgm.}$

➤ Line A-B' (9-15)

◆ Berat sendiri =  $23.40 \times 6.80 \times 0.12 \times 2400$  = 44478.72 Kg.

◆ Jarak (X) pusat massa element ke pusat sumbu XY = 45.69 m.

◆ Jarak (Y) pusat massa element ke pusat sumbu XY = 6.80 m.

◆ Berat x Jarak (X) =  $44478.72 \times 45.69$  = 2032232.7 Kgm.

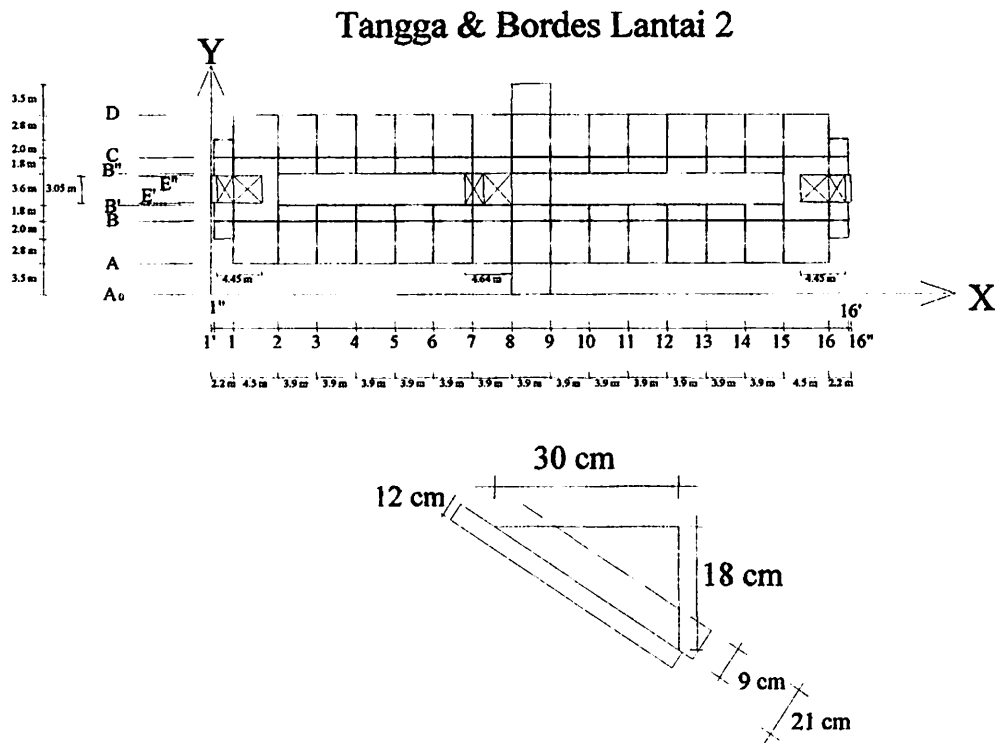
◆ Berat x Jarak (Y) =  $44478.72 \times 6.80$  = 302455.30 Kgm.

Untuk perhitungan selanjutnya dapat dilihat pada tabel 3.3

Untuk perhitungan pusat massa pada lantai berikutnya dapat dilihat pada lampiran

d). Tangga dan Bordes

Garis patokan arah memanjang (X) adalah line A<sup>0</sup>, sedangkan untuk arah melintang (Y) garis patokannya adalah line 0.



**Gambar 3.5 : Denah Pusat Massa Tangga dan Detail Lantai 2**

Berat sendiri = (Panjang miring x lebar x tebal plat tangga x 2400) +  
(Jumlah anak tangga x 0.5 x panjang anak tangga x lebar anak tangga x  
tinggi anak tangga x 2400)

Jarak = jarak patokan kolom ke garis patokan A<sup>0</sup>

➤ Tangga Samping Kiri

$$\begin{aligned} \blacklozenge \text{ Berat sendiri} &= (2.754 \times 1.375 \times 0.21 \times 2400) + (9 \times 0.5 \times \\ &1.375 \times 0.2 \times 2400) &= 7578.52 \text{ Kg.} \end{aligned}$$

- ◆ Jarak (X) pusat massa element ke pusat sumbu XY = 3.63 m.
- ◆ Jarak (Y) pusat massa element ke pusat sumbu XY = 11.90 m.
- ◆ Berat x jarak (X) = 7578.52 x 3.63 = 27510.03 Kgm.
- ◆ Berat x jarak (Y) = 7578.52 x 11.90 = 90184.41 Kgm.

➤ Bordes Samping Kiri

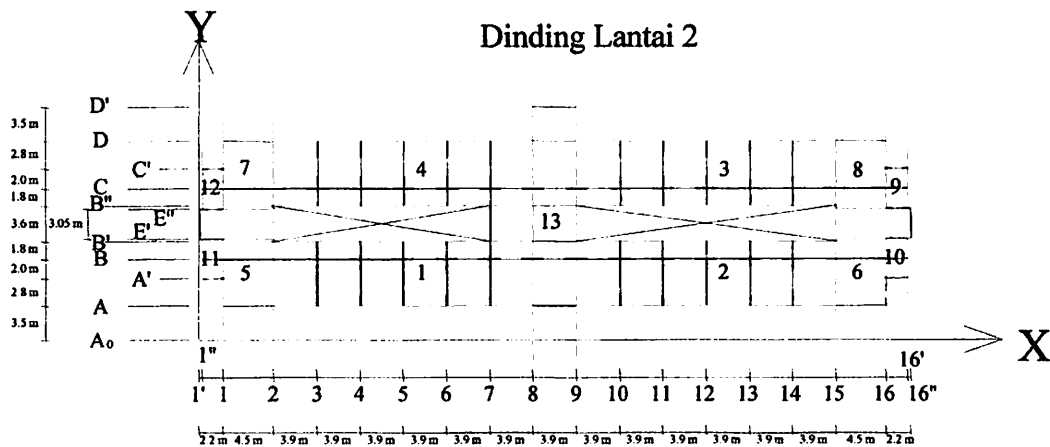
Berat sendiri = (n jumlah balok bordes x lebar balok bordes x tinggi balok bordes x 2400) + (tebal bores x lebar bordes x tinggi bordes x 2400)

- ◆ Berat sendiri = 2 x (0.2 x 0.3 x 1.825 x 2400) + (0.12 x 3.05 x 2400) = 1404Kg.
- ◆ Jarak (X) pusat massa element ke pusat sumbu XY = 1.40 m.
- ◆ Jarak (Y) pusat massa element ke pusat sumbu XY = 11.90 m.
- ◆ Berat x jarak (X) = 1404 x 1.40 = 1965.60 Kgm.
- ◆ Berat x jarak (Y) = 1404 x 11.90 = 16707.60 Kgm.

Untuk perhitungan selanjutnya dapat dilihat pada tabel 3.4

e). Dinding

Garis patokan arah memanjang (X) adalah line A<sup>0</sup>, sedangkan untuk arah melintang (Y) garis patokannya adalah line 0.



**Gambar 3.6 : Denah Pusat Massa Dinding Lantai 2**

Berat sendiri = Panjang x tinggi x 250

Jarak = jarak patokan kolom ke garis patokan A<sup>0</sup>

➤ Line A (2-8)

◆ Berat sendiri = 23.40 x 2.9 x 250 = 16965 Kg.

◆ Jarak (X) pusat massa element ke pusat sumbu XY = 18.40 m.

◆ Jarak (Y) pusat massa element ke pusat sumbu XY = 3.50 m.

◆ Berat x Jarak (X) = 16965 x 18.40 = 312156.00 Kgm.

◆ Berat x Jarak (Y) = 16965 x 3.50 = 59377.50 Kgm.

➤ Titik 2 (A-B)

◆ Berat sendiri = 4.80 x 2.9 x 250 = 3480 Kg.



- ◆ Jarak (X) pusat massa element ke pusat sumbu XY = 6.70 m.
- ◆ Jarak (Y) pusat massa element ke pusat sumbu XY = 5.90 m.
- ◆ Berat x Jarak (X) = 3480 x 6.70 = 23316.00 Kgm.
- ◆ Berat x Jarak (Y) = 3480 x 5.90 = 20532.00 Kgm.

Untuk perhitungan selanjutnya dapat dilihat pada tabel 3.5

$\Sigma$ Berat grafitasi	= 138532.92	Kg
$\Sigma$ [Berat grafitasi x Jarak (X)]	= 2549005.73	Kgm
$\Sigma$ [Berat grafitasi x Jarak (Y)]	= 928551.94	Kgm

$$\begin{aligned} \text{Pusat massa arah memanjang (X)} &= \frac{\Sigma \text{Berat gravitasi} \times \text{jarak}}{\Sigma \text{Berat gravitasi}} \\ &= \frac{2549005.73}{138532.92} = 18.40 \text{ m} \end{aligned}$$

$$\begin{aligned} \text{Pusat massa arah memanjang (Y)} &= \frac{\Sigma \text{Berat gravitasi} \times \text{jarak}}{\Sigma \text{Berat gravitasi}} \\ &= \frac{928551.94}{138532.92} = 6.80 \text{ m} \end{aligned}$$

Untuk perhitungan pusat massa selanjutnya dapat dilihat pada tabel 3.6 di lampiran.

### 3.4 Perhitungan Beban Gempa

#### 3.4.1 Perhitungan Waktu Getar Alami

$$- T = C_T \cdot H^{3/4}$$

Dimana:

$$C_T = 0.1 \cdot n \text{ (Method A dari UBC Section 1630.2.2)}$$

$$= 0.1 \times 4 = 0.4$$

$$H = \text{Tinggi Bangunan} = 13 \text{ m}$$

$$= 0.4 \cdot (13)^{3/4}$$

$$= 2.74 \text{ detik}$$

$$- T = \xi \cdot n$$

Dimana :

$$\xi = 0.17 \text{ (sesuai tabel 3.6)}$$

$$n = \text{Jumlah tingkat bangunan}$$

**Tabel 3.6 : Koefisien  $\zeta$  yang membatasi waktu getar alami  
Fundamental struktur gedung**

Wilayah Gempa	$\zeta$
1	0,20
2	0,19
3	0,18
4	0,17
5	0,16
6	0,15

$$T = \xi \cdot n$$

$$= 0,17 \cdot 4 = 0.68 \text{ detik}$$

Maka digunakan  $T = 0.68$  detik

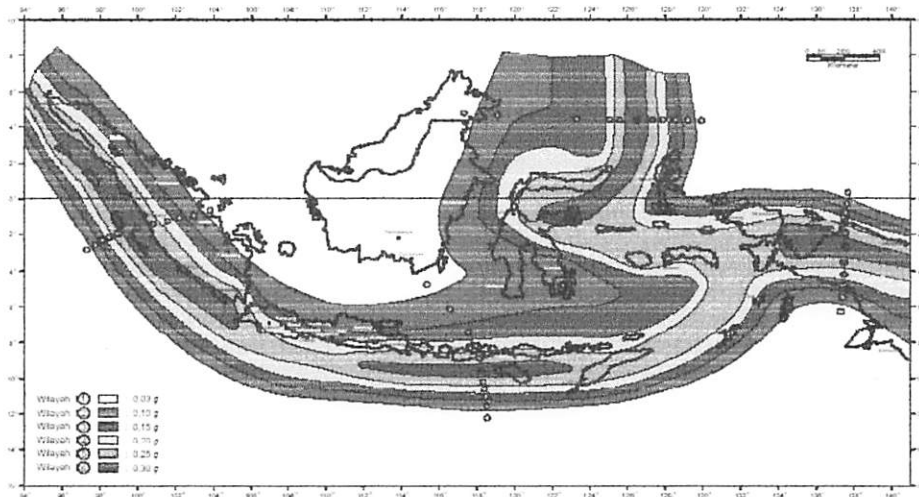
### 3.4.2 Koefisien Gempa Dasar

Berdasarkan gambar 3.1, wilayah gempa 4 pada tanah sedang di dapat

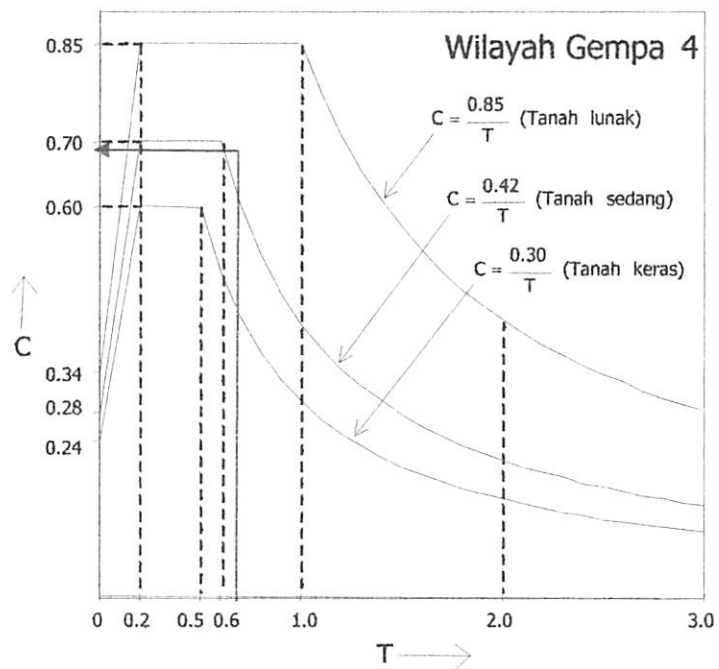
$$C = 0.42/T$$

$$C = 0.42/0.68$$

$$= 0.61$$



**Gambar 3.7 : Wilayah Gempa Indonesia Dengan Percepatan Puncak Batuan Dasar Dengan Periode Ulang 500 Tahun**



**Gambar 3.8 : Respon spectrum gempa rencana**

### 3.4.3 Faktor Keutamaan

Dari Tabel 3.7 didapat  $I = 1,0$  (Gedung umum seperti untuk perumahan, pemiagaan dan perkantoran).

**Tabel 3.7 : Faktor Keutamaan I untuk berbagai kategori gedung dan bangunan**

Kategori gedung	Faktor Keutamaan		
	I <sub>1</sub>	I <sub>2</sub>	I
Gedung umum seperti untuk penghunian, perniagaan dan perkantoran	1,0	1,0	1,0
Monumen dan bangunan monumental	1,0	1,6	1,6
Gedung penting pasca gempa seperti rumah sakit, instalasi air bersih, pembangkit tenaga listrik, pusat penyelamatan dalam keadaan darurat, fasilitas radio dan televisi.	1,4	1,0	1,4
Gedung untuk menyimpan bahan berbahaya seperti gas, produk minyak bumi, asam, bahan beracun.	1,6	1,0	1,6
Cerobong, tangki di atas menara	1,5	1,0	1,5

#### 3.4.4 Perhitungan Gaya Geser Dasar Horizontal Total Akibat Gempa

$$V = \frac{C \times I}{R} \times W_t$$

Dimana :

W<sub>t</sub> : Berat total Bangunan

R : SRMPK sesuai tabel 3.8 yang besarnya 8.5

**Tabel 3.8 : Faktor daktilitas maksimum, faktor reduksi gempa maksimum, faktor tahanan lebih struktur dan faktor tahanan lebih total beberapa jenis sistem dan subsistem struktur gedung**

Sistem dan sub sistem struktur gedung	Uraian sistem pemikul beban gempa	$\mu_m$	$R_m$ Pers. (6)	f Pers. (39)
1. Sistem dinding penumpu (Sistem struktur yang tidak memiliki rangka ruang pemikul beban gravitasi secara lengkap. Dinding penumpu atau sistem bresing memikul hampir semua beban gravitasi. Beban lateral dipikul dinding geser atau rangka bresing).	1.Dinding geser beton bertulang	2,7	4,5	2,8
	2.Dinding penumpu dengan rangka baja ringan dan bresing tarik	1,8	2,8	2,2
	3.Rangka bresing di mana bresingnya memikul beban gravitasi			
	a.Baja	2,8	4,4	2,2
	b.Beton bertulang (tidak untuk Wilayah 5 & 6)	1,8	2,8	2,2
2. Sistem rangka gedung (Sistem struktur yang pada dasarnya memiliki rangka ruang pemikul beban gravitasi secara lengkap. Beban lateral dipikul dinding geser atau rangka bresing).	1.Rangka bresing eksentris baja (RBE)	4,3	7,0	2,8
	2.Dinding geser beton bertulang	3,3	5,5	2,8
	3.Rangka bresing biasa			
	a.Baja	3,6	5,6	2,2
	b.Beton bertulang (tidak untuk Wilayah 5 & 6)	3,6	5,6	2,2
	4.Rangka bresing konsentrik khusus			
	a.Baja	4,1	6,4	2,2
	5.Dinding geser beton bertulang berangkai daktail	4,0	6,5	2,8
	6.Dinding geser beton bertulang kantilever daktail penuh	3,6	6,0	2,8
7.Dinding geser beton bertulang kantilever daktail parsial	3,3	5,5	2,8	
3. Sistem rangka pemikul momen (Sistem struktur yang pada dasarnya memiliki rangka ruang pemikul beban gravitasi secara lengkap. Beban lateral dipikul rangka pemikul momen terutama melalui mekanisme lentur)	1.Rangka pemikul momen khusus (SRPMK)			
	a.Baja	5,2	8,5	2,8
	b.Beton bertulang	5,2	8,5	2,8
	2.Rangka pemikul momen menengah beton (SRPMM)	3,3	5,5	2,8
	3.Rangka pemikul momen biasa (SRPMB)			
	a.Baja	2,7	4,5	2,8
	b.Beton bertulang	2,1	3,5	2,8
	4.Rangka batang baja pemikul momen khusus (SRBPMK)	4,0	6,5	2,8

4. Sistem ganda (Terdiri dari: 1. rangka ruang yang memikul seluruh beban gravitasi; 2. pemikul beban lateral berupa dinding geser atau rangka bresing dengan rangka pemikul momen. Rangka pemikul momen harus direncanakan secara terpisah mampu memikul sekurang-kurangnya 25% dari seluruh beban lateral; 3. kedua sistem harus direncanakan untuk memikul secara bersama-sama seluruh beban lateral dengan memperhatikan interaksi /sistem ganda)	1.Dinding geser			
	a.Beton bertulang dengan SRPMK beton bertulang	5,2	8,5	2,8
	b.Beton bertulang dengan SRPMB baja	2,6	4,2	2,8
	c.Beton bertulang dengan SRPMM beton bertulang	4,0	6,5	2,8
	2.RBE baja			
	a.Dengan SRPMK baja	5,2	8,5	2,8
	b.Dengan SRPMB baja	2,6	4,2	2,8
	3.Rangka bresing biasa			
	a.Baja dengan SRPMK baja	4,0	6,5	2,8
	b.Baja dengan SRPMB baja	2,6	4,2	2,8
	c.Beton bertulang dengan SRPMK beton bertulang (tidak untuk Wilayah 5 & 6)	4,0	6,5	2,8
	d.Beton bertulang dengan SRPMM beton bertulang (tidak untuk Wilayah 5 & 6)	2,6	4,2	2,8
	4.Rangka bresing konsentrik khusus			
	a.Baja dengan SRPMK baja	4,6	7,5	2,8
b.Baja dengan SRPMB baja	2,6	4,2	2,8	
5. Sistem struktur gedung kolom kantilever: (Sistem struktur yang memanfaatkan kolom kantilever untuk memikul beban lateral)	Sistem struktur kolom kantilever	1,4	2,2	2
6. Sistem interaksi dinding geser dengan rangka	Beton bertulang biasa (tidak untuk Wilayah 3, 4, 5 & 6)	3,4	5,5	2,8
7. Subsistem tunggal (Subsistem struktur bidang yang membentuk struktur gedung secara keseluruhan)	1.Rangka terbuka baja	5,2	8,5	2,8
	2.Rangka terbuka beton bertulang	5,2	8,5	2,8
	3. Rangka terbuka beton bertulang dengan balok beton pratekan (bergantung pada indeks baja total)	3,3	5,5	2,8
	4.Dinding geser beton bertulang berangkai daktail penuh.	4,0	6,5	2,8
	5.Dinding geser beton bertulang kantilever daktail parsial	3,3	5,5	2,8

$$V = \frac{C \times I}{R} \times Wt$$

$$V = \frac{0,62 \cdot 1,0}{8,5} \cdot 3332750,96$$

$$= 243094,78 \text{ Kg}$$

### 3.4.5 Perhitungan Gaya Gempa Tiap Lantai

$$F_i = \frac{W_i \times h_i}{\sum (W_i \times h_i)} \times V$$

$$F_i = \frac{(827597,48 \times 13,00)}{27260026,54} \times 234094,78$$

$$= 95942,68 \text{ Kg}$$

**Tabel 3.9 : Gaya Gempa Tiap Lantai**

Lantai ke -	hi (m)	Wi (Kgm)	Wi x hi (Kgm)	Fi X-Y (Kg)	Vi (Kg)
4	13.00	827597.48	10758767.24	95942.68	95942.68
3	9.80	827597.48	8116629.30	72381.08	168323.76
2	6.60	838463.00	553385.80	49348.87	217672.63
1	3.40	838463.00	2850774.20	25422.14	243094.78
Σ		3332750.96	27260026.54		



### 3.4.6 Perhitungan Gaya Gempa Tiap Pusat massa

$$F_i (\text{Tiap pusat massa}) = \frac{\text{Luas tipe pusat massa}}{\text{Luas total lantai}} \times F_i (\text{X-Y})$$

$$\text{Lantai 1 : Luas total} = 1104.62 \text{ m}^2$$

$$\text{Luas tipe pusat massa 1-4} = 154.42 \text{ m}^2$$

$$F_i (1 - 4) = \frac{154.42}{1104.62} \times 95942.68$$

$$= 13412.27 \text{ Kg}$$

**Tabel 3.10 : Gaya Gempa Tiap Tipe Pusat Massa**

Lantai ke-	Tipe pusat massa	$\frac{\text{(Luas tipe pusat massa)}}{\text{(Luas total lantai)}}$	Fi X-Y (Kg)	Fi X-Y Tiap pusat massa (Kg)	
				(100%)	(30%)
4	1-4	0.140	95942.68	13412.27	4023.68
	5-8	0.028		2659.17	797.75
	9-12	0.007		673.31	201.24
	13	0.059		5690.78	1707.24
3	1-4	0.140	72381.08	10118.48	3035.55
	5-8	0.028		2006.13	601.84
	9-12	0.007		507.95	152.39
	13	0.059		4293.24	1287.97
2	1-4	0.140	49348.87	6898.71	2069.61
	5-8	0.028		1367.77	401.33
	9-12	0.007		346.32	103.90
	13	0.059		2927.10	878.13
1	1-4	0.140	25422.14	3553.88	1066.16
	5-8	0.028		704.61	211.38
	9-12	0.007		178.41	53.52
	13	0.059		2141.58	642.47

## **BAB IV**

### **PERENCANAAN PONDASI**

#### **4.1 Data Perencanaan**

Dalam merencanakan suatu pondasi kita diperlukan suatu data perencanaan agar dalam merencanakannya didapat hasil yang maksimal. Berikut adalah data yang digunakan :

1. Tebal Poer : 60 cm
2. Kedalaman Poer : 1,5 m
3. Pengeboran pada kedalaman : 6,4 m
4. Mutu beton ( $f_c'$ ) : 25 Mpa = 250 kg/cm<sup>2</sup>
5. Mutu baja ( $f_y$ ) : 400 Mpa

#### **4.2 Perencanaan Pondasi Tiang Bor**

##### **4.2.1 Pengelompokan Beban Pada Pondasi**

Berdasarkan dari data output analisa pembebanan dengan program Staad Pro diambil tiga contoh tipe sebagai perencanaan pondasi dengan gaya – gaya yang bekerja pada masing – masing tipe dapat dilihat pada table dibawah ini :

**Tabel 4.1 : Pengelompokan Beban ( Reactiaon )**

<b>Reactions</b>	<b>Kolom (Node)</b>	<b>Gaya Vertikal (Kg)</b>	<b>Momen X (Kgm)</b>	<b>Momen Z (Kgm)</b>	<b>Kombinasi</b>
Berat	3752	306000	2040	1970	1.2D + 1.0L + 1.0EX
Sedang	3751	283000	2590	2030	1.2D + 1.0L + 1.0EX
Ringan	3750	57000	23,707	218,719	1.2D + 1.0L + 1.0EX

#### 4.2.2 Perencanaan Pondasi Tiang Bor Tipe 1 pada Join 3752

V : 306 ton ; Mx : 2,04 tm ; Mz : 1,97 tm

Diameter Tiang (D) = 50 cm = 0,50 m

Luas Tiang (Ap) =  $\frac{1}{4} \cdot \pi \cdot D^2$   
 $= \frac{1}{4} \cdot \pi \cdot 0,5^2 = 0,196 \text{ m}^2$

Keliling Tiang (p) =  $2 \cdot \pi \cdot D$   
 $= 2 \cdot \pi \cdot 0,5 = 3.14 \text{ m}$

Kedalaman Tiang (D<sub>f</sub>) = 4,9 m

Luas Selimut Tiang (As) = p . Kedalaman tiang (D<sub>f</sub>)



Pada kedalaman tersebut didapat nilai  $q_c$  :

1. 6,4  $\rightarrow q_c = 150 \text{ kg/cm}^2 \rightarrow$  ( nilai terakhir dari data sondir)

$$q_{c2} = 150 \text{ kg/cm}^2$$

$$Q_c = \frac{q_{c1} + q_{c2}}{2} = \frac{555 + 150}{2} = 105,833 \text{ kg/cm}^2$$

$$N_b = \frac{Q_c}{4} = \frac{105,833}{4} = 26,458 \sim 26 \text{ Pukulan/feet}$$

❖ Daya dukung berdasarkan kekuatan tanah

1. Untuk end bearing pile

$$\begin{aligned} Q_p &= 40 \cdot N_b \cdot A_p \\ &= 40 \cdot 26 \cdot 0,196 \\ &= 204,100 \text{ ton} \end{aligned}$$

Nilai  $q_{c \text{ rata-rata}}$  sepanjang tiang bor :

1. 1,5  $\rightarrow q_c = 20 \text{ kg/cm}^2$
2. 2,0  $\rightarrow q_c = 25 \text{ kg/cm}^2$
3. 2,4  $\rightarrow q_c = 45 \text{ kg/cm}^2$
4. 2,8  $\rightarrow q_c = 60 \text{ kg/cm}^2$
5. 3,2  $\rightarrow q_c = 70 \text{ kg/cm}^2$
6. 3,6  $\rightarrow q_c = 50 \text{ kg/cm}^2$
7. 4,0  $\rightarrow q_c = 40 \text{ kg/cm}^2$
8. 4,4  $\rightarrow q_c = 35 \text{ kg/cm}^2$
9. 4,8  $\rightarrow q_c = 25 \text{ kg/cm}^2$

$$\begin{aligned}
10. \quad 5,2 & \rightarrow qc = 35 \quad \text{kg/cm}^2 \\
11. \quad 5,6 & \rightarrow qc = 50 \quad \text{kg/cm}^2 \\
12. \quad 6,0 & \rightarrow qc = 90 \quad \text{kg/cm}^2 \\
13. \quad 6,4 & \rightarrow qc = 150 \quad \text{kg/cm}^2 + \\
& \Sigma qc = 695 \quad \text{kg/cm}^2
\end{aligned}$$

$$qc_{\text{rata-rata}} = \frac{qc}{13} = \frac{695}{13} = 53,46 \quad \text{kg/cm}^2$$

$$N_s = \frac{qc_{\text{rata-rata}}}{4} = \frac{53,46}{4} = 13,365 \sim 13 \text{ Pukulan/feet}$$

## 2. Untuk friction pile

$$\begin{aligned}
Q_s &= 0,1 \cdot N_s \cdot A_s \\
&= 0,1 \cdot 13 \cdot 15,386 \\
&= 20,002 \text{ ton}
\end{aligned}$$

### ➤ Untuk kombinasi end bearing pile dan friction pile

$$\begin{aligned}
Q_u &= Q_p + Q_s \\
&= 204,100 + 20,002 \\
&= 224,102 \text{ ton}
\end{aligned}$$

### ❖ Daya dukung berdasarkan kekuatan bahan

$$Q_d = \sigma_{\text{bahan}} \cdot A$$

Dimana :

$\sigma_{\text{bahan}}$  : Tegangan ijin bahan

A : Luas Penampang

1. Untuk dinding tiang (K-250)

$$\begin{aligned} Q_d &= (0,85 \cdot f_c') \times \left(\frac{1}{4} \cdot \pi \cdot D_{\text{tiang}}^2\right) \\ &= (0,85 \cdot 250) \times \left(\frac{1}{4} \cdot \pi \cdot 50^2\right) \\ &= 417031,25 \text{ kg} = 417,031 \text{ ton} \end{aligned}$$

➤ Daya dukung satu tiang yang diizinkan ( $Q_{\text{izin}}$ )

$$\begin{aligned} Q_{\text{izin}} &= \frac{Q_u}{SF} \\ &= \frac{224,102}{3} = 74,70060 \text{ ton} = 74700,60 \text{ kg} \end{aligned}$$

#### 4.2.2.1 Perencanaan Tiang Bor

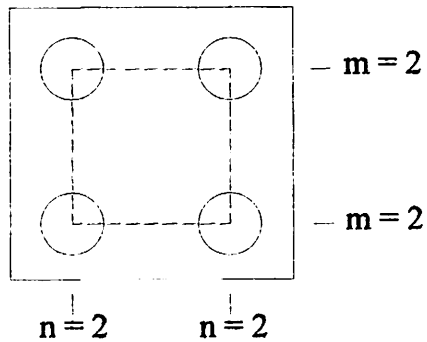
Jumlah tiang dalam satu poer ( $n$ )

$$\begin{aligned} n &= \frac{V}{Q_{\text{izin 1 tiang}}} \\ &= \frac{306}{74,70060} = 4,096 \end{aligned}$$

Dicoba 4 buah tiang dengan susunan :

$$m \text{ (jumlah baris tiang)} = 2$$

$$n \text{ (jumlah tiang dalam baris)} = 2$$



**Gambar 4.1 : Jumlah Tiang Bor Tipe 1**

- Syarat jarak antar tiang

$$S \leq \frac{(1,57 \times D \times m \times n) - 2D}{(m+n) - 2}$$

$$S \leq \frac{(1,57 \times 0,5 \times 2 \times 2) - 2 \times 0,5}{(2+2) - 2}$$

$$S \leq 1,08$$

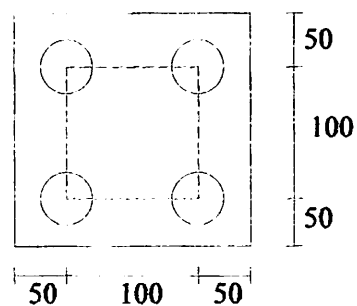
- Kontrol jarak antar tiang

$$2,5D \leq S \leq 3D$$

$$2,5(0,5) \leq S \leq 3(0,5)$$

$$1,25 \leq S \leq 1,50$$

Dipakai jarak antar tiang (s) = 1,00 m



**Gambar 4.2 : Jarak Antar Tiang Bor Tipe 1**



Dengan demikian perencanaan pondasi tiang bor tipe 1 dapat dihitung dengan efisiensi kelompok tiang.

### Efisiensi kelompok tiang

#### 1. Berdasarkan Rumus Los Angles

$$\begin{aligned}\eta &= 1 - \frac{D}{\pi \cdot s \cdot m \cdot n} [(m) \cdot (n - 1) + (n) \cdot (m - 1) + (m - 1) \cdot (n - 1) \cdot \sqrt{2}] \\ &= 1 - \frac{0,5}{3,14 \cdot 1 \cdot 2 \cdot 2} [(2) \cdot (2 - 1) + (2) \cdot (2 - 1) + (2 - 1) \cdot (2 - 1) \cdot \sqrt{2}] \\ &= 0,784 < 1 \quad \text{OK}\end{aligned}$$

#### 2. Berdasarkan Rumus Converse - Labarre

$$\begin{aligned}\eta &= 1 - \frac{(n-1) \cdot m + (m-1) \cdot n}{90 \cdot m \cdot n} \times \theta & \Rightarrow \theta &= \arctan \frac{D}{s} \\ & & &= \arctan \frac{0,5}{1,0} = 26,565^\circ \\ &= 1 - \frac{(2-1) \cdot 2 + (2-1) \cdot 2}{90 \cdot 2 \cdot 2} \times 26,565^\circ \\ &= 0,705 < 1 \quad \text{OK}\end{aligned}$$

#### 3. Berdasarkan Rumus Seiler - Keeney

$$\begin{aligned}\eta &= 1 - \frac{(36 \cdot s) \cdot (m+n-2)}{(75 \cdot s^2 - 7) \cdot (m+n-1)} + \frac{0,3}{(m+n)} \\ &= 1 - \frac{(36 \cdot 1) \cdot (2+2-2)}{(75 \cdot 1^2 - 7) \cdot (2+2-1)} + \frac{0,3}{(2+2)} \\ &= 0,722 < 1 \quad \text{OK}\end{aligned}$$

Berdasarkan dari perhitungan nilai efisiensi, diambil nilai  $\eta$  yang terkecil yaitu : 0,705 yang digunakan untuk perhitungan selanjutnya.

❖ Daya dukung tiang kelompok

$$Q_{pg} = \eta \times n \times Q_{izin}$$

$$= 0,705 \times 4 \times 74,70060$$

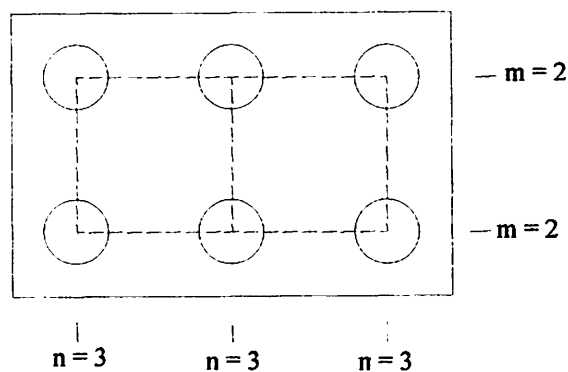
$$= 210,606 \text{ ton} < V = 306 \text{ ton (Tidak OK)}, \text{ Syarat } Q_{pg} > V$$

Karena daya dukung tiang kelompok tidak memenuhi dengan susunan  $m = 2$  dan  $n = 2$ , maka dicoba lagi dengan susunan tiang  $m = 2$  dan  $n = 3$ .

Dicoba 6 buah tiang dengan susunan :

$$m \text{ (jumlah baris tiang)} = 2$$

$$n \text{ (jumlah tiang dalam baris)} = 3$$



**Gambar 4.3 : Jumlah Tiang Bor Tipe 1**

➤ Syarat jarak antar tiang

$$S \leq \frac{(1,57 \times D \times m \times n) - 2D}{(m+n) - 2}$$

$$S \leq \frac{(1,57 \times 0,5 \times 2 \times 3) - (2 \times 0,5)}{(2+3) - 2}$$

$$S \leq 1,25$$

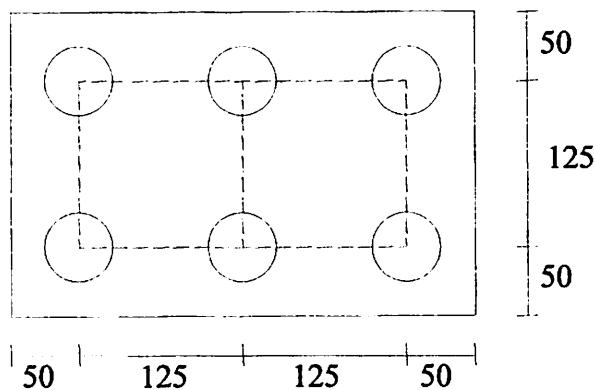
➤ Kontrol jarak antar tiang

$$2,5D \leq S \leq 3D$$

$$2,5(0,5) \leq S \leq 3(0,5)$$

$$1,25 \leq S \leq 1,50$$

Dipakai jarak antar tiang (s) = 1,25 m



**Gambar 4.4 : Jarak Antar Tiang Bor Tipe 1**

Dengan demikian perencanaan pondasi tiang bor tipe 1 dapat dihitung dengan efisiensi kelompok tiang.

Efisiensi kelompok tiang

1. Berdasarkan Rumus Los Angles

$$\begin{aligned} \eta &= 1 - \frac{D}{\pi \cdot s \cdot m \cdot n} [(m) \cdot (n - 1) + (n) \cdot (m - 1) + (m - 1) \cdot (n - 1) \cdot \sqrt{2}] \\ &= 1 - \frac{0,5}{3,14 \cdot 1,25 \cdot 2 \cdot 3} [(2) \cdot (3 - 1) + (3) \cdot (2 - 1) + (2 - 1) \cdot (3 - 1) \cdot \sqrt{2}] \\ &= 0,791 < 1 \quad \text{OK} \end{aligned}$$

2. Berdasarkan Rumus Converse - Labarre

$$\begin{aligned}\eta &= 1 - \frac{(n-1).m+(m-1).n}{90.m.n} \times \theta && \Rightarrow \theta = \arctan \frac{D}{S} \\ & && = \arctan \frac{0,5}{1,25} = 21,801^\circ \\ &= 1 - \frac{(3-1).2+(2-1).3}{90.2.3} \times 21,801^\circ \\ &= 0,717 < 1 \quad \text{OK}\end{aligned}$$

3. Berdasarkan Rumus Seiler – Keeney

$$\begin{aligned}\eta &= 1 - \frac{(36.s).(m+n-2)}{(75.s^2.-7).(m+n-1)} + \frac{0,3}{(m+n)} \\ \eta &= 1 - \frac{(36.1,25).(2+3-2)}{(75.1,25^2.-7).(2+3-1)} + \frac{0,3}{(2+3)} \\ &= 0,754 < 1 \quad \text{OK}\end{aligned}$$

Berdasarkan dari perhitungan nilai efisiensi, diambil nilai  $\eta$  yang terkecil yaitu : 0,717 yang digunakan untuk perhitungan selanjutnya.

❖ Daya dukung tiang kelompok

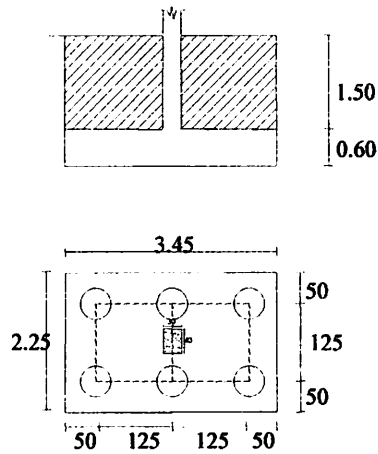
$$Q_{pg} = \eta \times n \times Q_{izin}$$

$$= 0,717 \times 6 \times 74,70060$$

$$= 321,539 \text{ ton} > V = 306 \text{ ton} \quad (\text{OK}), \text{ Syarat } Q_{pg} > V$$

❖ Kontrol  $\Sigma V$  dimana,  $\Sigma V = ( V + \text{berat poer} ) < Q_{pg}$

Perhitungan beban poer



**Gambar 4.5 : Perencanaan Poer**

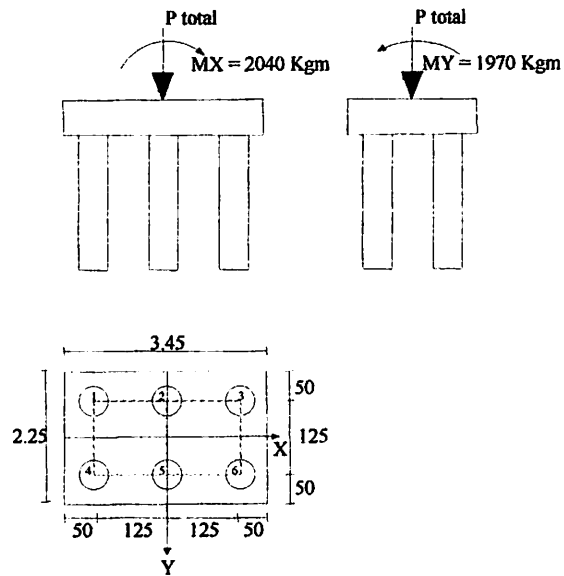
$$\begin{aligned} \text{Pouer} &= [(3,45 \times 2,25) + (0,3 \times 0,4 \times 1,5)] \times 2400 \\ &= 439,763 \text{ kg} \end{aligned}$$

$$\begin{aligned} \text{Tanah urug} &= [(3,45 \times 2,25 \times 0,6) - (0,3 \times 0,4 \times 1,5)] \times 1700 \\ &= 7611,750 \text{ kg} \end{aligned}$$

$$V = 306000 \text{ kg}$$

$$\begin{aligned} \Sigma V &= \text{Berat sendiri poer} + \text{Berat tanah urug} + V \\ &= 439,763 + 7611,750 + 306000 \\ &= 314051,513 \text{ kg} \\ &= 314,052 \text{ ton} < Q_{pg} = 321,539 \text{ ton} \quad (\text{OK}) \end{aligned}$$

#### 4.2.2.2 Perhitungan Beban Yang Diterima Oleh Pondasi Tiang Bor



Gambar 4.6 : Poadasi Tiang Bor yang Menerima Beban V dan M

Mencari beban tiang maximum

$$P_{max} = \frac{\Sigma V}{n} \pm \frac{M_y \cdot X_{max}}{n_y \cdot \Sigma X^2} \pm \frac{M_x \cdot Y_{max}}{n_x \cdot \Sigma Y^2}$$

Dimana :

$$\Sigma V = 314051,51 \text{ kg} \quad ; M_x = 2040 \text{ kgm}$$

$$n = 6 \text{ tiang} \quad ; M_y = 1970 \text{ kgm}$$

$$n_y = 2 \text{ tiang}$$

$$n_x = 3 \text{ tiang}$$

$$\Sigma X^2 = \text{Jumlah kuadrat absis tiang}$$

$$= (-1,25^2) + (0^2) + (1,25^2) = 3,13 \text{ m}^2$$

$\Sigma Y^2$  = Jumlah kuadrat ordinat tiang

$$= (-0,625^2) + (0,625^2) = 0,78 \text{ m}^2$$

Sehingga :

$$(x_1 = -1,25 ; y_1 = 0,625)$$

$$\begin{aligned} P1 &= \frac{314051,51}{6} \pm \frac{1970 \times (-1,25)}{2 \times 3,13} \pm \frac{2040 \times (0,625)}{3 \times 0,78} \\ &= 52491,92 \text{ kg} \end{aligned}$$

$$(x_2 = 0 ; y_1 = 0,625)$$

$$\begin{aligned} P2 &= \frac{314051,51}{6} \pm \frac{1970 \times (0)}{2 \times 3,13} \pm \frac{2040 \times (0,625)}{3 \times 0,78} \\ &= 52885,92 \text{ kg} \end{aligned}$$

$$(x_3 = 1,25 ; y_3 = 0,625)$$

$$\begin{aligned} P3 &= \frac{314051,51}{6} \pm \frac{1970 \times (1,25)}{2 \times 3,13} \pm \frac{2040 \times (0,625)}{3 \times 0,78} \\ &= 53279,92 \text{ kg} \end{aligned}$$

$$(x_4 = -1,25 ; y_1 = -0,625)$$

$$\begin{aligned} P4 &= \frac{314051,51}{6} \pm \frac{1970 \times (-1,25)}{2 \times 3,13} \pm \frac{2040 \times (-0,625)}{3 \times 0,78} \\ &= 51403,92 \text{ kg} \end{aligned}$$

$$(x_5 = 0 ; y_1 = - 0,625)$$

$$P_5 = \frac{314051,51}{6} \pm \frac{1970 \times (0)}{2 \times 3,13} \pm \frac{2040 \times (-0,625)}{3 \times 0,78}$$

$$= 51797,92 \text{ kg}$$

$$(x_6 = 1,25 ; y_6 = - 0,625)$$

$$P_6 = \frac{314051,51}{6} \pm \frac{1970 \times (1,25)}{2 \times 3,13} \pm \frac{2040 \times (-0,625)}{3 \times 0,78}$$

$$= 52191,92 \text{ kg}$$

$$P_{\max} = 53279.92 \text{ kg} < Q_1 \text{ tiang} = 74700,60 \text{ kg} \dots\dots\dots(\text{aman})$$

Jadi dapat digunakan pondasi tiang bor pada kedalaman 6,4 m dengan diameter 50 cm.

#### 4.2.3 Perencanaan Pondasi Tiang Bor Tipe 2 pada Join 3751

$$V : 283 \text{ ton} ; M_x : 2,59 \text{ tm} ; M_z : 2,03 \text{ tm}$$

$$\text{Diameter Tiang (D)} = 50 \text{ cm} = 0,50 \text{ m}$$

$$\text{Luas Tiang (A}_p) = \frac{1}{4} \cdot \pi \cdot D^2$$

$$= \frac{1}{4} \cdot \pi \cdot 0,5^2 = 0,196 \text{ m}^2$$

$$\text{Keliling Tiang (p)} = 2 \cdot \pi \cdot D$$

$$= 2 \cdot \pi \cdot 0,5 = 3.14 \text{ m}$$



$$\text{Kedalaman Tiang (D}_f\text{)} = 4,9 \text{ m}$$

$$\begin{aligned}\text{Luas Selimut Tiang (A}_s\text{)} &= p \cdot \text{Kedalaman tiang (D}_f\text{)} \\ &= 3,14 \cdot 4,9 = 15,386 \text{ m}^2\end{aligned}$$

$$\text{Angka Keamanan (SF)} = 3$$

$$\text{Data Sondir} = \text{Titik S}_2$$

Nilai  $N_{SPT}$  disekitar dasar tiang ( $N_b$ ) dihitung rata-rata antara 8D di atas dasar tiang dan 4D di bawah dasar tiang. (Rahardjo,P.P,2007, Manual Pondasi Tiang Edisi 3 Hal : 42).

$$6,4 - 8D = 6,4 - (8 \cdot 0,5) = 2,4 \text{ m}$$

Pada kedalaman tersebut didapat nilai  $q_c$  :

1.	2,4	→ $q_c = 45$	$\text{kg/cm}^2$
2.	3,0	→ $q_c = 65$	$\text{kg/cm}^2$
3.	3,4	→ $q_c = 65$	$\text{kg/cm}^2$
4.	4,0	→ $q_c = 40$	$\text{kg/cm}^2$
5.	4,4	→ $q_c = 35$	$\text{kg/cm}^2$
6.	5,0	→ $q_c = 30$	$\text{kg/cm}^2$
7.	5,4	→ $q_c = 35$	$\text{kg/cm}^2$
8.	6,0	→ $q_c = 90$	$\text{kg/cm}^2$
9.	6,4	→ $q_c = 150$	$\text{kg/cm}^2 +$
		<u>Σ <math>q_c = 555</math></u>	$\text{kg/cm}^2$

$$q_{c1} = \frac{\Sigma q_{c1}}{9} = \frac{555}{9} = 61,67 \text{ kg/cm}^2$$

$$6,4 + 4D = 6,4 + (4 \cdot 0,5) = 8,4 \text{ m}$$

Pada kedalaman tersebut didapat nilai  $q_c$  :

1. 6,4 →  $q_c = 150 \text{ kg/cm}^2$  → ( nilai terakhir dari data sondir)

$$q_{c2} = 150 \text{ kg/cm}^2$$

$$Q_c = \frac{q_{c1} + q_{c2}}{2} = \frac{555 + 150}{2} = 105,833 \text{ kg/cm}^2$$

$$N_b = \frac{Q_c}{4} = \frac{105,833}{4} = 26,458 \sim 26 \text{ Pukulan/feet}$$

❖ Daya dukung berdasarkan kekuatan tanah

1. Untuk end bearing pile

$$\begin{aligned} Q_p &= 40 \cdot N_b \cdot A_p \\ &= 40 \cdot 26 \cdot 0,196 \\ &= 204,100 \text{ ton} \end{aligned}$$

Nilai  $q_{c \text{ rata-rata}}$  sepanjang tiang bor :

1. 1,5 →  $q_c = 20 \text{ kg/cm}^2$
2. 2,0 →  $q_c = 25 \text{ kg/cm}^2$
3. 2,4 →  $q_c = 45 \text{ kg/cm}^2$
4. 2,8 →  $q_c = 60 \text{ kg/cm}^2$
5. 3,2 →  $q_c = 70 \text{ kg/cm}^2$
6. 3,6 →  $q_c = 50 \text{ kg/cm}^2$

$$\begin{array}{ll}
7. & 4,0 \quad \rightarrow qc = 40 \quad \text{kg/cm}^2 \\
8. & 4,4 \quad \rightarrow qc = 35 \quad \text{kg/cm}^2 \\
9. & 4,8 \quad \rightarrow qc = 25 \quad \text{kg/cm}^2 \\
10. & 5,2 \quad \rightarrow qc = 35 \quad \text{kg/cm}^2 \\
11. & 5,6 \quad \rightarrow qc = 50 \quad \text{kg/cm}^2 \\
12. & 6,0 \quad \rightarrow qc = 90 \quad \text{kg/cm}^2 \\
13. & 6,4 \quad \rightarrow qc = 150 \quad \text{kg/cm}^2 + \\
& \hline
& \Sigma qc = 695 \quad \text{kg/cm}^2
\end{array}$$

$$qc_{\text{rata-rata}} = \frac{qc}{13} = \frac{695}{13} = 53,46 \text{ kg/cm}^2$$

$$N_s = \frac{qc_{\text{rata-rata}}}{4} = \frac{53,46}{4} = 13,365 \sim 13 \frac{\text{Pukulan}}{\text{feet}}$$

## 2. Untuk end friction pile

$$\begin{aligned}
Q_s &= 0,1 \cdot N_s \cdot A_s \\
&= 0,1 \cdot 13 \cdot 15,386 \\
&= 20,002 \text{ ton}
\end{aligned}$$

### ➤ Untuk kombinasi end bearing pile dan friction pile

$$\begin{aligned}
Q_u &= Q_p + Q_s \\
&= 204,100 + 20,002 \\
&= 224,102 \text{ ton}
\end{aligned}$$

### ❖ Daya dukung berdasarkan kekuatan bahan

$$Q_d = \sigma_{\text{bahan}} \cdot A$$

Dimana :

$\sigma_{\text{bahan}}$  : Tegangan ijin bahan

A : Luas Penampang

1. Untuk dinding tiang (K-250)

$$\begin{aligned} Q_d &= (0,85 \cdot fc') \times \left( \frac{1}{4} \cdot \pi \cdot D_{\text{tiang}}^2 \right) \\ &= (0,85 \cdot 250) \times \left( \frac{1}{4} \cdot \pi \cdot 50^2 \right) \\ &= 417031,25 \text{ kg} = 417,031 \text{ ton} \end{aligned}$$

➤ Daya dukung satu tiang yang diizinkan ( $Q_{\text{izin}}$ )

$$\begin{aligned} Q_{\text{izin}} &= \frac{Q_u}{SF} \\ &= \frac{224,102}{3} = 74,70060 \text{ ton} = 74700,60 \text{ kg} \end{aligned}$$

#### 4.2.3.1 Perencanaan Tiang Bor

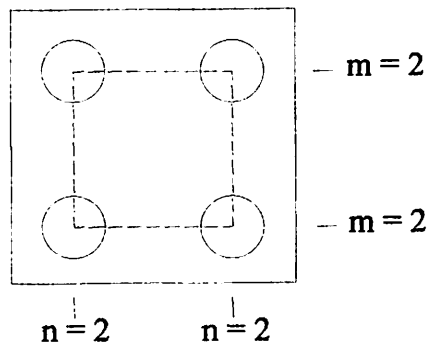
Jumlah tiang dalam satu poer (n)

$$\begin{aligned} n &= \frac{V}{Q_{\text{izin 1 tiang}}} \\ &= \frac{283}{74,70060} = 3,788 \end{aligned}$$

Dicoba 4 buah tiang dengan susunan :

$$m \text{ (jumlah baris tiang)} = 2$$

$$n \text{ (jumlah tiang dalam baris)} = 2$$



**Gambar 4.7 : Jumlah Tiang Bor Tipe 2**

- Syarat jarak antar tiang

$$S \leq \frac{(1,57 \times D \times m \times n) - 2D}{(m+n) - 2}$$

$$S \leq \frac{(1,57 \times 0,5 \times 2 \times 2) - 2 \times 0,5}{(2+2) - 2}$$

$$S \leq 1,08$$

- Kontrol jarak antar tiang

$$2,5D \leq S \leq 3D$$

$$2,5(0,5) \leq S \leq 3(0,5)$$

$$1,25 \leq S \leq 1,50$$

Dipakai jarak antar tiang (s) = 1,00 m

Dengan demikian perencanaan pondasi tiang bor tipe 2 dapat dihitung dengan efisiensi kelompok tiang.

Efisiensi kelompok tiang

1. Berdasarkan Rumus Los Angles

$$\eta = 1 - \frac{D}{\pi \cdot s \cdot m \cdot n} [(m) \cdot (n - 1) + (n) \cdot (m - 1) + (m - 1) \cdot (n - 1) \cdot \sqrt{2}]$$

$$= 1 - \frac{0,5}{3,14 \cdot 1 \cdot 2 \cdot 2} [(2) \cdot (2 - 1) + (2) \cdot (2 - 1) + (2 - 1) \cdot (2 - 1) \cdot \sqrt{2}]$$

$$= 0,784 < 1 \quad \text{OK}$$

## 2. Berdasarkan Rumus Converse - Labarre

$$\eta = 1 - \frac{(n-1) \cdot m + (m-1) \cdot n}{90 \cdot m \cdot n} \times \theta \qquad \Rightarrow \theta = \arctan \frac{D}{S}$$

$$= \arctan \frac{0,5}{1,0} = 26,565^\circ$$

$$= 1 - \frac{(2-1) \cdot 2 + (2-1) \cdot 2}{90 \cdot 2 \cdot 2} \times 26,565^\circ$$

$$= 0,705 < 1 \quad \text{OK}$$

## 3. Berdasarkan Rumus Seiler – Keeney

$$\eta = 1 - \frac{(36 \cdot s) \cdot (m+n-2)}{(75 \cdot s^2 \cdot -7) \cdot (m+n-1)} + \frac{0,3}{(m+n)}$$

$$= 1 - \frac{(36 \cdot 1) \cdot (2+2-2)}{(75 \cdot 1^2 \cdot -7) \cdot (2+2-1)} + \frac{0,3}{(2+2)}$$

$$= 0,722 < 1 \quad \text{OK}$$

Berdasarkan dari perhitungan nilai efisiensi, diambil nilai  $\eta$  yang terkecil yaitu : 0,705 yang digunakan untuk perhitungan selanjutnya.

❖ Daya dukung tiang kelompok

$$Q_{pg} = \eta \times n \times Q_{izin}$$

$$= 0,705 \times 4 \times 74,70060$$

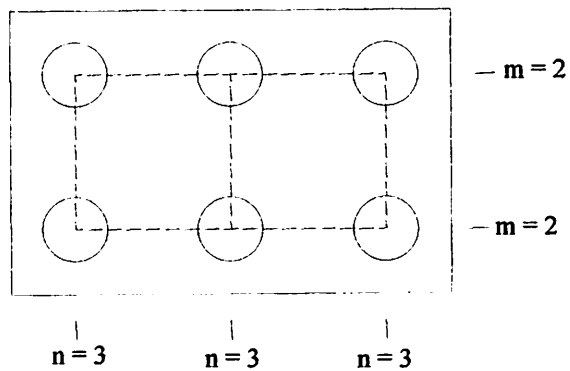
$$= 210,606 \text{ ton} < V = 283 \text{ ton ( Tidak OK ), Syarat } Q_{pg} > V$$

Karena daya dukung tiang kelompok tidak memenuhi dengan susunan  $m = 2$  dan  $n = 2$ , maka dicoba lagi dengan susunan tiang  $m = 2$  dan  $n = 3$ .

Dicoba 6 buah tiang dengan susunan :

$$m \text{ (jumlah baris tiang)} = 2$$

$$n \text{ (jumlah tiang dalam baris)} = 3$$



**Gambar 4.8 : Jumlah Tiang Bor Tipe 2**

➤ Syarat jarak antar tiang

$$S \leq \frac{(1,57 \times D \times m \times n) - 2D}{(m+n) - 2}$$

$$S \leq \frac{(1,57 \times 0,5 \times 2 \times 3) - (2 \times 0,5)}{(2+3) - 2}$$

$$S \leq 1,25$$

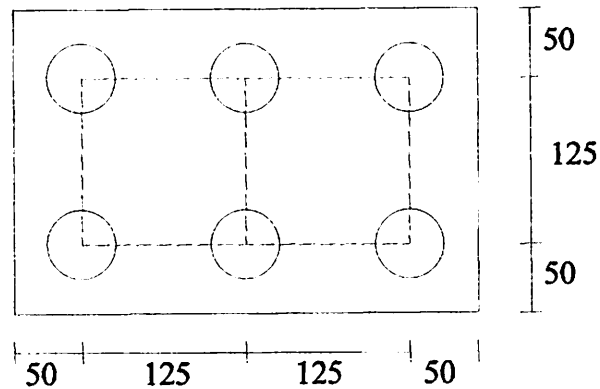
➤ Kontrol jarak antar tiang

$$2,5D \leq S \leq 3D$$

$$2,5(0,5) \leq S \leq 3(0,5)$$

$$1,25 \leq S \leq 1,50$$

Dipakai jarak antar tiang (s) = 1,25 m



**Gambar 4.9 : Jarak Antar Tiang Bor Tipe 2**

Dengan demikian perencanaan pondasi tiang bor tipe 2 dapat dihitung dengan efisiensi kelompok tiang.

Efisiensi kelompok tiang

1. Berdasarkan Rumus Los Angles

$$\eta = 1 - \frac{D}{\pi \cdot s \cdot m \cdot n} [(m) \cdot (n - 1) + (n) \cdot (m - 1) + (m - 1) \cdot (n - 1) \cdot \sqrt{2}]$$

$$= 1 - \frac{0,5}{3,14 \cdot 1,25 \cdot 2 \cdot 3} [(2) \cdot (3 - 1) + (3) \cdot (2 - 1) + (2 - 1) \cdot (3 - 1) \cdot \sqrt{2}]$$

$$= 0,791 < 1 \quad \text{OK}$$

2. Berdasarkan Rumus Converse - Labarre

$$\eta = 1 - \frac{(n-1)m + (m-1)n}{90 \cdot m \cdot n} \times \theta \quad \Rightarrow \theta = \arctan \frac{D}{s}$$

$$= \arctan \frac{0,5}{1,25} = 21,801^\circ$$

$$= 1 - \frac{(3-1)2 + (2-1)3}{90 \cdot 2 \cdot 3} \times 21,801^\circ$$



$$= 0,717 < 1 \quad \text{OK}$$

3. Berdasarkan Rumus Seiler – Keeney

$$\eta = 1 - \frac{(36 \cdot s) \cdot (m+n-2)}{(75 \cdot s^2 \cdot -7) \cdot (m+n-1)} + \frac{0,3}{(m+n)}$$

$$\eta = 1 - \frac{(36 \cdot 1,25) \cdot (2+3-2)}{(75 \cdot 1,25^2 \cdot -7) \cdot (2+3-1)} + \frac{0,3}{(2+3)}$$

$$= 0,754 < 1 \quad \text{OK}$$

Berdasarkan dari perhitungan nilai efisiensi, diambil nilai  $\eta$  yang terkecil yaitu : 0,717 yang digunakan untuk perhitungan selanjutnya.

❖ Daya dukung tiang kelompok

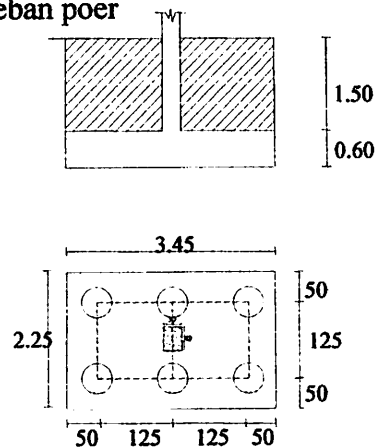
$$Q_{pg} = \eta \times n \times Q_{izin}$$

$$= 0,717 \times 6 \times 74,70060$$

$$= 321,539 \text{ ton} > V = 283 \text{ ton ( OK ), Syarat } Q_{pg} > V$$

❖ Kontrol  $\Sigma V$  dimana,  $\Sigma V = ( V + \text{berat poer} ) < Q_{pg}$

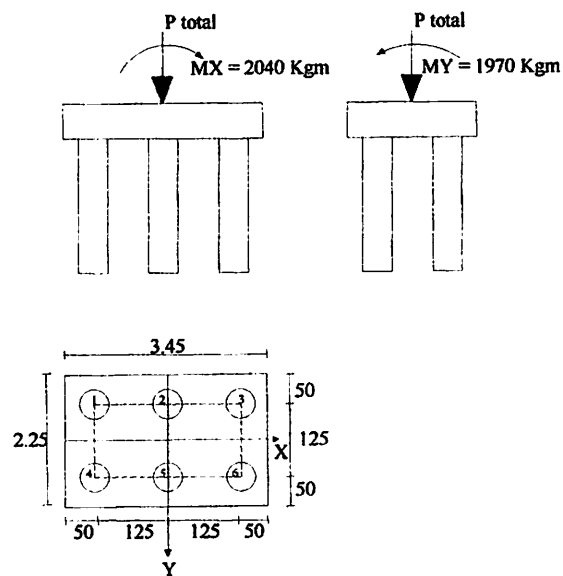
Perhitungan beban poer



Gambar 4.10 : Perencanaan Poer

$$\begin{aligned}
 \text{Poer} &= [(3,45 \times 2,25) + (0,3 \times 0,4 \times 1,5)] \times 2400 \\
 &= 439,763 \text{ kg} \\
 \text{Tanah urug} &= [(3,45 \times 2,25 \times 0,6) - (0,3 \times 0,4 \times 1,5)] \times 1700 \\
 &= 7611,750 \text{ kg} \\
 V &= 283000 \text{ kg} \\
 \Sigma V &= \text{Berat sendiri poer} + \text{Berat tanah urug} + V \\
 &= 439,763 + 7611,750 + 283000 \\
 &= 283873,315 \text{ kg} \\
 &= 283,873 \text{ ton} < Q_{pg} = 321,539 \text{ ton} \quad (\text{OK})
 \end{aligned}$$

#### 4.2.3.2 Perhitungan Beban Yang Diterima Oleh Pondasi Tiang Bor



**Gambar 4.11 : Pondasi Tiang Bor yang Menerima Beban V dan M**

Mencari beban tiang maximum

$$P_{\max} = \frac{\Sigma V}{n} \pm \frac{M_y \cdot X_{\max}}{n_y \cdot \Sigma X^2} \pm \frac{M_x \cdot Y_{\max}}{n_x \cdot \Sigma Y^2}$$

Dimana :

$$\Sigma V = 283873,315 \text{ kg} \quad ; M_x = 2590 \text{ kgm}$$

$$n = 6 \text{ tiang} \quad ; M_y = 2030 \text{ kgm}$$

$$n_y = 2 \text{ tiang}$$

$$n_x = 3 \text{ tiang}$$

$$\Sigma X^2 = \text{Jumlah kuadrat absis tiang}$$

$$= (-1,25^2) + (0^2) + (1,25^2) = 3,13 \text{ m}^2$$

$$\Sigma Y^2 = \text{Jumlah kuadrat ordinat tiang}$$

$$= (-0,625^2) + (0,625^2) = 0,78 \text{ m}^2$$

Sehingga :

$$(x_1 = -1,25 ; y_1 = 0,625)$$

$$P1 = \frac{283873,32}{6} \pm \frac{1970 \times (-1,25)}{2 \times 3,13} \pm \frac{2040 \times (0,625)}{3 \times 0,78}$$

$$= 47596,866 \text{ kg}$$

$$(x_2 = 0 ; y_1 = 0,625)$$

$$P2 = \frac{283873,32}{6} \pm \frac{1970 \times (0)}{2 \times 3,13} \pm \frac{2040 \times (0,625)}{3 \times 0,78}$$

$$= 48319\,386 \text{ kg}$$

$$(x_3 = 1,25 ; y_3 = 0,625)$$

$$P3 = \frac{283873,32}{6} \pm \frac{1970 \times (1,25)}{2 \times 3,13} \pm \frac{2040 \times (0,625)}{3 \times 0,78}$$

$$= 48797,219 \text{ kg}$$

$$(x_4 = -1,25 ; y_4 = -0,625)$$

$$P4 = \frac{283873,32}{6} \pm \frac{1970 \times (-1,25)}{2 \times 3,13} \pm \frac{2040 \times (-0,625)}{3 \times 0,78}$$

$$= 45875,386 \text{ kg}$$

$$(x_5 = 0 ; y_5 = -0,625)$$

$$P5 = \frac{283873,32}{6} \pm \frac{1970 \times (0)}{2 \times 3,13} \pm \frac{2040 \times (-0,625)}{3 \times 0,78}$$

$$= 46233,053 \text{ kg}$$

$$(x_6 = 1,25 ; y_6 = -0,625)$$

$$P6 = \frac{283873,32}{6} \pm \frac{1970 \times (1,25)}{2 \times 3,13} \pm \frac{2040 \times (-0,625)}{3 \times 0,78}$$

$$= 46639,053 \text{ kg}$$

$$P_{\max} = 48797,386 \text{ kg} < Q_1 \text{ tiang} = 74700,60 \text{ kg} \dots\dots\dots(\text{aman})$$

Jadi dapat digunakan pondasi tiang bor pada kedalaman 6,4 m dengan diameter 50 cm.

#### 4.2.4 Perencanaan Pondasi Tiang Bor Tipe 3 pada Join 3750

$$V : 57 \text{ ton} ; M_x : 0,02371 \text{ tm} ; M_z : 0,21872 \text{ tm}$$

$$\text{Diameter Tiang (D)} = 50 \text{ cm} = 0,50 \text{ m}$$

$$\begin{aligned} \text{Luas Tiang (A}_p) &= \frac{1}{4} \cdot \pi \cdot D^2 \\ &= \frac{1}{4} \cdot \pi \cdot 0,5^2 = 0,196 \text{ m}^2 \end{aligned}$$

$$\begin{aligned} \text{Keliling Tiang (p)} &= 2 \cdot \pi \cdot D \\ &= 2 \cdot \pi \cdot 0,5 = 3,14 \text{ m} \end{aligned}$$

$$\text{Kedalaman Tiang (D}_f) = 4,9 \text{ m}$$

$$\begin{aligned} \text{Luas Selimut Tiang (A}_s) &= p \cdot \text{Kedalaman tiang (D}_f) \\ &= 3,14 \cdot 4,9 = 15,386 \text{ m}^2 \end{aligned}$$

$$\text{Angka Keamanan (SF)} = 3$$

$$\text{Data Sondir} = \text{Titik S}_2$$

Nilai  $N_{SPT}$  disekitar dasar tiang ( $N_b$ ) dihitung rata-rata antara  $8D$  di atas dasar tiang dan  $4D$  di bawah dasar tiang. (Rahardjo,P.P,2007, Manual Pondasi Tiang Edisi 3 Hal : 42).

$$6,4 - 8D = 6,4 - (8 \cdot 0,5) = 2,4 \text{ m}$$

Pada kedalaman tersebut didapat nilai  $q_c$  :

$$1. \quad 2,4 \rightarrow q_c = 45 \text{ kg/cm}^2$$

2. 3,0 → qc = 65 kg/cm<sup>2</sup>
3. 3,4 → qc = 65 kg/cm<sup>2</sup>
4. 4,0 → qc = 40 kg/cm<sup>2</sup>
5. 4,4 → qc = 35 kg/cm<sup>2</sup>
6. 5,0 → qc = 30 kg/cm<sup>2</sup>
7. 5,4 → qc = 35 kg/cm<sup>2</sup>
8. 6,0 → qc = 90 kg/cm<sup>2</sup>
9. 6,4 → qc = 150 kg/cm<sup>2</sup> +  


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Σ qc = 555 kg/cm<sup>2</sup>

$$qc_1 = \frac{\Sigma qc_1}{9} = \frac{555}{9} = 61,67 \text{ kg/cm}^2$$

$$6,4 + 4D = 6,4 + (4 \cdot 0,5) = 8,4 \text{ m}$$

Pada kedalaman tersebut didapat nilai qc :

1. 6,4 → qc = 150 kg/cm<sup>2</sup> → ( nilai terakhir dari data sondir)

$$qc_2 = 150 \text{ kg/cm}^2$$

$$Q_c = \frac{qc_1 + qc_2}{2} = \frac{555 + 150}{2} = 105,833 \text{ kg/cm}^2$$

$$N_b = \frac{Q_c}{4} = \frac{105,833}{4} = 26,458 \sim 26 \text{ Pukulan/feet}$$

❖ Daya dukung berdasarkan kekuatan tanah

1. Untuk end bearing pile

$$Q_p = 40 \cdot N_b \cdot A_p$$

$$= 40 \cdot 26 \cdot 0,196$$

$$= 204,100 \text{ ton}$$

Nilai  $q_{c \text{ rata-rata}}$  sepanjang tiang bor :

1. 1,5	→ $q_c = 20$	$\text{kg/cm}^2$
2. 2,0	→ $q_c = 25$	$\text{kg/cm}^2$
3. 2,4	→ $q_c = 45$	$\text{kg/cm}^2$
4. 2,8	→ $q_c = 60$	$\text{kg/cm}^2$
5. 3,2	→ $q_c = 70$	$\text{kg/cm}^2$
6. 3,6	→ $q_c = 50$	$\text{kg/cm}^2$
7. 4,0	→ $q_c = 40$	$\text{kg/cm}^2$
8. 4,4	→ $q_c = 35$	$\text{kg/cm}^2$
9. 4,8	→ $q_c = 25$	$\text{kg/cm}^2$
10. 5,2	→ $q_c = 35$	$\text{kg/cm}^2$
11. 5,6	→ $q_c = 50$	$\text{kg/cm}^2$
12. 6,0	→ $q_c = 90$	$\text{kg/cm}^2$
13. 6,4	→ $q_c = 150$	$\text{kg/cm}^2 +$
	<hr/>	
	$\Sigma q_c = 695$	$\text{kg/cm}^2$

$$q_{c \text{ rata-rata}} = \frac{q_c}{13} = \frac{695}{13} = 53,46 \text{ kg/cm}^2$$

$$N_s = \frac{q_{c \text{ rata-rata}}}{4} = \frac{53,46}{4} = 13,365 \sim 13 \text{ Pukulan/feet}$$

2. Untuk end friction pile

$$\begin{aligned} Q_s &= 0,1 \cdot N_s \cdot A_s \\ &= 0,1 \cdot 13 \cdot 15,386 \\ &= 20,002 \text{ ton} \end{aligned}$$

➤ Untuk kombinasi end bearing pile dan friction pile

$$\begin{aligned} Q_u &= Q_p + Q_s \\ &= 204,100 + 20,002 \\ &= 224,102 \text{ ton} \end{aligned}$$

❖ Daya dukung berdasarkan kekuatan bahan

$$Q_d = \sigma_{\text{bahan}} \cdot A$$

Dimana :

$\sigma_{\text{bahan}}$  : Tegangan ijin bahan

A : Luas Penampang

1. Untuk dinding tiang (K-250)

$$\begin{aligned} Q_d &= (0,85 \cdot f_c') \times \left( \frac{1}{4} \cdot \pi \cdot D_{\text{tiang}}^2 \right) \\ &= (0,85 \cdot 250) \times \left( \frac{1}{4} \cdot \pi \cdot 50^2 \right) \\ &= 417031,25 \text{ kg} = 417,031 \text{ ton} \end{aligned}$$

➤ Daya dukung satu tiang yang diizinkan ( $Q_{\text{izin}}$ )

$$\begin{aligned} Q_{\text{izin}} &= \frac{Q_u}{SF} \\ &= \frac{224,102}{3} = 74,70060 \text{ ton} = 74700,60 \text{ kg} \end{aligned}$$



#### 4.2.4.1 Perencanaan Tiang Bor

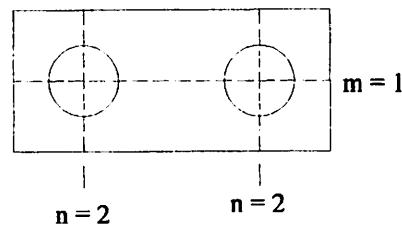
Jumlah tiang dalam satu poer (n)

$$n = \frac{V}{Q_{izin\ 1\ tiang}}$$
$$= \frac{57}{74,70060} = 0,763$$

Dicoba 2 buah tiang dengan susunan :

$$m \text{ (jumlah baris tiang)} = 1$$

$$n \text{ (jumlah tiang dalam baris)} = 2$$



**Gambar 4.11 : Jumlah Tiang Bor Tipe 3**

➤ Syarat jarak antar tiang

$$S \leq \frac{(1,57 \times D \times m \times n) - 2D}{(m+n) - 2}$$

$$S \leq \frac{(1,57 \times 0,5 \times 1 \times 2) - 2 \times 0,5}{(1+2) - 2}$$

$$S \leq 0,57$$

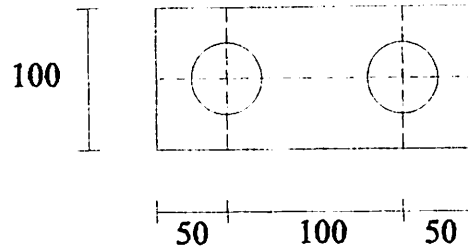
➤ Kontrol jarak antar tiang

$$2,5D \leq S \leq 3D$$

$$2,5(0,5) \leq S \leq 3(0,5)$$

$$1,25 \leq S \leq 1,50$$

Dipakai jarak antar tiang (s) = 1,00 m



Gambar 4.12 : Jarak Antar Tiang Bor Tipe 3

Dengan demikian perencanaan pondasi tiang bor tipe 3 dapat dihitung dengan efisiensi kelompok tiang.

Efisiensi kelompok tiang

1. Berdasarkan Rumus Los Angles

$$\begin{aligned} \eta &= 1 - \frac{D}{\pi \cdot s \cdot m \cdot n} [(m) \cdot (n - 1) + (n) \cdot (m - 1) + (m - 1) \cdot (n - 1) \cdot \sqrt{2}] \\ &= 1 - \frac{0,5}{3,14 \cdot 1 \cdot 1 \cdot 2} [(1) \cdot (2 - 1) + (2) \cdot (1 - 1) + (1 - 1) \cdot (2 - 1) \cdot \sqrt{2}] \\ &= 0,920 < 1 \quad \text{OK} \end{aligned}$$

2. Berdasarkan Rumus Converse - Labarre

$$\begin{aligned} \eta &= 1 - \frac{(n-1) \cdot m + (m-1) \cdot n}{90 \cdot m \cdot n} \times \theta & \Rightarrow \theta &= \arctan \frac{D}{s} \\ & & &= \arctan \frac{0,5}{1,0} = 26,565^\circ \\ &= 1 - \frac{(2-1) \cdot 1 + (1-1) \cdot 2}{90 \cdot 1 \cdot 2} \times 26,565^\circ \\ &= 0,852 < 1 \quad \text{OK} \end{aligned}$$

3. Berdasarkan Rumus Seiler – Keeney

$$\eta = 1 - \frac{(36 \cdot s) \cdot (m+n-2)}{(75 \cdot s^2 \cdot -7) \cdot (m+n-1)} + \frac{0,3}{(m+n)}$$

$$\eta = 1 - \frac{(36 \cdot 1) \cdot (1+2-2)}{(75 \cdot 1^2 \cdot -7) \cdot (1+2-1)} + \frac{0,3}{(1+2)}$$

$$= 0,835 < 1 \quad \text{OK}$$

Berdasarkan dari perhitungan nilai efisiensi, diambil nilai  $\eta$  yang terkecil yaitu : 0,835 yang digunakan untuk perhitungan selanjutnya.

❖ Daya dukung tiang kelompok

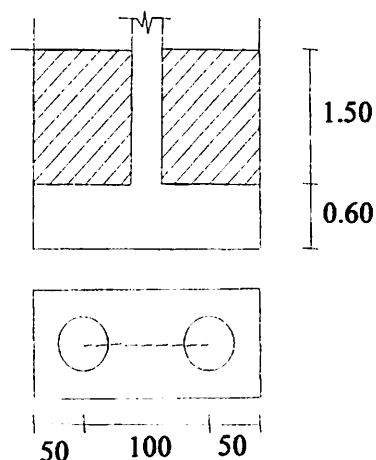
$$Q_{pg} = \eta \times n \times Q_{izin}$$

$$= 0,635 \times 2 \times 74,70060$$

$$= 124,794 \text{ ton} > V = 57 \text{ ton} \quad (\text{OK}), \text{ Syarat } Q_{pg} > V$$

❖ Kontrol  $\Sigma V$  dimana,  $\Sigma V = (V + \text{berat poer}) < Q_{pg}$

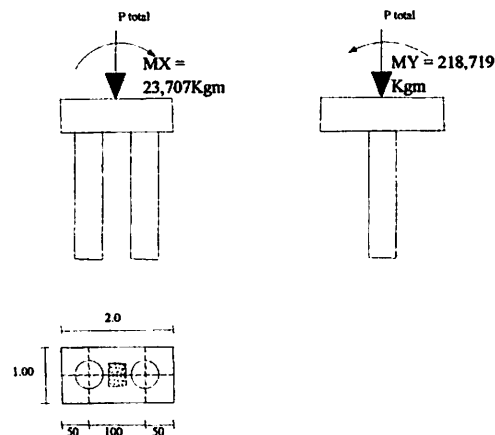
Perhitungan beban poer



**Gambar 4.13 : Perencanaan Poer**

$$\begin{aligned}
 \text{Poer} &= [(2,0 \times 1,00) + (0,3 \times 0,4 \times 1,5)] \times 2400 \\
 &= 433,350 \text{ kg} \\
 \text{Tanah urug} &= [(2,0 \times 1,00 \times 0,6) - (0,3 \times 0,4 \times 1,5)] \times 1700 \\
 &= 1989 \text{ kg} \\
 V &= 57000 \text{ kg} \\
 \Sigma V &= \text{Berat sendiri poer} + \text{Berat tanah urug} + V \\
 &= 433,350 + 1989 + 57000 \\
 &= 59422,350 \text{ kg} \\
 &= 59,9423 \text{ ton} < Q_{pg} = 124,794 \text{ ton} \quad (\text{OK})
 \end{aligned}$$

#### 4.2.4.2 Perhitungan Beban Yang Diterima Oleh Pondasi Tiang Bor



**Gambar 4.15 : Pondasi Bor yang Menenrima Beban V dan M**

Mencari beban tiang maximum

$$P_{\max} = \frac{\Sigma V}{n} \pm \frac{M_y \cdot X_{\max}}{n_y \cdot \Sigma X^2} \pm \frac{M_x \cdot Y_{\max}}{n_x \cdot \Sigma Y^2}$$

Dimana :

$$\Sigma V = 59422,350 \text{ kg} \quad ; Mx = 23,707 \text{ kgm}$$

$$n = 2 \text{ tiang} \quad ; My = 218,719 \text{ kgm}$$

$$ny = 1 \text{ tiang}$$

$$nx = 2 \text{ tiang}$$

$$\Sigma X^2 = \text{Jumlah kuadrat absis tiang}$$

$$= (-1,25^2) + (1,25^2) = 3,13 \text{ m}^2$$

$$\Sigma Y^2 = \text{Jumlah kuadrat ordinat tiang}$$

$$= (-0^2) + (0^2) = 0 \text{ m}^2$$

Sehingga :

$$(x_1 = -1,25 ; y_1 = 0)$$

$$P1 = \frac{59422,350}{2} \pm \frac{218,719 \times (-1,25)}{1 \times 3,13} \pm \frac{23,707 \times (0)}{2 \times 0}$$

$$= 29623,687 \text{ kg}$$

$$(x_2 = 1,25 ; y_2 = 0)$$

$$P2 = \frac{59422,350}{2} \pm \frac{218,719 \times (1,25)}{1 \times 3,13} \pm \frac{22,707 \times (0)}{2 \times 0}$$

$$= 30385,523 \text{ kg}$$

$$P_{\max} = 30385,523 \text{ kg} < Q1 \text{ tiang} = 74700,60 \text{ kg} \dots\dots\dots(\text{aman})$$

Jadi dapat digunakan pondasi tiang bor pada kedalaman 6,4 m dengan diameter 50 cm.

### 4.3 Perencanaan Penulangan Poer Pondasi Strauss

#### 4.3.1 Penulangan Poer Pondasi Strauss Tipe 1

Diketahui :

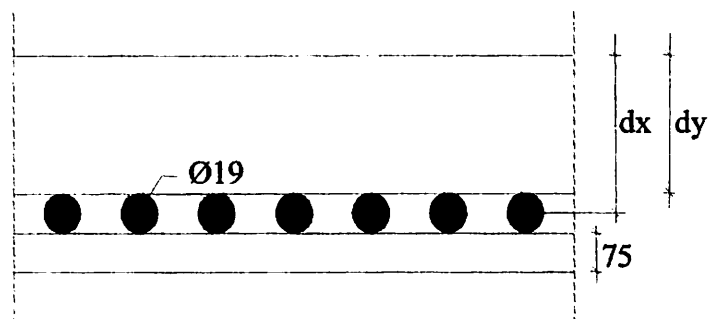
- $P_{max} = 53279,92 \text{ kg} = 53,27992 \text{ ton}$
- $P = \Sigma V = 314051,93 \text{ kg} = 314,05193 \text{ ton}$
- $M_x = 2,03 \text{ tm}$
- $M_y = 1,97 \text{ tm}$
- Mutu Beton ( $f_c'$ ) = 25 Mpa
- Mutu Baja ( $f_y$ ) = 400 Mpa

Direncanakan :

Tebal Poer (H) = 60 cm = 600 mm

Tebal Selimut = 7,5 cm = 75 mm

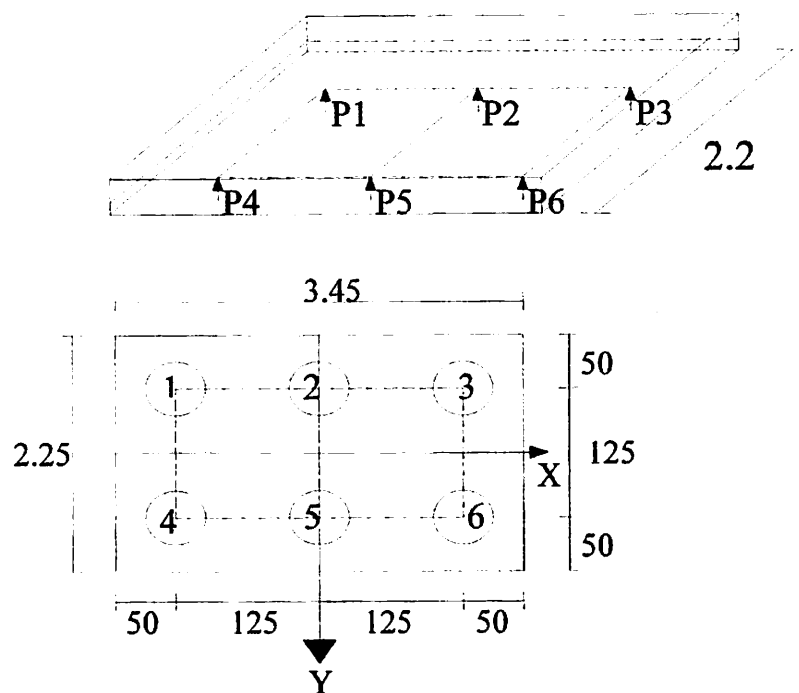
Tul. Pokok ( $\emptyset$ ) = 19



$$dx = 600 - 75 - (\frac{1}{2} \times 19) = 515,5 \text{ mm}$$

$$dy = 600 - 75 - 19 - (\frac{1}{2} \times 19) = 496,5 \text{ mm}$$

### Penulangan Poer



**Gambar 4.14 : Arah Pembebanan Pondasi Tiang Bor Pada Poer**

### Perhitungan Momen

Pada bagian bawah poer diasumsikan sebagai jalur yang dijepit pada bagian sisinya. Dari table : (pelat : Stigle/Wipel : 209) didapat Mye dengan cara interpolasi.

**Tabel 4.2 : Pelat Stiglet/Wipel**

Y/L	0	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1,0
Mye	0,32	0,31	0,30	0,28	0,25	0,21	0,18	0,14	0,09	0,05	0

Momen arah x yang terjadi akibat reaksi dari tiang bor :

Mencari beban masing-masing tiang :

$$P = \frac{\Sigma V}{n} \pm \frac{My \cdot X_{max}}{n_y \cdot \Sigma X^2} \pm \frac{Mx \cdot Y_{max}}{n_x \cdot \Sigma Y^2}$$

Dimana :

$$\Sigma V = 314,05193 \text{ ton} ; Mx = 2,04 \text{ tm}$$

$$n = 6 \text{ tiang} ; My = 1,97 \text{ tm}$$

$$n_y = 2 \text{ tiang}$$

$$n_x = 3 \text{ tiang}$$

$$\Sigma X^2 = \text{Jumlah kuadrat absis tiang}$$

$$= (-1,25^2) + (0^2) + (1,25^2) = 3,13 \text{ m}^2$$

$$\Sigma Y^2 = \text{Jumlah kuadrat ordinat tiang}$$

$$= (-0,625^2) + (0,625^2) = 0,78 \text{ m}^2$$

Sehingga :

$$(x_1 = -1,25 ; y_1 = 0,625)$$



$$P1 = \frac{314,05193}{6} \pm \frac{1,97 \times (-1,25)}{2 \times 3,13} \pm \frac{2,04 \times (0,625)}{3 \times 0,78}$$

$$= 52,489 \text{ ton}$$

$$(x_2 = 0 ; y_1 = 0,625)$$

$$P2 = \frac{314,05193}{6} \pm \frac{1,97 \times (0)}{2 \times 3,13} \pm \frac{2,04 \times (0,625)}{3 \times 0,78}$$

$$= 52,883 \text{ ton}$$

$$(x_3 = 1,25 ; y_3 = 0,625)$$

$$P3 = \frac{314,05193}{6} \pm \frac{1,97 \times (1,25)}{2 \times 3,13} \pm \frac{2,04 \times (0,625)}{3 \times 0,78}$$

$$= 54,277 \text{ ton}$$

$$(Y/L)_1 = \frac{0,5}{0,625+0,625} = 0,4$$

$$= 0,25$$

$$(Y/L)_2 = \frac{0,5}{0,625+0,625} = 0,4$$

$$= 0,25$$

$$(Y/L)_3 = \frac{0,5}{0,625+0,625} = 0,4$$

$$= 0,25$$

$$Mxe 1 = (P1 \times 0,25) + (P2 \times 0,25) + (P3 \times 0,25)$$

$$= (52,482 \times 0,25) + (52,883 \times 0,25) + (53,277 \times 0,25)$$

$$= 39,662 \text{ tm}$$

$$M_{xe 2} = (P_1 \times 0,25) + (P_2 \times 0,25) + (P_3 \times 0,25)$$

$$= (52,482 \times 0,25) + (52,883 \times 0,25) + (53,277 \times 0,25)$$

$$= 39,662 \text{ tm}$$

$$M_{xe 3} = (P_1 \times 0,25) + (P_2 \times 0,25) + (P_3 \times 0,25)$$

$$= (52,482 \times 0,25) + (52,883 \times 0,25) + (53,277 \times 0,25)$$

$$= 39,662 \text{ tm}$$

Momen arah y yang terjadi akibat reaksi dari tiang bor :

Mencari beban masing-masing tiang :

$$P = \frac{\Sigma V}{n} \pm \frac{M_y \cdot X_{max}}{n_y \cdot \Sigma X^2} \pm \frac{M_x \cdot Y_{max}}{n_x \cdot \Sigma Y^2}$$

Sehingga :

$$(x_2 = 0 ; y_2 = 0,625)$$

$$P_2 = \frac{314,05193}{6} \pm \frac{1,97 \times (0)}{2 \times 3,13} \pm \frac{2,04 \times (0,625)}{3 \times 0,78}$$

$$= 52,883 \text{ ton}$$

$$(x_3 = 1,25 ; y_1 = 0,625)$$

$$P_3 = \frac{314,05193}{6} \pm \frac{1,97 \times (1,25)}{2 \times 3,13} \pm \frac{2,04 \times (0,625)}{3 \times 0,78}$$

$$= 53,277 \text{ ton}$$

$$(x_5 = 0 ; y_5 = -0,625)$$

$$P5 = \frac{314,05193}{6} \pm \frac{1,97 \times (0)}{2 \times 3,13} \pm \frac{2,04 \times (-0,625)}{3 \times 0,78}$$

$$= 51,801 \text{ ton}$$

$$(x_6 = 1,25 ; y_6 = -0,625)$$

$$P6 = \frac{314,05193}{6} \pm \frac{1,97 \times (1,25)}{2 \times 3,13} \pm \frac{2,04 \times (-0,625)}{3 \times 0,78}$$

$$= 52,195 \text{ ton}$$

$$\text{Mye 2} = (P2 \times 0,25) + (P3 \times 0,25) + (P5 \times 0,25) + (P6 \times 0,25)$$

$$= (52,883 \times 0,25) + (53,227 \times 0,25) + (51,801 \times 0,25) +$$

$$(52,195 \times 0,25) = 52,539 \text{ tm}$$

$$\text{Mye 3} = (P2 \times 0,25) + (P3 \times 0,25) + (P5 \times 0,25) + (P6 \times 0,25)$$

$$= (52,883 \times 0,25) + (53,227 \times 0,25) + (51,801 \times 0,25) +$$

$$(52,195 \times 0,25) = 52,539 \text{ tm}$$

$$\text{Mye 5} = (P2 \times 0,25) + (P3 \times 0,25) + (P5 \times 0,25) + (P6 \times 0,25)$$

$$= (52,883 \times 0,25) + (53,227 \times 0,25) + (51,801 \times 0,25) +$$

$$(52,195 \times 0,25) = 52,539 \text{ tm}$$

$$\text{Mye 6} = (P2 \times 0,25) + (P3 \times 0,25) + (P5 \times 0,25) + (P6 \times 0,25)$$

$$= (52,883 \times 0,25) + (53,227 \times 0,25) + (51,801 \times 0,25) +$$

$$(52,195 \times 0,25) = 52,539 \text{ tm}$$

#### 4.3.1.1 Perhitungan penulangan poer arah x

$$M_u = 39,662 \text{ tm} = 39662 \text{ kgm}$$

$$M_n = \frac{M_u}{\phi} = \frac{39662}{0,8} = 49578 \text{ kgm} = 49578 \times 10^4 \text{ Nmm}$$

$$d_x = 600 - 75 - (1/2 \times 19) = 515,5 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$R_n = \frac{M_n}{b \times d_x^2} = \frac{49578 \times 10^4}{1000 \times 515,5^2} = 1,866 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{\max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,004$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times 1 -$$

$$\sqrt{1 - \frac{2 \times 1,866 \times 18,824}{400}}$$

$$= 0,002$$

$\rho_{\text{perlu}} < \rho_{\text{min}}$ , maka yang  $\rho_{\text{min}} = 0,004$

$$\begin{aligned}\text{As perlu} &= \rho_{\text{min}} \times b \times dx \\ &= 0,004 \times 1000 \times 515,5 \\ &= 1804,250 \text{ mm}^2\end{aligned}$$

Direncanakan menggunakan tulangan pokok D 19

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d^2} = \frac{1804,250}{0,25 \times \pi \times 19^2} = 6,367 \approx 6 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{6} = 166,667 \approx 150 \text{ mm}$$

$$\begin{aligned}\text{As ada} &= \frac{1000}{150} \times 0,25 \times \pi \times 19^2 \\ &= 1889,233 \text{ mm}^2 > \text{As perlu} = 1804,250 \text{ mm}^2 \text{ (OK)}\end{aligned}$$

**Jadi digunakan tulangan tarik arah x 6D19-150**

Perhitungan tulangan tekan

$$\begin{aligned}\text{As tekan} &= 20\% \times \text{As perlu} \\ &= 20\% \times 1804,250 \\ &= 360,850 \text{ mm}^2\end{aligned}$$

Direncanakan menggunakan tul.tekan D16

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{360,850}{0,25 \times \pi \times 16^2} = 1,796 \approx 5 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{5} = 200 \approx 200 \text{ mm}$$

$$\begin{aligned} \text{As ada} &= \frac{1000}{200} \times 0,25 \times \pi \times 16^2 \\ &= 1004,800 \text{ mm}^2 > \text{As tekan} = 360,850 \text{ mm}^2 \text{ (OK)} \end{aligned}$$

**Jadi digunakan tulangan tekan arah x 5D16**

#### 4.3.1.2 Perhitungan penulangan poer arah y

$$\text{Mu} = 52,539 \text{ tm} = 52539 \text{ kgm}$$

$$\text{Mn} = \frac{\text{Mu}}{\phi} = \frac{52539}{0,8} = 65674 \text{ kgm} = 65674 \times 10^4 \text{ Nmm}$$

$$\text{dy} = 600 - 75 - 19 - (1/2 \times 19) = 496,5 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$\text{Rn} = \frac{\text{Mn}}{b \times \text{dy}^2} = \frac{65674 \times 10^4}{1000 \times 496,5^2} = 2,664 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{\text{max}} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,004$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times 1 - \sqrt{1 - \frac{2 \times 1,866 \times 18,824}{400}}$$

$$= 0,001$$

$\rho_{\text{perlu}} < \rho_{\min}$ , maka yang  $\rho_{\min} = 0,004$

$$\begin{aligned} \text{As perlu} &= \rho_{\min} \times b \times d_y \\ &= 0,004 \times 1000 \times 496,5 \\ &= 1737,750 \text{ mm}^2 \end{aligned}$$

Direncanakan menggunakan tulangan pokok D 19

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d^2} = \frac{1737,750}{0,25 \times \pi \times 19^2} = 6,13 \approx 6 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{6} = 166,667 \approx 160 \text{ mm}$$

$$\begin{aligned} \text{As ada} &= \frac{1000}{160} \times 0,25 \times \pi \times 19^2 \\ &= 1771,156 \text{ mm}^2 > \text{As perlu} = 1737,750 \text{ mm}^2 \text{ (OK)} \end{aligned}$$

**Jadi digunakan tulangan tarik arah y 6D19-160**

Perhitungan tulangan tekan

$$\text{As tekan} = 20\% \times \text{As perlu}$$

$$= 20\% \times 1737,750$$

$$= 347,550 \text{ mm}^2$$

Direncanakan menggunakan tul.tekan D16

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{347,550}{0,25 \times \pi \times 16^2} = 1,73 \approx 5 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{5} = 200 \approx 200 \text{ mm}$$

$$\text{As ada} = \frac{1000}{200} \times 0,25 \times \pi \times 16^2$$

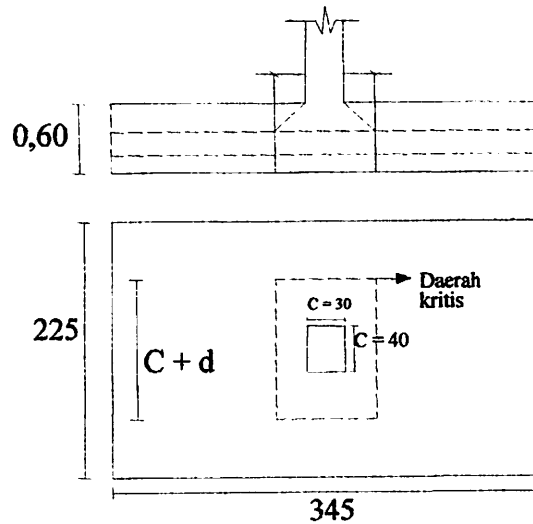
$$= 1004,800 \text{ mm}^2 > \text{As tekan} = 347,550 \text{ mm}^2 \text{ (OK)}$$

**Jadi digunakan tulangan tekan arah x 5D16-200**



### 4.3.1.3 Kontrol Geser Pons (Gaya Geser Dua Arah Sumbu)

- Geser Pons terhadap Kolom



Gambar 4.15 : Skema Geser Pons

Diketahui :

$$V_u = 306 \text{ ton} = 306000 \text{ kg}$$

Tinggi efektif (d)

$$d = \text{tebal poer} - \text{tebal selimut beton} - \frac{1}{2} \text{ diameter tulangan}$$

terluar

$$= 600 - 75 - \frac{1}{2} \cdot 19$$

$$= 515,5 \text{ mm}$$

Dimensi kolom (c) = 30/40

- Keliling bidang kritis geser pons ( $b_o$ )

$$b_o = 4 \cdot [(c_1 + d) + (c_2 + d)]$$

$$= 4 \cdot [(300 + 515,5) + (400 + 515,5)]$$

$$= 6924 \text{ mm}$$

- Kuat geser beton maksimum

$$\begin{aligned} V_c &= \frac{\sqrt{f_c'}}{3} \times b_o \times d \\ &= \frac{\sqrt{25}}{3} \times 6924 \times 515,5 \\ &= 5948870 \text{ kg} \end{aligned}$$

$$\begin{aligned} \phi V_c &= 0,6 \times V_c \\ &= 0,6 \times 5948870 \\ &= 3569322 \text{ kg} \end{aligned}$$

Maka  $V_u = 306000 \text{ kg} < \phi V_c = 3569322 \text{ kg} \dots\dots\dots(\mathbf{OK})$

Karena  $V_u < \phi V_c$ , maka tidak diperlukan tulangan geser terhadap kolom dan poer aman terhadap geser pons akibat kolom.

### 4.3.2 Penulangan Poer Pondasi Strauss Tipe 2

Diketahui :

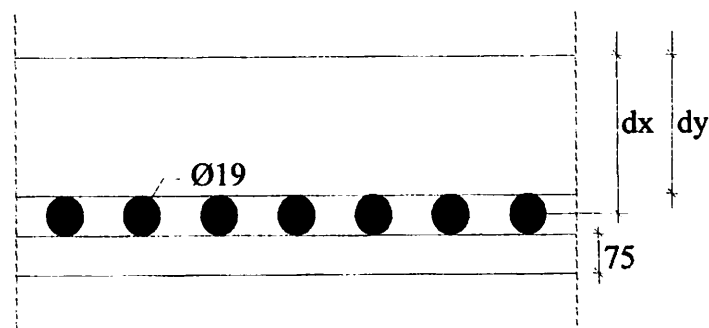
- $P_{max} = 48797,386 \text{ kg} = 48,797386 \text{ ton}$
- $P = \Sigma V = 283873,32 \text{ kg} = 283,87332 \text{ ton}$
- $M_x = 2,59 \text{ tm}$
- $M_y = 2,03 \text{ tm}$
- Mutu Beton ( $f_c'$ ) = 25 Mpa
- Mutu Baja ( $f_y$ ) = 400 Mpa

Direncanakan :

Tebal Poer (H) = 60 cm = 600 mm

Tebal Selimut = 7,5 cm = 75 mm

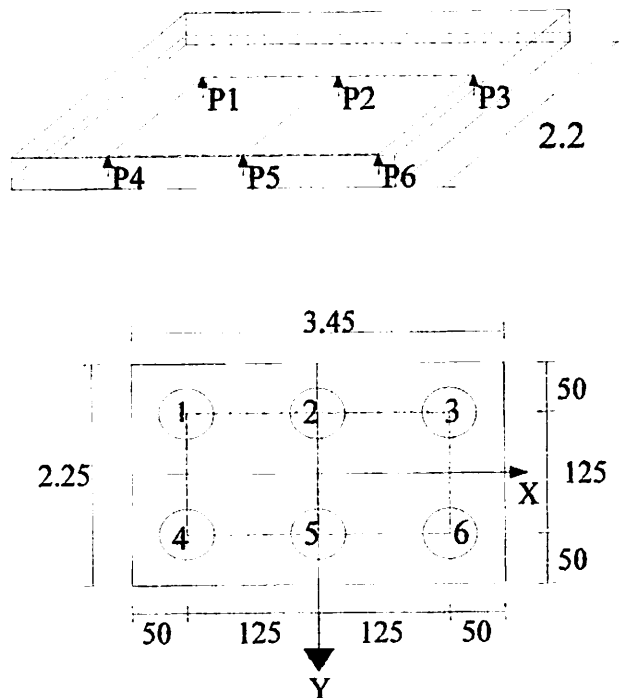
Tul. Pokok ( $\emptyset$ ) = 19



$$dx = 600 - 75 - (1/2 \times 19) = 515,5 \text{ mm}$$

$$dy = 600 - 75 - 19 - (1/2 \times 19) = 496,5 \text{ mm}$$

## Penulangan Poer



**Gambar 4.16 : Arah Pembebanan Pondasi Tiang Bor Pada Poer**

## Perhitungan Momen

Pada bagian bawah poer diasumsikan sebagai jalur yang dijepit pada bagian sisinya. Dari table : (pelat : Stigle/Wipel : 209) didapat Mye dengan cara interpolasi.

**Tabel 4.3 : Pelat Stigle/Wipel**

Y/L	0	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1,0
Mye	0,32	0,31	0,30	0,28	0,25	0,21	0,18	0,14	0,09	0,05	0

Momen arah x yang terjadi akibat reaksi dari tiang bor :

Mencari beban masing-masing tiang :

$$P = \frac{\Sigma V}{n} \pm \frac{M_y \cdot X_{max}}{n_y \cdot \Sigma X^2} \pm \frac{M_x \cdot Y_{max}}{n_x \cdot \Sigma Y^2}$$

Dimana :

$$\Sigma V = 283,873 \text{ ton} \quad ; \quad M_x = 2,59 \text{ tm}$$

$$n = 6 \text{ tiang} \quad ; \quad M_y = 2,03 \text{ tm}$$

$$n_y = 2 \text{ tiang}$$

$$n_x = 3 \text{ tiang}$$

$$\Sigma X^2 = \text{Jumlah kuadrat absis tiang}$$

$$= (-1,25^2) + (0^2) + (1,25^2) = 3,13 \text{ m}^2$$

$$\Sigma Y^2 = \text{Jumlah kuadrat ordinat tiang}$$

$$= (-0,625^2) + (0,625^2) = 0,78 \text{ m}^2$$

Sehingga :

$$(x_1 = -1,25 ; y_1 = 0,625)$$

$$P_1 = \frac{283,873}{6} \pm \frac{2,03 \times (-1,25)}{2 \times 3,13} \pm \frac{2,59 \times (0,625)}{3 \times 0,78}$$

$$= 47,597 \text{ ton}$$

$$(x_2 = 0 ; y_2 = 0,625)$$

$$P2 = \frac{283,873}{6} \pm \frac{2,03 \times (0)}{2 \times 3,13} \pm \frac{2,59 \times (0,625)}{3 \times 0,78}$$

$$= 48,003 \text{ ton}$$

$$(x_3 = 1,25 ; y_3 = 0,625)$$

$$P3 = \frac{283,873}{6} \pm \frac{2,03 \times (1,25)}{2 \times 3,13} \pm \frac{2,59 \times (0,625)}{3 \times 0,78}$$

$$= 48,409 \text{ ton}$$

$$(Y/L)_1 = \frac{0,5}{0,625+0,625} = 0,4$$

$$= 0,25$$

$$(Y/L)_2 = \frac{0,5}{0,625+0,625} = 0,4$$

$$= 0,25$$

$$(Y/L)_3 = \frac{0,5}{0,625+0,625} = 0,4$$

$$= 0,25$$

$$Mxe 1 = (P1 \times 0,25) + (P2 \times 0,25) + (P3 \times 0,25)$$

$$= (47,597 \times 0,25) + (48,003 \times 0,25) + (48,409 \times 0,25)$$

$$= 36,002 \text{ tm}$$

$$Mxe 2 = (P1 \times 0,25) + (P2 \times 0,25) + (P3 \times 0,25)$$

$$= (47,597 \times 0,25) + (48,003 \times 0,25) + (48,409 \times 0,25)$$

$$= 36,002 \text{ tm}$$

$$\begin{aligned} M_{xe3} &= (P1 \times 0,25) + (P2 \times 0,25) + (P3 \times 0,25) \\ &= (47,597 \times 0,25) + (48,003 \times 0,25) + (48,409 \times 0,25) \\ &= 36,002 \text{ tm} \end{aligned}$$

Momen arah y yang terjadi akibat reaksi dari tiang bor :

Mencari beban masing-masing tiang :

$$P = \frac{\Sigma V}{n} \pm \frac{My \cdot X_{max}}{ny \cdot \Sigma X^2} \pm \frac{Mx \cdot Y_{max}}{nx \cdot \Sigma Y^2}$$

Sehingga :

$$(x_2 = 0 ; y_2 = 0,625)$$

$$\begin{aligned} P_2 &= \frac{283,873}{6} \pm \frac{2,03 \times (0)}{2 \times 3,13} \pm \frac{2,59 \times (0,625)}{3 \times 0,78} \\ &= 48,003 \text{ ton} \end{aligned}$$

$$(x_3 = 1,25 ; y_1 = 0,625)$$

$$\begin{aligned} P_3 &= \frac{283,873}{6} \pm \frac{2,03 \times (1,25)}{2 \times 3,13} \pm \frac{2,59 \times (0,625)}{3 \times 0,78} \\ &= 48,409 \text{ ton} \end{aligned}$$

$$(x_5 = 0 ; y_5 = -0,625)$$

$$P_5 = \frac{283,873}{6} \pm \frac{2,03 \times (0)}{2 \times 3,13} \pm \frac{2,59 \times (-0,625)}{3 \times 0,78}$$

$$= 46,622 \text{ ton}$$

$$(x_6 = 1,25 ; y_6 = -0,625)$$

$$P_6 = \frac{283,873}{6} \pm \frac{2,03 \times (1,25)}{2 \times 3,13} \pm \frac{2,59 \times (-0,625)}{3 \times 0,78}$$

$$= 47,028 \text{ ton}$$

$$\text{Mye 2} = (P_2 \times 0,25) + (P_3 \times 0,25) + (P_5 \times 0,25) + (P_6 \times 0,25)$$

$$= (48,003 \times 0,25) + (48,409 \times 0,25) + (46,622 \times 0,25) +$$

$$(47,028 \times 0,25) = 47,515 \text{ tm}$$

$$\text{Mye 3} = (P_2 \times 0,25) + (P_3 \times 0,25) + (P_5 \times 0,25) + (P_6 \times 0,25)$$

$$= (48,003 \times 0,25) + (48,409 \times 0,25) + (46,622 \times 0,25) +$$

$$(47,028 \times 0,25) = 47,515 \text{ tm}$$

$$\text{Mye 5} = (P_2 \times 0,25) + (P_3 \times 0,25) + (P_5 \times 0,25) + (P_6 \times 0,25)$$

$$= (48,003 \times 0,25) + (48,409 \times 0,25) + (46,622 \times 0,25) +$$

$$(47,028 \times 0,25) = 47,515 \text{ tm}$$

$$\text{Mye 6} = (P_2 \times 0,25) + (P_3 \times 0,25) + (P_5 \times 0,25) + (P_6 \times 0,25)$$

$$= (48,003 \times 0,25) + (48,409 \times 0,25) + (46,622 \times 0,25) +$$

$$(47,028 \times 0,25) = 47,515 \text{ tm}$$



#### 4.3.2.1 Perhitungan penulangan poer arah x

$$M_u = 36,002 \text{ tm} = 36002 \text{ kgm}$$

$$M_n = \frac{M_u}{\phi} = \frac{36002}{0,8} = 45003 \text{ kgm} = 45003 \times 10^4 \text{ Nmm}$$

$$d_x = 600 - 75 - (1/2 \times 19) = 515,5 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$R_n = \frac{M_n}{b \times d_x^2} = \frac{45003 \times 10^4}{1000 \times 515,5^2} = 1,693 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$
$$= 0,0271$$

$$\rho_{\max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,004$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times 1 - \sqrt{1 - \frac{2 \times 1,693 \times 18,824}{400}}$$
$$= 0,001$$

$\rho_{\text{perlu}} < \rho_{\min}$ , maka yang  $\rho_{\min} = 0,004$

$$\begin{aligned}
 \text{As perlu} &= \rho_{\min} \times b \times dx \\
 &= 0,004 \times 1000 \times 515,5 \\
 &= 1804,250 \text{ mm}^2
 \end{aligned}$$

Direncanakan menggunakan tulangan pokok D 19

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d^2} = \frac{1804,250}{0,25 \times \pi \times 19^2} = 6,367 \approx 6 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{6} = 166,667 \approx 150 \text{ mm}$$

$$\begin{aligned}
 \text{As ada} &= \frac{1000}{150} \times 0,25 \times \pi \times 19^2 \\
 &= 1889,233 \text{ mm}^2 > \text{As perlu} = 1804,250 \text{ mm}^2 \text{ (OK)}
 \end{aligned}$$

**Jadi digunakan tulangan tarik arah x 6D19-150**

Perhitungan tulangan tekan

$$\begin{aligned}
 \text{As tekan} &= 20\% \times \text{As perlu} \\
 &= 20\% \times 1804,250 \\
 &= 360,850 \text{ mm}^2
 \end{aligned}$$

Direncanakan menggunakan tul.tekan D16

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{360,850}{0,25 \times \pi \times 16^2} = 1,796 \approx 5 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{5} = 200 \approx 200 \text{ mm}$$

$$\text{As ada} = \frac{1000}{200} \times 0,25 \times \pi \times 16^2$$

$$= 1004,800 \text{ mm}^2 > \text{As tekan} = 360,850 \text{ mm}^2 \text{ (OK)}$$

**Jadi digunakan tulangan tekan arah x 5D16-200**

#### 4.3.2.2 Perhitungan penulangan poer arah y

$$M_u = 47,515 \text{ tm} = 47515 \text{ kgm}$$

$$M_n = \frac{M_u}{\phi} = \frac{47515}{0,8} = 59394 \text{ kgm} = 59394 \times 10^4 \text{ Nmm}$$

$$d_y = 600 - 75 - 19 - (1/2 \times 19) = 496,5 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$R_n = \frac{M_n}{b \times d^2} = \frac{59394 \times 10^4}{1000 \times 496,5^2} = 2,409 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{\max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,004$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times 1 - \sqrt{1 - \frac{2 \times 2,409 \times 18,824}{400}}$$

$$= 0,006$$

$\rho_{\text{perlu}} > \rho_{\text{min}}$ , maka yang  $\rho_{\text{perlu}} = 0,006$

$$\begin{aligned} \text{As perlu} &= \rho_{\text{min}} \times b \times d_y \\ &= 0,006 \times 1000 \times 496,5 \\ &= 2979 \text{ mm}^2 \end{aligned}$$

Direncanakan menggunakan tulangan pokok D 19

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d^2} = \frac{2979}{0,25 \times \pi \times 19^2} = 10,51 \approx 11 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{11} = 90,91 \approx 90 \text{ mm}$$

$$\begin{aligned} \text{As ada} &= \frac{1000}{90} \times 0,25 \times \pi \times 19^2 \\ &= 3148,722 \text{ mm}^2 > \text{As perlu} = 2979 \text{ mm}^2 \text{ (OK)} \end{aligned}$$

**Jadi digunakan tulangan tarik arah y 11D19-90**

### Perhitungan tulangan tekan

$$\begin{aligned}\text{As tekan} &= 20\% \times \text{As perlu} \\ &= 20\% \times 2979 \\ &= 595,800 \text{ mm}^2\end{aligned}$$

### Direncanakan menggunakan tul.tekan D16

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{595,800}{0,25 \times \pi \times 16^2} = 2,96 \approx 3 \text{ tul.}$$

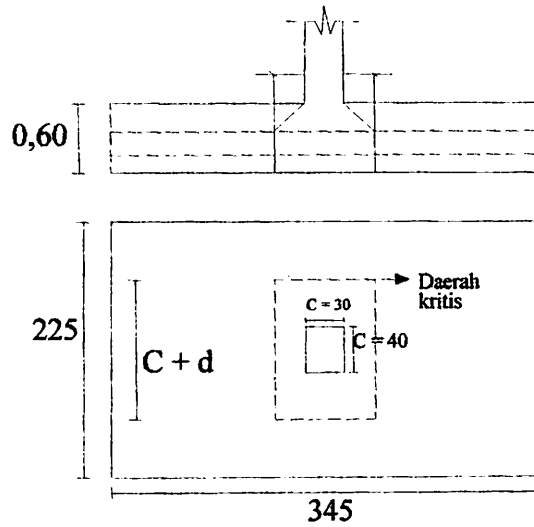
$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{3} = 333,33 \approx 300 \text{ mm}$$

$$\begin{aligned}\text{As ada} &= \frac{1000}{300} \times 0,25 \times \pi \times 16^2 \\ &= 666,867 \text{ mm}^2 > \text{As tekan} = 595,800 \text{ mm}^2 \text{ (OK)}\end{aligned}$$

**Jadi digunakan tulangan tekan arah x 3D16-300**

### 4.3.2.3 Kontrol Geser Pons (Gaya Geser Dua Arah Sumbu)

- Geser Pons terhadap Kolom



Gambar 4.17 : Skema Geser Pons

Diketahui :

$$V_u = 283 \text{ ton} = 283000 \text{ kg}$$

Tinggi efektif (d)

$$d = \text{tebal poer} - \text{tebal selimut beton} - \frac{1}{2} \text{ diameter tulangan}$$

terluar

$$= 600 - 75 - \frac{1}{2} \cdot 19$$

$$= 515,5 \text{ mm}$$

Dimensi kolom (c) = 30/40

- Keliling bidang kritis geser pons ( $b_o$ )

$$b_o = 4 \cdot [(c_1 + d) + (c_2 + d)]$$

$$= 4 \cdot [(300 + 515,5) + (400 + 515,5)]$$

$$= 6924 \text{ mm}$$

- Kuat geser beton maksimum

$$\begin{aligned}V_c &= \frac{\sqrt{f_c'}}{3} \times b_o \times d \\ &= \frac{\sqrt{25}}{3} \times 6924 \times 515,5 \\ &= 5948870 \text{ kg}\end{aligned}$$

$$\begin{aligned}\phi V_c &= 0,6 \times V_c \\ &= 0,6 \times 5948870 \\ &= 3569322 \text{ kg}\end{aligned}$$

Maka  $V_u = 306000 \text{ kg} < \phi V_c = 3569322 \text{ kg} \dots \dots \dots \text{(OK)}$

Karena  $V_u < \phi V_c$ , maka tidak diperlukan tulangan geser terhadap kolom dan poer aman terhadap geser pons akibat kolom.

### 4.3.3 Penulangan Poer Pondasi Strauss Tipe 3

Diketahui :

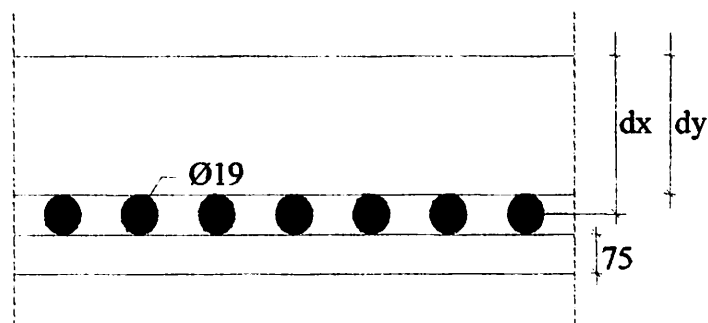
- $P_{max} = 29798,663 \text{ kg} = 29,798663 \text{ ton}$
- $P = \Sigma V = 59422,350 \text{ kg} = 59,422350 \text{ ton}$
- $M_x = 0,0237 \text{ tm}$
- $M_y = 0,2187 \text{ tm}$
- Mutu Beton ( $f_c'$ ) = 25 Mpa
- Mutu Baja ( $f_y$ ) = 400 Mpa

Direncanakan :

Tebal Poer (H) = 60 cm = 600 mm

Tebal Selimut = 7,5 cm = 75 mm

Tul. Pokok ( $\emptyset$ ) = 19

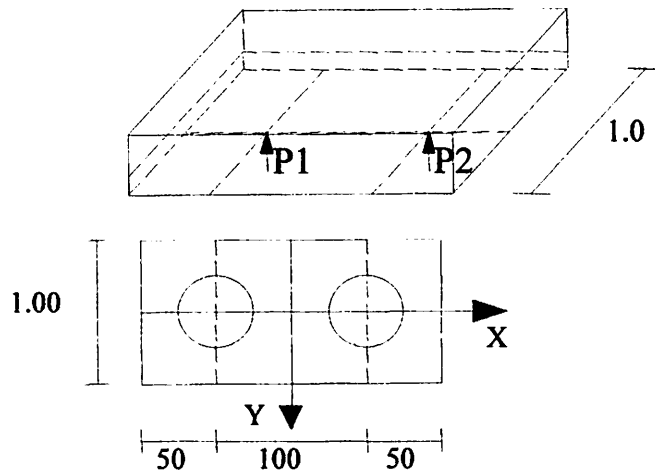


$$dx = 600 - 75 - (\frac{1}{2} \times 19) = 515,5 \text{ mm}$$

$$dy = 600 - 75 - 19 - (\frac{1}{2} \times 19) = 496,5 \text{ mm}$$



### Penulangan Poer



**Gambar 4.18 : Arah Pembebanan Pondasi Tiang Bor Pada Poer**

### Perhitungan Momen

Pada bagian bawah poer diasumsikan sebagai jalur yang dijepit pada bagian sisinya. Dari table : (pelat : Stigle/Wipel : 209) didapat Mye dengan cara interpolasi.

**Tabel 4.4 : Tabel pelat Stigle / Wipel**

Y/L	0	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1,0
Mye	0,32	0,31	0,30	0,28	0,25	0,21	0,18	0,14	0,09	0,05	0

Momen arah x yang terjadi akibat reaksi dari tiang bor :

Mencari beban masing-masing tiang :

$$P = \frac{\sum V}{n} \pm \frac{M_y \cdot X_{max}}{n_y \cdot \sum X^2} \pm \frac{M_x \cdot Y_{max}}{n_x \cdot \sum Y^2}$$

Dimana :

$$\Sigma V = 59,422 \text{ ton} \quad ; Mx = 0,0237 \text{ tm}$$

$$n = 2 \text{ tiang} \quad ; My = 0,2187 \text{ tm}$$

$$ny = 1 \text{ tiang}$$

$$nx = 2 \text{ tiang}$$

$$\Sigma X^2 = \text{Jumlah kuadrat absis tiang}$$

$$= (-1,25^2) + (1,25^2) = 3,13 \text{ m}^2$$

$$\Sigma Y^2 = \text{Jumlah kuadrat ordinat tiang}$$

$$= (-0^2) + (0^2) = 0 \text{ m}^2$$

Sehingga :

$$(x_1 = -1,25 ; y_1 = 0)$$

$$P1 = \frac{59,422}{2} \pm \frac{0,2187 \times (-1,25)}{1 \times 3,13} \pm \frac{0,0237 \times (0)}{2 \times 0}$$

$$= 29,624 \text{ ton}$$

$$(x_2 = 1,25 ; y_2 = 0)$$

$$P2 = \frac{59,422}{2} \pm \frac{0,2187 \times (1,25)}{1 \times 3,13} \pm \frac{0,0237 \times (0)}{2 \times 0}$$

$$= 29,799 \text{ ton}$$

$$(Y/L)_1 = \frac{0,5}{0,625+0,625} = 0,4$$

$$= 0,25$$

$$(Y/L)_2 = \frac{0,5}{0,625+0,625} = 0,4$$

$$= 0,25$$

$$M_{xe 1} = (P_1 \times 0,25) + (P_2 \times 0,25)$$

$$= (29,624 \times 0,25) + (29,799 \times 0,25)$$

$$= 14,856 \text{ tm}$$

$$M_{xe 2} = (P_1 \times 0,25) + (P_2 \times 0,25)$$

$$= (29,624 \times 0,25) + (29,799 \times 0,25)$$

$$= 14,856 \text{ tm}$$

Momen arah y yang terjadi akibat reaksi dari tiang bor :

Mencari beban masing-masing tiang :

$$P = \frac{\Sigma V}{n} \pm \frac{M_y \cdot X_{max}}{n_y \cdot \Sigma X^2} \pm \frac{M_x \cdot Y_{max}}{n_x \cdot \Sigma Y^2}$$

Sehingga :

$$(x_2 = 1,25 ; y_2 = 0)$$

$$P2 = \frac{59,422}{2} \pm \frac{0,2187 \times (1,25)}{1 \times 3,13} \pm \frac{0,0237 \times (0)}{2 \times 0}$$

$$= 29,799 \text{ ton}$$

$$M_{xe\ 2} = (P1 \times 0,25) + (P2 \times 0,25)$$

$$= (29,624 \times 0,25) + (29,799 \times 0,25)$$

$$= 14,856 \text{ tm}$$

#### 4.3.3.1 Perhitungan penulangan poer arah x

$$M_u = 14,856 \text{ tm} = 14856 \text{ kgm}$$

$$M_n = \frac{M_u}{\phi} = \frac{14856}{0,8} = 18569 \text{ kgm} = 18569 \times 10^4 \text{ Nmm}$$

$$d_x = 600 - 75 - (1/2 \times 19) = 515,5 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$R_n = \frac{M_n}{b \times d_x^2} = \frac{18569 \times 10^4}{1000 \times 515,5^2} = 0,699 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{\max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,004$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times 1 - \sqrt{1 - \frac{2 \times 0,699 \times 18,824}{400}}$$

$$= 0,002$$

$\rho_{\text{perlu}} < \rho_{\text{min}}$ , maka yang  $\rho_{\text{min}} = 0,004$

$$\begin{aligned} \text{As perlu} &= \rho_{\text{min}} \times b \times dx \\ &= 0,004 \times 1000 \times 515,5 \\ &= 1804,250 \text{ mm}^2 \end{aligned}$$

Direncanakan menggunakan tulangan pokok D 19

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d^2} = \frac{1804,250}{0,25 \times \pi \times 19^2} = 6,367 \approx 6 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{6} = 166,667 \approx 150 \text{ mm}$$

$$\begin{aligned} \text{As ada} &= \frac{1000}{150} \times 0,25 \times \pi \times 19^2 \\ &= 1889,233 \text{ mm}^2 > \text{As perlu} = 1804,250 \text{ mm}^2 \text{ (OK)} \end{aligned}$$

**Jadi digunakan tulangan tarik arah x 6D19-150**

Perhitungan tulangan tekan

$$\begin{aligned}\text{As tekan} &= 20\% \times \text{As perlu} \\ &= 20\% \times 1804,250 \\ &= 360,850 \text{ mm}^2\end{aligned}$$

Direncanakan menggunakan tul.tekan D16

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{360,850}{0,25 \times \pi \times 16^2} = 1,796 \approx 5 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{5} = 200 \approx 200 \text{ mm}$$

$$\begin{aligned}\text{As ada} &= \frac{1000}{200} \times 0,25 \times \pi \times 16^2 \\ &= 1004,800 \text{ mm}^2 > \text{As tekan} = 360,850 \text{ mm}^2 \text{ (OK)}\end{aligned}$$

**Jadi digunakan tulangan tekan arah x 5D16-200**

#### 4.3.3.2 Perhitungan penulangan poer arah y

$$\text{Mu} = 14,856 \text{ tm} = 14856 \text{ kgm}$$

$$\text{Mn} = \frac{\text{Mu}}{\phi} = \frac{14856}{0,8} = 18569 \text{ kgm} = 18569 \times 10^4 \text{ Nmm}$$

$$\text{dy} = 600 - 75 - 19 - \left(\frac{1}{2} \times 19\right) = 496,5 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$R_n = \frac{M_n}{b \times d^2} = \frac{18569 \times 10^4}{1000 \times 496,5^2} = 0,753 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{\max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,004$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times 1 -$$

$$\sqrt{1 - \frac{2 \times 0,753 \times 18,824}{400}}$$

$$= 0,001$$

$\rho_{\text{perlu}} < \rho_{\min}$ , maka yang  $\rho_{\min} = 0,004$

$$\text{As perlu} = \rho_{\min} \times b \times d_y$$

$$= 0,004 \times 1000 \times 496,5$$

$$= 1737,750 \text{ mm}^2$$

Direncanakan menggunakan tulangan pokok D 19

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d^2} = \frac{1737,750}{0,25 \times \pi \times 19^2} = 6,13 \approx 6 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{6} = 166,667 \approx 160 \text{ mm}$$

$$\begin{aligned} \text{As ada} &= \frac{1000}{160} \times 0,25 \times \pi \times 19^2 \\ &= 1771,156 \text{ mm}^2 > \text{As perlu} = 1737,750 \text{ mm}^2 \text{ (OK)} \end{aligned}$$

**Jadi digunakan tulangan tarik arah y 6D19-160**

Perhitungan tulangan tekan

$$\begin{aligned} \text{As tekan} &= 20\% \times \text{As perlu} \\ &= 20\% \times 1737,750 \\ &= 347,550 \text{ mm}^2 \end{aligned}$$

Direncanakan menggunakan tul.tekan D16

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{347,550}{0,25 \times \pi \times 16^2} = 1,73 \approx 5 \text{ tul.}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{5} = 200 \approx 200 \text{ mm}$$

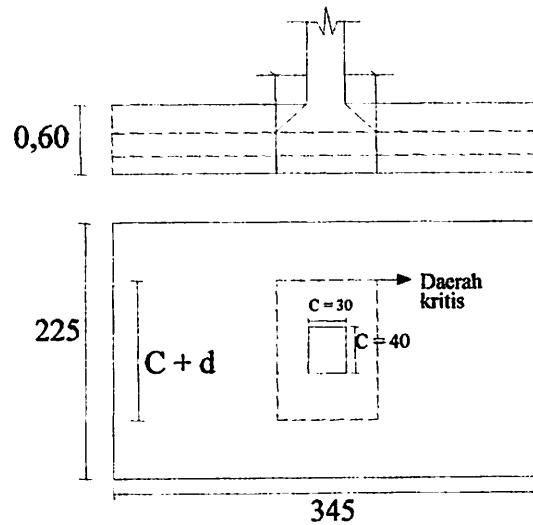
$$\begin{aligned} \text{As ada} &= \frac{1000}{200} \times 0,25 \times \pi \times 16^2 \\ &= 1004,800 \text{ mm}^2 > \text{As tekan} = 347,550 \text{ mm}^2 \text{ (OK)} \end{aligned}$$

**Jadi digunakan tulangan tekan arah x 5D16-200**



### 4.3.3.3 Kontrol Geser Pons (Gaya Geser Dua Arah Sumbu)

- Geser Pons terhadap Kolom



Gambar 4.19 : Skema Geser Pons

Diketahui :

$$V_u = 57 \text{ ton} = 57000 \text{ kg}$$

Tinggi efektif (d)

$$d = \text{tebal poer} - \text{tebal selimut beton} - \frac{1}{2} \text{ diameter tulangan}$$

terluar

$$= 600 - 75 - \frac{1}{2} \cdot 19$$

$$= 515,5 \text{ mm}$$

Dimensi kolom (c) = 30/40

- Keliling bidang kritis geser pons ( $b_o$ )

$$b_o = 4 \cdot [(c_1 + d) + (c_2 + d)]$$

$$= 4 \cdot [(300 + 515,5) + (400 + 515,5)]$$

$$= 6924 \text{ mm}$$

- Kuat geser beton maksimum

$$\begin{aligned} V_c &= \frac{\sqrt{f_c'}}{3} \times b_o \times d \\ &= \frac{\sqrt{25}}{3} \times 6924 \times 515,5 \\ &= 5948870 \text{ kg} \end{aligned}$$

$$\begin{aligned} \phi V_c &= 0,6 \times V_c \\ &= 0,6 \times 5948870 \\ &= 3569322 \text{ kg} \end{aligned}$$

Maka  $V_u = 306000 \text{ kg} < \phi V_c = 3569322 \text{ kg} \dots\dots\dots(\text{OK})$

Karena  $V_u < \phi V_c$ , maka tidak diperlukan tulangan geser terhadap kolom dan poer aman terhadap geser pons akibat kolom.

## 4.4 Perhitungan Tulangan Pondasi Strauss

### 4.4.1 Penulangan Pondasi Strauss Tipe 1

Perhitungan pondasi strauss diasumsikan seperti perhitungan kolom bulat.

Data Perencanaan :

- P max = 53279,92 kg = 53,27992 ton
- $P_u = \Sigma V$  = 314051,51 kg = 314,05151 ton
- Mutu beton ( $f_c'$ ) = 25 Mpa
- Mutu baja tulangan ( $f_y$ ) = 400 Mpa
- D tulangan pokok = 19 mm
- $D_{\text{tiang}}$  = 50 cm = 500 mm
- Tebal selimut = 75 mm

#### 1. Tebal efektif selimut beton terpusat tulangan terluar

$$\begin{aligned}d' &= \text{tebal selimut beton} + \emptyset \text{ sengkang} + \left(\frac{1}{2} D_{\text{tul.pokok}}\right) \\ &= 75 + 12 + \left(\frac{1}{2} 19\right) \\ &= 96,50 \text{ mm}\end{aligned}$$

$$\begin{aligned}d_{\text{efektif}} &= D_{\text{tiang}} - (2 \times d') \\ &= 500 - (2 \times 96,50) \\ &= 307 \text{ mm}\end{aligned}$$

#### 2. Luas penampang Strauss ( $A_g$ )

$$A_g = \left(\frac{1}{4} \times \pi \times D_{\text{tiang}}^2\right)$$

$$= \left(\frac{1}{4} \times \pi \times 500^2\right)$$

$$= 196250 \text{ mm}^2$$

### 3. Luas penampang baja ( $A_{st}$ )

Rencana penulangan dengan perkiraan luas tulangan pokok adalah 3% dari luas tiang.

- $A_{st} = 3\% \times A_g$ 

$$= 3\% \times 196250$$

$$= 5887,50 \text{ mm}^2$$

- Jumlah Tulangan ( $n$ )

$$n = \frac{A_{st}}{\frac{1}{4} \times \pi \times D_{tul}^2}$$

$$= \frac{5887,50}{\frac{1}{4} \times \pi \times 19^2}$$

$$= 20,772 \approx 22 \text{ buah}$$

- $A_{s_{ada}} = n \times \frac{1}{4} \times \pi \times D_{tul}^2$ 

$$= 22 \times \frac{1}{4} \times \pi \times 19^2$$

$$= 6234,27 \text{ mm}^2 > A_{st} = 5887,50 \text{ mm}^2 \text{ (OK)}$$

- $A_s = A_{s'} = 0,5 \times A_{st}$ 

$$= 0,5 \times 5887,50$$

$$= 2943,75 \text{ mm}^2$$

- Jarak tulangan pokok ( $s$ )

$$s = \frac{\pi \times d_{efektif}}{22}$$

$$= \frac{\pi \times 307}{22} = 43,817 \approx 45 \text{ mm}$$

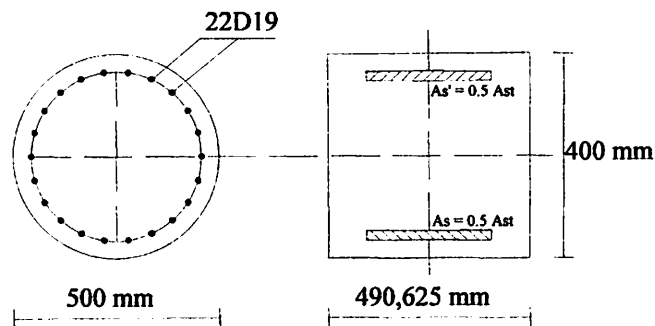
4. Pemeriksaan beban ultimate beton ( $P_{ub}$ ) dan momen ultimate beton ( $M_{ub}$ )

- Tebal penampang segi empat ekivalen

$$\begin{aligned} t_{ek} &= 0,8 \times D_{tiang} \\ &= 0,8 \times 500 \\ &= 400 \text{ mm} \end{aligned}$$

- Lebar penampang segi empat ekivalen

$$\begin{aligned} l_{ek} &= \frac{1/4 \times \pi \times D_{tiang}^2}{t_{ek}} \\ &= \frac{1/4 \times \pi \times 500^2}{400} \\ &= 490,625 \text{ mm} \end{aligned}$$



**Gambar 4.20 : Ekivalen Penampang Bulat ke Penampang Segi Empat**

- Pemeriksaan P terhadap beban seimbang

➤ Jarak antar lapis tulangan

$$\begin{aligned} d - d' &= \frac{2}{3} \times d_{efektif} \\ &= \frac{2}{3} \times 307 \end{aligned}$$

$$= 205,690 \text{ mm}$$

- Jarak tulangan tarik terhadap tepi terluar beton ( $d_b$ )

$$\begin{aligned} d_b &= t_{ek} - \text{tebal selimut efektif} \\ &= 400 - 96,50 \\ &= 303,50 \text{ mm} \end{aligned}$$

- Jarak serat tekan terluar ke garis netral ( $c_b$ )

$$\begin{aligned} c_b &= \frac{600 \times d_b}{600 + f_y} \\ &= \frac{600 \times 303,50}{600 + 400} \\ &= 182,10 \text{ mm} \end{aligned}$$

- Lebar daerah tekan ( $a_b$ )

$$\begin{aligned} a_b &= \beta \times c_b \\ &= 0,85 \times 182,10 \\ &= 154,785 \text{ mm} \end{aligned}$$

- Tegangan tekan tulangan baja ( $f_s'$ )

$$\begin{aligned} f_s' &= \frac{0,003 \times 200000 \times (c_b - d')}{c_b} \\ &= \frac{0,003 \times 200000 \times (182,10 - 96,50)}{182,10} \\ &= 282 \text{ Mpa} < f_y = 400 \text{ Mpa} \end{aligned}$$

- Beban ultimate beton ( $P_{ub}$ )

$$\begin{aligned} P_{ub} &= [(0,85 \times f_c' \times a_b \times l_{ek}) + (A_s' \times f_s') - (A_s \times \\ &f_y)] \times 10^{-3} \end{aligned}$$

$$\begin{aligned}
&= [(0,85 \times 25 \times 154,785 \times 490,63) + \\
&(2943,75 \times 282) - (2943,75 \times 400)] \times 10^{-3} \\
&= 1266,518 \text{ kN}
\end{aligned}$$

- Momen ultimate beton ( $M_{ub}$ )

$$\begin{aligned}
M_{ub} &= [(0,85 \times f_c' \times l_{ek} \times a_b \times \{\frac{t_{ek}}{2} - (\frac{1}{2} \times a_b)\}) + \\
&(A_s' \times f_s' \times (\frac{1}{2} \times (d - d''))) + (A_s \times f_s \times (\frac{1}{2} \\
&(d - d'')))] \times 10^{-6} \\
&= [(0,85 \times 25 \times 154,785 \times 490,63 \times \{\frac{400}{2} - (\frac{1}{2} \\
&\times 154,785)\}) + (2943,75 \times 282 \times (\frac{1}{2} \times \\
&(205,690)) + (2943,75 \times 400 \times (\frac{1}{2} \times \\
&(205,690)))] \times 10^{-6} \\
&= 404,347 \text{ kNmm}
\end{aligned}$$

- Eksentrisitas beton ( $e_b$ )

$$\begin{aligned}
e_b &= \frac{M_{ub}}{P_{ub}} \\
&= \frac{404,347}{1266,518} = 0,319 \text{ m}
\end{aligned}$$

- Eksentrisitas beban ( $e$ )

$$\begin{aligned}
e &= \frac{M_y}{P_{max}} \\
&= \frac{1970}{53270,93} \\
&= 0,037 \text{ m}
\end{aligned}$$

#### 4.4.1.1 Memeriksa Kekuatan Penampang Pondasi Tiang Bor

- Rasio penulangan memanjang ( $\rho_s$ )

$$\begin{aligned}\rho_s &= \frac{A_{st}}{A_g} \\ &= \frac{5887,50}{196250} \\ &= 0,030\end{aligned}$$

- Lebar efektif ( $D_s$ )

$$\begin{aligned}D_s &= D_{tiang} - (2 \times d') \\ &= 500 - (2 \times 96,50) \\ &= 307 \text{ mm}\end{aligned}$$

$$\begin{aligned}m &= \frac{f_y}{0,85 \times f_c'} \\ &= \frac{400}{0,85 \times 25} \\ &= 18,824\end{aligned}$$

- Beban aksial nominal yang diperlukan ( $P_{n_{perlu}}$ )

$$\begin{aligned}P_{n_{perlu}} &= \frac{P_u}{0,7} \\ &= \frac{313763,080}{0,7} \\ &= 448645,019 \text{ kg}\end{aligned}$$

- Persamaan untuk penampang pondasi tiang bor dengan hancur tarik ( $P_n$ )

$$\begin{aligned}P_n &= 0,85 \times f_c \times h^2 \times \\ &\sqrt{\left(\frac{0,85 \times e}{h} - 0,38\right)^2 + \frac{\rho_s \times m \times d_s}{2,50 \times h}}\end{aligned}$$



$$\left(\frac{0,85 \times e}{h} - 0,38\right)$$

$$= 0,85 \times 25 \times 500^2 \times$$

$$\sqrt{\left(\frac{0,85 \times 0,037}{500} - 0,38\right)^2 + \frac{0,03 \times 18,824 \times 307}{2,50 \times 500}} - \left(\frac{0,85 \times 0,37}{500} - 0,38\right)$$

$$= 3723033,628 \text{ kg} > P_{n_{\text{perlu}}} = 448645,019 \text{ kg}$$

- Kuat kolom ( $\emptyset \times P_n$ )

$$\emptyset \times P_n = 0,7 \times 3723033,628$$

$$= 2606123,539 \text{ kg} > P_{\text{max}} = 532270,930 \text{ kg}$$

Dengan demikian perencanaan penampang pondasi tiang bor memenuhi persyaratan sehingga ukuran strauss dan tulangan dapat digunakan.

#### 4.4.1.2 Perencanaan Tulangan Spiral

Direncanakan menggunakan tulangan spiral  $\varnothing$  12 mm

$$A_g = 196250 \text{ mm}^2$$

$$\begin{aligned} A_{S_{\text{spiral}}} &= \frac{1}{4} \times \pi \times d^2 \\ &= \frac{1}{4} \times \pi \times 12^2 = 113,04 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} D_c &= D_{\text{tiang}} - (2 \times \text{selimut beton}) \\ &= 500 - (2 \times 75) \\ &= 350 \text{ mm} \end{aligned}$$

$$\begin{aligned} A_c &= \frac{1}{4} \times \pi \times D_c^2 \\ &= \frac{1}{4} \times \pi \times 350^2 \\ &= 96162,5 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} \rho_{\text{perlu}} &= 0,45 \times \left( \frac{A_g}{A_c} - 1 \right) \times \left( \frac{f_c'}{f_y} \right) \\ &= 0,45 \times \left( \frac{196250}{96162,5} - 1 \right) \times \left( \frac{25}{400} \right) \\ &= 0,029 \end{aligned}$$

- Jarak antar sengkang spiral

$$\begin{aligned} S_{\text{maks}} &= \frac{4 \times A_{S_{\text{spiral}}} \times (D_c - d)}{D_c^2 \times \rho_{\text{perlu}}} \\ &= \frac{4 \times 113,04 \times (350 - 12)}{350^2 \times 0,029} \end{aligned}$$

$$= 42,619 \text{ mm} \approx 40 \text{ mm}$$

Dari perhitungan penulangan pondasi strauss, maka digunakan tulangan pokok 22D19 dan tulangan spiral  $\emptyset 12-40$ .

#### 4.4.2 Penulangan Pondasi Strauss Tipe 2

Perhitungan pondasi strauss diasumsikan seperti perhitungan kolom bulat.

Data Perencanaan :

- P max = 51680,26 kg = 51,68026 ton
- $P_u = \Sigma V$  = 290760,09 kg = 290,76009 ton
- Mutu beton ( $f_c'$ ) = 25 Mpa
- Mutu baja tulangan ( $f_y$ ) = 400 Mpa
- D tulangan pokok = 19 mm
- $D_{\text{tiang}}$  = 50 cm = 500 mm
- Tebal selimut = 75 mm

1. Tebal efektif selimut beton terpusat tulangan terluar

$$\begin{aligned}d' &= \text{tebal selimut beton} + \emptyset \text{ sengkang} + \left(\frac{1}{2} D_{\text{tul.pokok}}\right) \\ &= 75 + 12 + \left(\frac{1}{2} 19\right) \\ &= 96,50 \text{ mm}\end{aligned}$$

$$\begin{aligned}d_{\text{efektif}} &= D_{\text{tiang}} - (2 \times d') \\ &= 500 - (2 \times 96,50) \\ &= 307 \text{ mm}\end{aligned}$$

2. Luas penampang Strauss ( $A_g$ )

$$\begin{aligned}A_g &= \left(\frac{1}{4} \times \pi \times D_{\text{tiang}}^2\right) \\ &= \left(\frac{1}{4} \times \pi \times 500^2\right)\end{aligned}$$

$$= 196250 \text{ mm}^2$$

### 3. Luas penampang baja (Ast)

Rencana penulangan dengan perkiraan luas tulangan pokok adalah 3% dari luas tiang.

- Ast = 3% x Ag  
 = 3% x 196250  
 = 5887,50 mm<sup>2</sup>

- Jumlah Tulangan (n)

$$n = \frac{Ast}{\frac{1}{4} \times \pi \times D_{tul}^2}$$

$$= \frac{5887,50}{\frac{1}{4} \times \pi \times 19^2}$$

$$= 20,772 \approx 22 \text{ buah}$$

- AS<sub>sada</sub> = n x  $\frac{1}{4} \times \pi \times D_{tul}^2$   
 = 22 x  $\frac{1}{4} \times \pi \times 19^2$   
 = 6234,27 mm<sup>2</sup> > Ast = 5887,50 mm<sup>2</sup> (OK)

- As = As' = 0,5 x Ast  
 = 0,5 x 5887,50  
 = 2943,75 mm<sup>2</sup>

- Jarak tulangan pokok (s)

$$s = \frac{\pi \times d_{efektif}}{22}$$

$$= \frac{\pi \times 307}{22} = 43,817 \approx 45 \text{ mm}$$

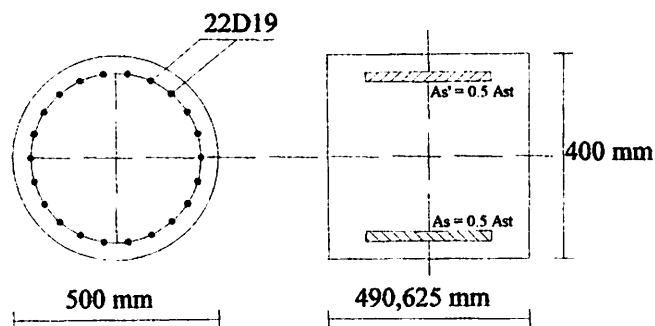
4. Pemeriksaan beban ultimate beton ( $P_{ub}$ ) dan momen ultimate beton ( $M_{ub}$ )

- Tebal penampang segi empat ekivalen

$$\begin{aligned} t_{ek} &= 0,8 \times D_{tiang} \\ &= 0,8 \times 500 \\ &= 400 \text{ mm} \end{aligned}$$

- Lebar penampang segi empat ekivalen

$$\begin{aligned} l_{ek} &= \frac{1/4 \times \pi \times D_{tiang}^2}{t_{ek}} \\ &= \frac{1/4 \times \pi \times 500^2}{400} \\ &= 490,625 \text{ mm} \end{aligned}$$



**Gambar 4.21 : Ekivalen Penampang Bulat ke Penampang Segi Empat**

- Pemeriksaan  $P$  terhadap beban seimbang
  - Jarak antar lapis tulangan

$$\begin{aligned} d - d' &= \frac{2}{3} \times d_{efektif} \\ &= \frac{2}{3} \times 307 \\ &= 205,690 \text{ mm} \end{aligned}$$

- Jarak tulangan tarik terhadap tepi terluar beton ( $d_b$ )

$$\begin{aligned} d_b &= t_{ek} - \text{tebal selimut efektif} \\ &= 400 - 96,50 \\ &= 303,50 \text{ mm} \end{aligned}$$

- Jarak serat tekan terluar ke garis netral ( $c_b$ )

$$\begin{aligned} c_b &= \frac{600 \times d_b}{600 + f_y} \\ &= \frac{600 \times 303,50}{600 + 400} \\ &= 182,10 \text{ mm} \end{aligned}$$

- Lebar daerah tekan ( $a_b$ )

$$\begin{aligned} a_b &= \beta \times c_b \\ &= 0,85 \times 182,10 \\ &= 154,785 \text{ mm} \end{aligned}$$

- Tegangan tekan tulangan baja ( $f_s'$ )

$$\begin{aligned} f_s' &= \frac{0,003 \times 200000 \times (c_b - d')}{c_b} \\ &= \frac{0,003 \times 200000 \times (182,10 - 96,50)}{182,10} \\ &= 282 \text{ Mpa} < f_y = 400 \text{ Mpa} \end{aligned}$$

- Beban ultimate beton ( $P_{ub}$ )

$$\begin{aligned} P_{ub} &= [(0,85 \times f_c' \times a_b \times l_{ek}) + (A_s' \times f_s') - (A_s \times f_y)] \times 10^{-3} \\ &= [(0,85 \times 25 \times 154,785 \times 490,63) + \\ &\quad (2943,75 \times 282) - (2943,75 \times 400)] \times 10^{-3} \end{aligned}$$

$$= 1266,518 \text{ kN}$$

Momen ultimate beton ( $M_{ub}$ )

$$\begin{aligned} M_{ub} &= [(0,85 \times f_c' \times l_{ek} \times ab \times \{\frac{l_{ek}}{2} - (\frac{1}{2} \times ab)\}) + \\ & (A_s' \times f_s' \times (\frac{1}{2} \times (d - d''))) + (A_s \times f_s \times (\frac{1}{2} \\ & (d - d'')))] \times 10^{-6} \\ &= [(0,85 \times 25 \times 154,785 \times 490,63 \times \{\frac{400}{2} - (\frac{1}{2} \\ & \times 154,785)\}) + (2943,75 \times 282 \times (\frac{1}{2} \times \\ & (205,690)) + (2943,75 \times 400 \times (\frac{1}{2} \times \\ & (205,690)))] \times 10^{-6} \\ &= 404,347 \text{ kNmm} \end{aligned}$$

- Eksentrisitas beton ( $e_b$ )

$$\begin{aligned} e_b &= \frac{M_{ub}}{P_{ub}} \\ &= \frac{404,347}{1266,518} \\ &= 0,319 \text{ m} \end{aligned}$$

- Eksentrisitas beban ( $e$ )

$$\begin{aligned} e &= \frac{M_y}{P_{max}} \\ &= \frac{2030}{51680,26} \\ &= 0,039 \text{ m} \end{aligned}$$



#### 4.4.2.1 Memeriksa Kekuatan Penampang Pondasi Tiang Bor

- Rasio penulangan memanjang ( $\rho_s$ )

$$\begin{aligned}\rho_s &= \frac{A_{st}}{A_g} \\ &= \frac{5887,50}{196250} \\ &= 0,030\end{aligned}$$

- Lebar efektif ( $D_s$ )

$$\begin{aligned}D_s &= D_{tiang} - (2 \times d') \\ &= 500 - (2 \times 96,50) \\ &= 307 \text{ mm}\end{aligned}$$

$$\begin{aligned}m &= \frac{f_y}{0,85 \times f_c'} \\ &= \frac{400}{0,85 \times 25} \\ &= 18,824\end{aligned}$$

- Beban aksial nominal yang diperlukan ( $P_{n_{perlu}}$ )

$$\begin{aligned}P_{n_{perlu}} &= \frac{P_u}{0,7} \\ &= \frac{290760,088}{0,7} \\ &= 415371,554 \text{ kg}\end{aligned}$$

- Persamaan untuk penampang pondasi tiang bor dengan hancur tarik ( $P_n$ )

$$P_n = 0,85 \times f_c \times h^2 \times \sqrt{\left(\frac{0,85 \times e}{h} - 0,38\right)^2 + \frac{\rho_s \times m \times d_s}{2,50 \times h}}$$

$$\left( \frac{0,85 \times e}{h} - 0,38 \right)$$

$$= 0,85 \times 25 \times 500^2 \times$$

$$\sqrt{\left( \frac{0,85 \times 0,039}{500} - 0,38 \right)^2 + \frac{0,03 \times 18,824 \times 307}{2,50 \times 500}} - \left( \frac{0,85 \times 0,39}{500} - 0,38 \right)$$

$$= 3722841,284 \text{ kg} > P_{n\text{perlu}} = 415371,554 \text{ kg}$$

- Kuat kolom ( $\emptyset \times P_n$ )

$$\emptyset \times P_n = 0,7 \times 3722841,284$$

$$= 2605988,889 \text{ kg} > P_{\text{max}} = 51680,260 \text{ kg}$$

Dengan demikian perencanaan penampang pondasi tiang bor memenuhi persyaratan sehingga ukuran strauss dan tulangan dapat digunakan.

#### 4.4.2.2 Perencanaan Tulangan Spiral

Direncanakan menggunakan tulangan spiral  $\varnothing$  12 mm

$$A_g = 196250 \text{ mm}^2$$

$$\begin{aligned} A_{s \text{ spiral}} &= \frac{1}{4} \times \pi \times d^2 \\ &= \frac{1}{4} \times \pi \times 12^2 = 113,04 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} D_c &= D_{\text{tiang}} - (2 \times \text{selimut beton}) \\ &= 500 - (2 \times 75) \\ &= 350 \text{ mm} \end{aligned}$$

$$\begin{aligned} A_c &= \frac{1}{4} \times \pi \times D_c^2 \\ &= \frac{1}{4} \times \pi \times 350^2 \\ &= 96162,5 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} \rho_{\text{perlu}} &= 0,45 \times \left( \frac{A_g}{A_c} - 1 \right) \times \left( \frac{f_c'}{f_y} \right) \\ &= 0,45 \times \left( \frac{196250}{96162,5} - 1 \right) \times \left( \frac{25}{400} \right) \\ &= 0,029 \end{aligned}$$

- Jarak antar sengkang spiral

$$\begin{aligned} S_{\text{maks}} &= \frac{4 \times A_{s \text{ spiral}} \times (D_c - d)}{D_c^2 \times \rho_{\text{perlu}}} \\ &= \frac{4 \times 113,04 \times (350 - 12)}{350^2 \times 0,029} \end{aligned}$$

$$= 42,619 \text{ mm} \approx 40 \text{ mm}$$

Dari perhitungan penulangan pondasi strauss, maka digunakan tulangan pokok 22D19 dan tulangan spiral Ø12-40.

#### 4.4.3 Penulangan Pondasi Strauss Tipe 3

Perhitungan pondasi strauss diasumsikan seperti perhitungan kolom bulat.

Data Perencanaan :

- P max = 29798,663 kg = 29,798663 ton
- Pu = ΣV = 59422,350 kg = 59,422350 ton
- Mutu beton (fc') = 25 Mpa
- Mutu baja tulangan (fy) = 400 Mpa
- D tulangan pokok = 19 mm
- D<sub>tiang</sub> = 50 cm = 500 mm
- Tebal selimut = 75 mm

1. Tebal efektif selimut beton terpusat tulangan terluar

$$d' = \text{tebal selimut beton} + \text{Ø sengkang} + \left(\frac{1}{2}\right)$$

$$D_{\text{tul.pokok}}$$

$$= 75 + 12 + \left(\frac{1}{2} 19\right)$$

$$= 96,50 \text{ mm}$$

$$d_{\text{efektif}} = D_{\text{tiang}} - (2 \times d')$$

$$= 500 - (2 \times 96,50)$$

$$= 307 \text{ mm}$$

## 2. Luas penampang Strauss ( $A_g$ )

$$A_g = \left(\frac{1}{4} \times \pi \times D_{\text{tiang}}^2\right)$$

$$= \left(\frac{1}{4} \times \pi \times 500^2\right)$$

$$= 196250 \text{ mm}^2$$

## 3. Luas penampang baja ( $A_{st}$ )

Rencana penulangan dengan perkiraan luas tulangan pokok adalah 3% dari luas tiang.

- $A_{st} = 3\% \times A_g$   
 $= 3\% \times 196250$   
 $= 5887,50 \text{ mm}^2$

- Jumlah Tulangan ( $n$ )

$$n = \frac{A_{st}}{\frac{1}{4} \times \pi \times D_{\text{tul}}^2}$$

$$= \frac{5887,50}{\frac{1}{4} \times \pi \times 19^2}$$

$$= 20,772 \approx 22 \text{ buah}$$

- $A_{Sada} = n \times \frac{1}{4} \times \pi \times D_{\text{tul}}^2$

$$= 22 \times \frac{1}{4} \times \pi \times 19^2$$

$$= 6234,27 \text{ mm}^2 > A_{st} = 5887,50 \text{ mm}^2 \text{ (OK)}$$

- $A_s = A_{s'} = 0,5 \times A_{st}$

$$= 0,5 \times 5887,50$$

$$= 2943,75 \text{ mm}^2$$

- Jarak tulangan pokok (s)

$$s = \frac{\pi \times d_{\text{efektif}}}{22}$$

$$= \frac{\pi \times 307}{22} = 43,817 \approx 45 \text{ mm}$$

4. Pemeriksaan beban ultimate beton ( $P_{ub}$ ) dan momen ultimate beton ( $M_{ub}$ )

- Tebal penampang segi empat ekivalen

$$t_{ek} = 0,8 \times D_{\text{tiang}}$$

$$= 0,8 \times 500$$

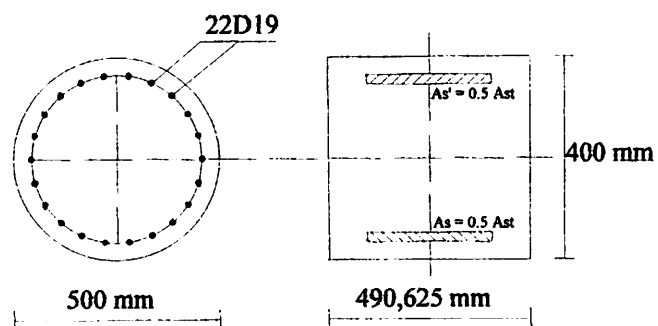
$$= 400 \text{ mm}$$

- Lebar penampang segi empat ekivalen

$$l_{ek} = \frac{1/4 \times \pi \times D_{\text{tiang}}^2}{t_{ek}}$$

$$= \frac{1/4 \times \pi \times 500^2}{400}$$

$$= 490,625 \text{ mm}$$



**Gambar 4.22 : Ekuivalen Penampang Bulat ke Penampang Segi Empat**

- Pemeriksaan P terhadap beban seimbang

- Jarak antar lapis tulangan

$$\begin{aligned} d - d' &= \frac{2}{3} \times d_{\text{efektif}} \\ &= \frac{2}{3} \times 307 \\ &= 205,690 \text{ mm} \end{aligned}$$

- Jarak tulangan tarik terhadap tepi terluar beton ( $d_b$ )

$$\begin{aligned} d_b &= t_{\text{ek}} - \text{tebal selimut efektif} \\ &= 400 - 96,50 \\ &= 303,50 \text{ mm} \end{aligned}$$

- Jarak serat tekan terluar ke garis netral ( $c_b$ )

$$\begin{aligned} c_b &= \frac{600 \times d_b}{600 + f_y} \\ &= \frac{600 \times 303,50}{600 + 400} \\ &= 182,10 \text{ mm} \end{aligned}$$

- Lebar daerah tekan ( $a_b$ )

$$\begin{aligned} a_b &= \beta \times c_b \\ &= 0,85 \times 182,10 \\ &= 154,785 \text{ mm} \end{aligned}$$

- Tegangan tekan tulangan baja ( $f_s'$ )

$$\begin{aligned} f_s' &= \frac{0,003 \times 200000 \times (c_b - d')}{c_b} \\ &= \frac{0,003 \times 200000 \times (182,10 - 96,50)}{182,10} \end{aligned}$$

$$= 282 \text{ Mpa} < f_y = 400 \text{ Mpa}$$

- Beban ultimate beton ( $P_{ub}$ )

$$\begin{aligned} P_{ub} &= [(0,85 \times f_c' \times a_b \times l_{ek}) + (A_s' \times f_s') - (A_s \times f_y)] \times 10^{-3} \\ &= [(0,85 \times 25 \times 154,785 \times 490,63) + (2943,75 \times 282) - (2943,75 \times 400)] \times 10^{-3} \\ &= 1266,518 \text{ kN} \end{aligned}$$

#### Momen ultimate beton ( $M_{ub}$ )

$$\begin{aligned} M_{ub} &= [(0,85 \times f_c' \times l_{ek} \times a_b \times \{\frac{l_{ek}}{2} - (\frac{1}{2} \times a_b)\}) + (A_s' \times f_s' \times (\frac{1}{2} \times (d - d'')) + (A_s \times f_s \times (\frac{1}{2} (d - d'')))] \times 10^{-6} \\ &= [(0,85 \times 25 \times 154,785 \times 490,63 \times \{\frac{400}{2} - (\frac{1}{2} \times 154,785)\}) + (2943,75 \times 282 \times (\frac{1}{2} \times (205,690)) + (2943,75 \times 400 \times (\frac{1}{2} \times (205,690)))] \times 10^{-6} \\ &= 404,347 \text{ kNmm} \end{aligned}$$

- Eksentrisitas beton ( $e_b$ )

$$\begin{aligned} e_b &= \frac{M_{ub}}{P_{ub}} \\ &= \frac{404,347}{1266,518} \\ &= 0,319 \text{ m} \end{aligned}$$



- Eksentrisitas beban (e)

$$\begin{aligned}
 e &= \frac{M_y}{P_{\max}} \\
 &= \frac{218,719}{29798,663} \\
 &= 0,007 \text{ m}
 \end{aligned}$$

#### 4.3.3.1 Memeriksa Kekuatan Penampang Pondasi Tiang Bor

- Rasio penulangan memanjang ( $\rho_s$ )

$$\begin{aligned}
 \rho_s &= \frac{A_{st}}{A_g} \\
 &= \frac{5887,50}{196250} \\
 &= 0,030
 \end{aligned}$$

- Lebar efektif (Ds)

$$\begin{aligned}
 D_s &= D_{\text{tiang}} - (2 \times d') \\
 &= 500 - (2 \times 96,50) \\
 &= 307 \text{ mm}
 \end{aligned}$$

$$\begin{aligned}
 m &= \frac{f_y}{0,85 \times f_c'} \\
 &= \frac{400}{0,85 \times 25} \\
 &= 18,824
 \end{aligned}$$

- Beban aksial nominal yang diperlukan ( $P_{n_{\text{perlu}}}$ )

$$P_{n_{\text{perlu}}} = \frac{P_u}{0,7}$$

$$= \frac{59422,350}{0,7}$$

$$= 84889,071 \text{ kg}$$

- Persamaan untuk penampang pondasi tiang bor dengan hancur tarik ( $P_n$ )

$$P_n = 0,85 \times f_c \times h^2 \times \sqrt{\left(\frac{0,85 \times e}{h} - 0,38\right)^2 + \frac{\rho_s \times m \times d_s}{2,50 \times h}} - \left(\frac{0,85 \times e}{h} - 0,38\right)$$

$$= 0,85 \times 25 \times 500^2 \times$$

$$\sqrt{\left(\frac{0,85 \times 0,007}{500} - 0,38\right)^2 + \frac{0,03 \times 18,824 \times 307}{2,50 \times 500}} - \left(\frac{0,85 \times 0,007}{500} - 0,38\right)$$

$$= 3722997,684 \text{ kg} > P_{n\text{perlu}} = 84889,663 \text{ kg}$$

- Kuat kolom ( $\phi \times P_n$ )

$$\phi \times P_n = 0,7 \times 3722997,684$$

$$= 2606098,379 \text{ kg} > P_{\text{max}} = 29798,663 \text{ kg}$$

Dengan demikian perencanaan penampang pondasi tiang bor memenuhi persyaratan sehingga ukuran strauss dan tulangan dapat digunakan.

#### 4.4.3.2 Perencanaan Tulangan Spiral

Direncanakan menggunakan tulangan spiral  $\emptyset$  12 mm

$$A_g = 196250 \text{ mm}^2$$

$$\begin{aligned} A_{s \text{ spiral}} &= \frac{1}{4} \times \pi \times d^2 \\ &= \frac{1}{4} \times \pi \times 12^2 = 113,04 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} D_c &= D_{\text{tiang}} - (2 \times \text{selimut beton}) \\ &= 500 - (2 \times 75) \\ &= 350 \text{ mm} \end{aligned}$$

$$\begin{aligned} A_c &= \frac{1}{4} \times \pi \times D_c^2 \\ &= \frac{1}{4} \times \pi \times 350^2 \\ &= 96162,5 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} \rho_{\text{pertu}} &= 0,45 \times \left( \frac{A_g}{A_c} - 1 \right) \times \left( \frac{f_c'}{f_y} \right) \\ &= 0,45 \times \left( \frac{196250}{96162,5} - 1 \right) \times \left( \frac{25}{400} \right) \\ &= 0,029 \end{aligned}$$

- Jarak antar sengkang spiral

$$\begin{aligned} S_{\text{maks}} &= \frac{4 \times A_{s \text{ spiral}} \times (D_c - d)}{D_c^2 \times \rho_{\text{pertu}}} \\ &= \frac{4 \times 113,04 \times (350 - 12)}{350^2 \times 0,029} \end{aligned}$$

$$= 42,610 \text{ mm} \approx 40 \text{ mm}$$

Dari perhitungan penulangan pondasi strauss, maka digunakan tulangan pokok 22D19 dan tulangan spiral Ø12-40.

## 4.5 Perencanaan Pondasi Sumuran

### 4.5.1 Perencanaan Pondasi Sumuran Tipe 1 pada Join 3752

V : 306 ton ; Mx : 2,04 tm ; Mz : 1,97 tm

Diameter Sumuran ( $D_{\text{luar}}$ ) = 100 cm = 1,00 m

Diameter Sumuran ( $D_{\text{dalam}}$ ) = 50 cm = 0,50 m

$$\begin{aligned}\text{Luas Penampang } (A_{\text{tiang}}) &= \frac{1}{4} \cdot \pi \cdot D_{\text{luar}}^2 - \frac{1}{4} \cdot \pi \cdot D_{\text{dalam}}^2 \\ &= \frac{1}{4} \cdot \pi \cdot 100^2 - \frac{1}{4} \cdot \pi \cdot 50^2 \\ &= 5887,5 \text{ cm}^2 = 5,8875 \text{ m}^2\end{aligned}$$

Angka Keamanan (SF) = 3

Data Sondir = Titik S<sub>2</sub>

Perhitungan Konversi Nilai Qc Kedalam Parameter Tanah

Konversi nilai sudut geser ( $\emptyset$ ), misalkan untuk kedalaman 6,00 meter.

Konversi nilai qc ke N

$$N = \frac{qc}{4}$$

Sehingga :

$$N = \frac{90}{4}$$

$$= 22,50 = 23 \text{ Pukulan/feet}$$

**Tabel 4.5 : konversi qc ke N pada S1**

N0	Kedalaman	qc	N
	(meter)	Kg/cm <sup>2</sup>	(Pukulan)
1	0,00	0	0,00
2	0,40	25	6
3	1,00	10	3
4	1,40	20	5
5	2,00	35	9
6	2,40	45	11
7	3,00	30	8
8	3,40	20	5
9	4,00	25	6
10	4,40	20	5
11	5,00	45	11
12	5,40	50	13
13	6,00	150	38

**Tabel 4.6 : konversi qc ke N pada S2**

N0	Kedalaman	qc	N
	(meter)	Kg/cm <sup>2</sup>	(Pukulan)
1	0,00	0	0,00
2	0,40	10	3
3	1,00	25	6
4	1,40	15	4
5	2,00	25	6
6	2,40	45	11
7	3,00	65	16
8	3,40	65	16
9	4,00	40	10
10	4,40	35	9
11	5,00	30	8
12	5,40	35	9
13	6,00	90	23
14	6,40	150	38

**Tabel 4.7 : konversi qc ke N pada S3**

N0	Kedalaman	qc	N
	(meter)	Kg/cm <sup>2</sup>	(Pukulan)
1	0,00	0	0,00
2	0,40	10	3
3	1,00	25	6
4	1,40	15	4
5	2,00	25	6
6	2,40	45	11
7	3,00	65	16
8	3,40	65	16
9	4,00	40	10
10	4,40	35	9
11	5,00	30	8
12	5,40	35	9
13	6,00	90	23
14	6,40	150	38
15	6,60	155	39



## Daya Dukung Pondasi Sumuran

### 1. Ditinjau dari daya dukung bahan

$$Qd = \sigma_{\text{bahan}} \times A$$

Dimana :

$$\sigma_{\text{bahan}} = \text{Tegangan ijin bahan}$$

$$A = \text{Luas Penampang}$$

#### a. Untuk dinding sumuran ( K-250)

$$\begin{aligned} Qd_1 &= (0,85 \times fc') \times \left\{ \left( \frac{1}{4} \times \pi \times D_{\text{luar}}^2 \right) - \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \right\} \\ &= (0,85 \times 250) \times \left\{ \left( \frac{1}{4} \times \pi \times 100^2 \right) - \left( \frac{1}{4} \times \pi \times 50^2 \right) \right\} \\ &= 1251093,750 \text{ Kg} \\ &= 1251,093750 \text{ ton} \end{aligned}$$

#### b. Untuk beton cyclop ( K-350)

$$\begin{aligned} Qd_2 &= (0,85 \times fc') \times \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \\ &= (0,85 \times 250) \times \left( \frac{1}{4} \times \pi \times 50^2 \right) \\ &= 583843,750 \text{ Kg} \\ &= 583,843750 \text{ ton} \end{aligned}$$

Jadi daya dukung berdasarkan kekuatan bahan adalah :

$$\begin{aligned} Qd &= Qd_1 + Qd_2 \\ &= 1251,093750 + 583,843750 \\ &= 1834,938 \text{ ton} \end{aligned}$$

2. Ditinjau dari daya dukung berdasarkan data SPT, data yang dipakai adalah data tanah pada S2.

$$Q_u = (40 \times N_b \times A_p)$$

Dimana :

$N_b$  = Nilai SPT pada ujung tiang

$A_p$  = Luas dasar tiang

$$= \frac{1}{4} \times \pi \times D_{\text{luar}}^2$$

$$= \frac{1}{4} \times \pi \times 1,00^2$$

$$= 0,785 \text{ m}^2$$

$$Q_u = 40 \times 38 \times 0,785$$

$$= 1177,500 \text{ ton}$$

$$Q_a = \frac{Q_{\text{tiang}}}{3}$$

$$= \frac{1177,500}{3}$$

$$= 392,500 \text{ ton}$$

## Daya Dukung Pondasi Sumuran Dalam Kelompok

Dari hasil analisa struktur dengan program staad pro diperoleh :

$$\Sigma V_u = 306000 \text{ Kg} = 306 \text{ ton}$$

$$n = \frac{\Sigma V_u}{Q_u}$$

$$= \frac{306}{1177,500}$$

$$= 0,260 \approx 1 \text{ buah}$$

$$P_u = \Sigma V + \text{berat sendiri tiang}$$

Berat sendiri tiang :

1. Untuk dinding sumuran (K-250)

$$\begin{aligned} P_1 &= \left( \frac{1}{4} \times \pi \times D_{\text{luar}}^2 \right) - \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \times D_f \times \gamma_{\text{beton}} \times \text{jumlah} \\ &\quad \text{tiang} \\ &= \left\{ \left( \frac{1}{4} \times \pi \times 1,0^2 \right) - \left( \frac{1}{4} \times \pi \times 0,5^2 \right) \right\} \times 6,0 \times 2,4 \times 1 \\ &= 8,478 \text{ ton} \end{aligned}$$

2. Untuk beton cyclop (K-350)

$$\begin{aligned} P_2 &= \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \times D_f \times \gamma_{\text{beton cyclop}} \times \text{jumlah tiang} \\ &= \left\{ \left( \frac{1}{4} \times \pi \times 1,0^2 \right) - \left( \frac{1}{4} \times \pi \times 0,5^2 \right) \right\} \times 6,0 \times 2,2 \times 1 \end{aligned}$$

$$= 2,591 \text{ ton}$$

Total berat sendiri pondasi sumuran :

$$P_{\text{tiang}} = P1 + P2$$

$$= 8,478 + 2,591$$

$$= 11,069 \text{ ton}$$

$$P_u = \Sigma V + \text{berat sendiri tiang}$$

$$= 306 + 11,069$$

$$= 317,069 \text{ ton} < Q_{\text{total}} = 349,500 \text{ ton} \dots\dots\dots(\text{aman})$$

#### 4.5.2 Perencanaan Pondasi Sumuran Tipe 2 pada Join 3751

$$V : 283 \text{ ton} ; M_x : 2,59 \text{ tm} ; M_z : 2,03 \text{ tm}$$

$$\text{Diameter Sumuran } (D_{\text{luar}}) = 100 \text{ cm} = 1,00 \text{ m}$$

$$\text{Diameter Sumuran } (D_{\text{dalam}}) = 50 \text{ cm} = 0,50 \text{ m}$$

$$\text{Luas Penampang } (A_{\text{tiang}}) = \frac{1}{4} \cdot \pi \cdot D_{\text{luar}}^2 - \frac{1}{4} \cdot \pi \cdot D_{\text{dalam}}^2$$

$$= \frac{1}{4} \cdot \pi \cdot 100^2 - \frac{1}{4} \cdot \pi \cdot 50^2$$

$$= 5887,5 \text{ cm}^2 = 5,8875 \text{ m}^2$$

$$\text{Angka Keamanan (SF)} = 3$$

$$\text{Data Sondir} = \text{Titik S}_2$$

### Daya Dukung Pondasi Sumuran

#### 1. Ditinjau dari daya dukung bahan

$$Q_d = \sigma_{\text{bahan}} \times A$$

Dimana :

$$\sigma_{\text{bahan}} = \text{Tegangan ijin bahan}$$

$$A = \text{Luas Penampang}$$

##### a. Untuk dinding sumuran ( K-250)

$$\begin{aligned} Q_{d_1} &= (0,85 \times f_c') \times \left\{ \left( \frac{1}{4} \times \pi \times D_{\text{luar}}^2 \right) - \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \right\} \\ &= (0,85 \times 250) \times \left\{ \left( \frac{1}{4} \times \pi \times 100^2 \right) - \left( \frac{1}{4} \times \pi \times 50^2 \right) \right\} \\ &= 1251093,750 \text{ Kg} \\ &= 1251,093750 \text{ ton} \end{aligned}$$

##### b. Untuk beton cyclop ( K-350)

$$\begin{aligned} Q_{d_2} &= (0,85 \times f_c') \times \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \\ &= (0,85 \times 250) \times \left( \frac{1}{4} \times \pi \times 50^2 \right) \\ &= 583843,750 \text{ Kg} \\ &= 583,843750 \text{ ton} \end{aligned}$$

Jadi daya dukung berdasarkan kekuatan bahan adalah :

$$Q_d = Q_{d_1} + Q_{d_2}$$

$$= 1251,093750 + 583,843750$$

$$= 1834,938 \text{ ton}$$

2. Ditinjau dari daya dukung berdasarkan data SPT, data yang dipakai adalah data tanah pada S2.

$$Q_u = (40 \times N_b \times A_p)$$

Dimana :

$N_b$  = Nilai SPT pada ujung tiang

$A_p$  = Luas dasar tiang

$$= \frac{1}{4} \times \pi \times D_{\text{luar}}^2$$

$$= \frac{1}{4} \times \pi \times 1,00^2$$

$$= 0,785 \text{ m}^2$$

$$Q_u = 40 \times 38 \times 0,785$$

$$= 1177,500 \text{ ton}$$

$$Q_a = \frac{Q_{1\text{tiang}}}{3}$$

$$= \frac{1177,500}{3}$$

$$= 392,500 \text{ ton}$$

## Daya Dukung Pondasi Sumuran Dalam Kelompok

Dari hasil analisa struktur dengan program staad pro diperoleh :

$$\Sigma V_u = 283000 \text{Kg} = 283 \text{ ton}$$

$$n = \frac{\Sigma V_u}{Q_u}$$
$$= \frac{283}{1177,500}$$

$$= 0,240 \approx 1 \text{ buah}$$

$$P_u = \Sigma V + \text{berat sendiri tiang}$$

Berat sendiri tiang :

1. Untuk dinding sumuran (K-250)

$$P1 = \left( \frac{1}{4} \times \pi \times D_{\text{luar}}^2 \right) - \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \times D_f \times \gamma_{\text{beton}} \times \text{jumlah tiang}$$
$$= \left\{ \left( \frac{1}{4} \times \pi \times 1,0^2 \right) - \left( \frac{1}{4} \times \pi \times 0,5^2 \right) \right\} \times 6,0 \times 2,4 \times 1$$
$$= 8,478 \text{ ton}$$

2. Untuk beton cyclop (K-350)

$$P2 = \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \times D_f \times \gamma_{\text{beton cyclop}} \times \text{jumlah tiang}$$
$$= \left\{ \left( \frac{1}{4} \times \pi \times 1,0^2 \right) - \left( \frac{1}{4} \times \pi \times 0,5^2 \right) \right\} \times 6,0 \times 2,2 \times 1$$

$$= 2,591 \text{ ton}$$

Total berat sendiri pondasi sumuran :

$$P_{\text{tiang}} = P1 + P2$$

$$= 8,478 + 2,591$$

$$= 11,069 \text{ ton}$$

$$P_u = \Sigma V + \text{berat sendiri tiang}$$

$$= 283 + 11,069$$

$$= 294,069 \text{ ton} < Q_{\text{total}} = 349,500 \text{ ton} \dots\dots\dots(\text{aman})$$

#### 4.5.3 Perencanaan Pondasi Sumuran Tipe 3 pada Join 3750

$$V : 57 \text{ ton} ; M_x : 0,02371 \text{ tm} ; M_z : 0,21872 \text{ tm}$$

$$\text{Diameter Sumuran } (D_{\text{luar}}) = 100 \text{ cm} = 1,00 \text{ m}$$

$$\text{Diameter Sumuran } (D_{\text{dalam}}) = 50 \text{ cm} = 0,50 \text{ m}$$

$$\text{Luas Penampang } (A_{\text{tiang}}) = \frac{1}{4} \cdot \pi \cdot D_{\text{luar}}^2 - \frac{1}{4} \cdot \pi \cdot D_{\text{dalam}}^2$$

$$= \frac{1}{4} \cdot \pi \cdot 100^2 - \frac{1}{4} \cdot \pi \cdot 50^2$$

$$= 5887,5 \text{ cm}^2 = 5,8875 \text{ m}^2$$



Angka Keamanan (SF) = 3

Data Sondir = Titik S<sub>2</sub>

### Daya Dukung Pondasi Sumuran

#### 1. Ditinjau dari daya dukung bahan

$$Q_d = \sigma_{\text{bahan}} \times A$$

Dimana :

$\sigma_{\text{bahan}}$  = Tegangan ijin bahan

A = Luas Penampang

##### a. Untuk dinding sumuran ( K-250)

$$\begin{aligned} Q_{d1} &= (0,85 \times f_c') \times \left\{ \left( \frac{1}{4} \times \pi \times D_{\text{luar}}^2 \right) - \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \right\} \\ &= (0,85 \times 250) \times \left\{ \left( \frac{1}{4} \times \pi \times 100^2 \right) - \left( \frac{1}{4} \times \pi \times 50^2 \right) \right\} \\ &= 1251093,750 \text{ Kg} \\ &= 1251,093750 \text{ ton} \end{aligned}$$

##### b. Untuk beton cyclop ( K-350)

$$\begin{aligned} Q_{d2} &= (0,85 \times f_c') \times \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \\ &= (0,85 \times 250) \times \left( \frac{1}{4} \times \pi \times 50^2 \right) \\ &= 583843,750 \text{ Kg} \\ &= 583,843750 \text{ ton} \end{aligned}$$

Jadi daya dukung berdasarkan kekuatan bahan adalah :

$$\begin{aligned} Q_d &= Q_{d1} + Q_{d2} \\ &= 1251,093750 + 583,843750 \\ &= 1834,938 \text{ ton} \end{aligned}$$

2. Ditinjau dari daya dukung berdasarkan data SPT, data yang dipakai adalah data tanah pada S2.

$$Q_u = (40 \times N_b \times A_p)$$

Dimana :

$N_b$  = Nilai SPT pada ujung tiang

$A_p$  = Luas dasar tiang

$$= \frac{1}{4} \times \pi \times D_{\text{luar}}^2$$

$$= \frac{1}{4} \times \pi \times 1,00^2$$

$$= 0,785 \text{ m}^2$$

$$Q_u = 40 \times 38 \times 0,785$$

$$= 1177,500 \text{ ton}$$

$$Q_a = \frac{Q_{\text{tiang}}}{3}$$

$$= \frac{1177,500}{3} = 392,500 \text{ ton}$$

## Daya Dukung Pondasi Sumuran Dalam Kelompok

Dari hasil analisa struktur dengan program Staad Pro diperoleh :

$$\Sigma V_u = 57000 \text{ Kg} = 57 \text{ ton}$$

$$\begin{aligned} n &= \frac{\Sigma V_u}{Q_u} \\ &= \frac{57}{1177,500} \\ &= 0,048 \approx 1 \text{ buah} \end{aligned}$$

$$P_u = \Sigma V + \text{berat sendiri tiang}$$

Berat sendiri tiang :

### 1. Untuk dinding sumuran (K-250)

$$\begin{aligned} P1 &= \left( \frac{1}{4} \times \pi \times D_{\text{luar}}^2 \right) - \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \times D_f \times \gamma_{\text{beton}} \times \text{jumlah} \\ &\quad \text{tiang} \\ &= \left\{ \left( \frac{1}{4} \times \pi \times 1,0^2 \right) - \left( \frac{1}{4} \times \pi \times 0,5^2 \right) \right\} \times 6,0 \times 2,4 \times 1 \\ &= 8,478 \text{ ton} \end{aligned}$$

### 2. Untuk beton cyclop (K-350)

$$\begin{aligned} P2 &= \left( \frac{1}{4} \times \pi \times D_{\text{dalam}}^2 \right) \times D_f \times \gamma_{\text{beton cyclop}} \times \text{jumlah tiang} \\ &= \left\{ \left( \frac{1}{4} \times \pi \times 1,0^2 \right) - \left( \frac{1}{4} \times \pi \times 0,5^2 \right) \right\} \times 6,0 \times 2,2 \times 1 \end{aligned}$$

$$= 2,591 \text{ ton}$$

Total berat sendiri pondasi sumuran :

$$P_{\text{tiang}} = P1 + P2$$

$$= 8,478 + 2,591$$

$$= 11,069 \text{ ton}$$

$$P_u = \Sigma V + \text{berat sendiri tiang}$$

$$= 57 + 11,069$$

$$= 68,069 \text{ ton} < Q_{\text{total}} = 349,500 \text{ ton} \dots\dots\dots(\text{aman})$$

#### 4.6 Perhitungan Penulangan Pondasi Sumuran

##### 4.6.1 Penulangan Poer Pondasi Sumuran Tipe 1

Mencari beban tiang bor yang menerima beban V dan M

$$P_{\text{max}} = \frac{P_u}{n} \pm \frac{M_y \cdot X_{\text{max}}}{n_y \cdot \Sigma X^2} \pm \frac{M_x \cdot Y_{\text{max}}}{n_x \cdot \Sigma Y^2}$$

Dimana :

$$P_u = 317,069 \text{ ton} \quad ; M_x = 2,04 \text{ tm}$$

$$n = 1 \text{ tiang} \quad ; M_y = 1,97 \text{ tm}$$

$$n_y = 1 \text{ tiang}$$

$$n_x = 1 \text{ tiang}$$

$$\Sigma X^2 = \text{Jumlah kuadrat absis tiang}$$

$$= (0^2) + (0^2) + = 0 \text{ m}^2$$

$$\Sigma Y^2 = \text{Jumlah kuadrat ordinat tiang}$$

$$= (0^2) + (0^2) + = 0 \text{ m}^2$$

Sehingga :

$$(x_1 = 0 ; y_1 = 0)$$

$$P_1 = \frac{317,069}{1} \pm \frac{1,97 \times (0)}{1 \times 0} \pm \frac{2,04 \times (0)}{1 \times 0}$$

$$= 317,069 \text{ ton}$$

$$P_{\max} = 317,069 \text{ ton} < Q_a \text{ tiang} = 994,940 \text{ ton} \dots\dots\dots (\text{aman})$$

## Perhitungan Momen

Pada bagian bawah poer diasumsikan sebagai plat jalur jepit pada bagian sisinya. Dari table P2.3 (Pelat : Stiglet/Wipel : 209).

**Tabel 4.8 : Pelat Stiglet/Wipel**

Y/L	0	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1,0
Mye	0,32	0,31	0,30	0,28	0,25	0,21	0,18	0,14	0,09	0,05	0

### 4.6.1.1 Perhitungan penulangan poer arah X

$$(Y/L) = \frac{0}{0,75+0,75} = 0$$
$$= 0,32$$

$$Mu = Mx = P \times mye$$

$$= 317,069 \times 0,32$$

$$= 101,462 \text{ tm}$$

$$= 101462 \text{ kgm} = 101462 \times 10^4 \text{ Nmm}$$

$$Mu = \frac{101462 \times 10^4}{1,5} = 67641,280 \times 10^4 \text{ Nmm}$$

$$Mn = \frac{Mu}{\phi} = \frac{67641,280 \times 10^4}{0,8} = 84551,600 \times 10^4 \text{ Nmm}$$

$$dx = 800 - 75 - (1/2 \times 22) = 714 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$Rn = \frac{Mn}{b \times dx^2} = \frac{84551,600 \times 10^4}{1000 \times 714^2} = 1,659 \text{ Mpa}$$

$$m = \frac{fy}{0,85 \times fc'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times fc'}{fy} \times \beta \times \frac{600}{600 + fy} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{min} = \frac{1,4}{fy} = \frac{1,4}{400} = 0,004$$

$$\rho_{perlu} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times Rn \times m}{fy}} = \frac{1}{18,824} \times 1 -$$

$$\sqrt{1 - \frac{2 \times 1,659 \times 18,824}{400}}$$

$$= 0,001$$

$\rho_{perlu} < \rho_{min}$ , maka yang  $\rho_{min} = 0,004$

$$As \text{ perlu} = \rho_{min} \times b \times dx$$

$$= 0,004 \times 1000 \times 714 = 2499 \text{ mm}^2$$

Direncanakan menggunakan tulangan pokok D 22

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d^2} = \frac{2499}{0,25 \times \pi \times 22^2} = 8,405 \approx 8 \text{ tul.}$$

$$\text{As ada} = n \times 0,25 \times \pi \times 22^2$$

$$= 8 \times 0,25 \times \pi \times 22^2$$

$$= 3039,520 \text{ mm}^2 > \text{As perlu} = 2499 \text{ mm}^2 \dots\dots\dots(\text{OK})$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{8} = 125,000 \approx 125 \text{ mm}$$

**Jadi digunakan tulangan tarik arah x 8D22**

Perhitungan tulangan tekan

$$\text{As tekan} = 20\% \times \text{As perlu}$$

$$= 20\% \times 2499$$

$$= 499,800 \text{ mm}^2$$

Direncanakan menggunakan tul.tekan D19

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{499,800}{0,25 \times \pi \times 19^2} = 1,764 \approx 3 \text{ tul.}$$

$$\text{As ada} = n \times 0,25 \times \pi \times 19^2$$



$$= 3 \times 0,25 \times \pi \times 19^2$$

$$= 850,155 \text{ mm}^2 > \text{As tekan} = 499,800 \text{ mm}^2 \dots \text{(OK)}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{3} = 333,33 \approx 250 \text{ mm}$$

**Jadi digunakan tulangan tekan arah x 3D19**

#### 4.6.1.2 Perhitungan Penulangan Poer Arah Y

$$M_u = M_y = P \times m_y$$

$$= 317,069 \times 0,32$$

$$= 101,462 \text{ tm}$$

$$= 101462 \text{ kgm} = 101462 \times 10^4 \text{ Nmm}$$

$$M_u = \frac{101462 \times 10^4}{1,5} = 67641,280 \times 10^4 \text{ Nmm}$$

$$M_n = \frac{M_u}{\phi} = \frac{67641,280 \times 10^4}{0,8} = 84551,600 \times 10^4 \text{ Nmm}$$

$$d_y = 800 - 75 - 22 - (1/2 \times 22) = 692 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$R_n = \frac{M_n}{b \times d^2} = \frac{84551,600 \times 10^4}{1000 \times 692^2} = 1,766 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{\max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,004$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times 1 -$$

$$\sqrt{1 - \frac{2 \times 1,766 \times 18,824}{400}}$$

$$= 0,005$$

$\rho_{\min} < \rho_{\text{perlu}}$ , maka yang  $\rho_{\text{perlu}} = 0,005$

$$\text{As perlu} = \rho_{\min} \times b \times d_y$$

$$= 0,005 \times 1000 \times 692$$

$$= 3193,300 \text{ mm}^2$$

Direncanakan menggunakan tulangan pokok D 22

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d_y^2} = \frac{3193,300}{0,25 \times \pi \times 22^2} = 8,405 \approx 9 \text{ tul.}$$

$$\begin{aligned}
 \text{As ada} &= n \times 0,25 \times \pi \times 22^2 \\
 &= 9 \times 0,25 \times \pi \times 22^2 \\
 &= 3419,460 \text{ mm}^2 > \text{As perlu} = 3193,300 \text{ mm}^2 \dots \text{(OK)}
 \end{aligned}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{9} = 111,111 \approx 120 \text{ mm.}$$

**Jadi digunakan tulangan tarik arah y 9D22**

Perhitungan tulangan tekan

$$\begin{aligned}
 \text{As tekan} &= 20\% \times \text{As perlu} \\
 &= 20\% \times 3193,300 \\
 &= 638,660 \text{ mm}^2
 \end{aligned}$$

Direncanakan menggunakan tul.tekan D19

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{638,660}{0,25 \times \pi \times 19^2} = 2,254 \approx 3 \text{ tul.}$$

$$\begin{aligned}
 \text{As ada} &= n \times 0,25 \times \pi \times 19^2 \\
 &= 3 \times 0,25 \times \pi \times 19^2 \\
 &= 850,155 \text{ mm}^2 > \text{As tekan} = 638,660 \text{ mm}^2 \dots \text{(OK)}
 \end{aligned}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{3} = 333,333 \approx 250 \text{ mm}$$

**Jadi digunakan tulangan tekan arah y 3D19**

#### **4.6.1.3 Kontrol Geser Pons (Gaya Geser Dua Arah Sumbu)**

- **Geser Pons terhadap Kolom**

Diketahui :

$$V_u = 306 \text{ ton} = 306000 \text{ kg}$$

Tinggi efektif (d)

$$d = \text{tebal poer} - \text{tebal selimut beton} - \frac{1}{2} \text{ diameter tulangan}$$

terluar

$$= 800 - 75 - \frac{1}{2} \cdot 22$$

$$= 714 \text{ mm}$$

$$\text{Dimensi kolom (c)} = 30/40$$

- **Keliling bidang kritis geser pons (bo)**

$$b_o = 4 \cdot [(c_1 + d) + (c_2 + d)]$$

$$= 4 \cdot [(300 + 714) + (400 + 714)]$$

$$= 4256 \text{ mm}$$

- **Kuat geser beton maksimum**

$$V_c = \frac{\sqrt{f_c'}}{3} \times b_o \times d$$

$$= \frac{\sqrt{25}}{3} \times 4256 \times 714$$

$$= 5064640 \text{ kg}$$

$$\emptyset V_c = 0,6 \times V_c$$

$$= 0,6 \times 5064640$$

$$= 3038784 \text{ kg}$$

Maka  $V_u = 306000 \text{ kg} < \emptyset V_c = 3038784 \text{ kg} \dots\dots\dots(\text{OK})$

Karena  $V_u < \emptyset V_c$ , maka tidak diperlukan tulangan geser terhadap kolom dan poer aman terhadap geser pons akibat kolom.

#### 4.6.2 Penulangan poer Pondasi Sumuran Tipe 2

Mencari beban tiang bor yang menerima beban V dan M

$$P_{\max} = \frac{P_u}{n} \pm \frac{M_y \cdot X_{\max}}{n_y \cdot \Sigma X^2} \pm \frac{M_x \cdot Y_{\max}}{n_x \cdot \Sigma Y^2}$$

Dimana :

$$P_u = 294,069 \text{ ton} \quad ; \quad M_x = 2,59 \text{ tm}$$

$$n = 1 \text{ tiang} \quad ; \quad M_y = 2,03 \text{ tm}$$

$$n_y = 1 \text{ tiang}$$

$$n_x = 1 \text{ tiang}$$

$\Sigma X^2$  = Jumlah kuadrat absis tiang

$$= (0^2) + (0^2) + = 0 \text{ m}^2$$

$\Sigma Y^2$  = Jumlah kuadrat ordinat tiang

$$= (0^2) + (0^2) + = 0 \text{ m}^2$$

Sehingga :

$$(x_1 = 0 ; y_1 = 0)$$

$$P1 = \frac{294,069}{1} \pm \frac{2,03 \times (0)}{1 \times 0} \pm \frac{2,59 \times (0)}{1 \times 0}$$

$$= 294,069 \text{ ton}$$

$$P_{\max} = 317,069 \text{ ton} < Q_a \text{ tiang} = 994,940 \text{ ton} \dots\dots\dots(\text{aman})$$

#### Perhitungan Momen

Pada bagian bawah poer diasumsikan sebagai plat jalur jepit pada bagian sisinya. Dari table P2.3 (Pelat : Stiglet/Wipel : 209).

**Tabel 4.9 : Pelat Stiglet/Wipel**

Y/L	0	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1,0
Mye	0,32	0,31	0,30	0,28	0,25	0,21	0,18	0,14	0,09	0,05	0

#### 4.6.2.1 Perhitungan penulangan poer arah X

$$(Y/L) = \frac{0}{0,75+0,75} = 0$$

$$= 0,32$$

$$M_u = M_x = P \times m_y$$

$$= 294,069 \times 0,32$$

$$= 94,102 \text{ tm}$$

$$= 94102 \text{ kgm} = 94102 \times 10^4 \text{ Nmm}$$

$$M_u = \frac{94120 \times 10^4}{1,5} = 62734,613 \times 10^4 \text{ Nmm}$$

$$M_n = \frac{M_u}{\phi} = \frac{62734,613 \times 10^4}{0,8} = 78418,267 \times 10^4 \text{ Nmm}$$

$$d_x = 800 - 75 - (1/2 \times 22) = 714 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$R_n = \frac{M_n}{b \times d_x^2} = \frac{78418,267 \times 10^4}{1000 \times 714^2} = 1,538 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{\max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,0035$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times 1 -$$

$$\sqrt{1 - \frac{2 \times 1,538 \times 18,824}{400}}$$

$$= 0,0040$$

$\rho_{\min} < \rho_{\text{perlu}}$ , maka yang  $\rho_{\text{perlu}} = 0,0040$

$$\begin{aligned} \text{As perlu} &= \rho_{\min} \times b \times d_x \\ &= 0,0040 \times 1000 \times 714 \\ &= 2853,034 \text{ mm}^2 \end{aligned}$$

Direncanakan menggunakan tulangan pokok D 22

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d^2} = \frac{2853,034}{0,25 \times \pi \times 22^2} = 7,509 \approx 8 \text{ tul.}$$

$$\begin{aligned} \text{As ada} &= n \times 0,25 \times \pi \times 22^2 \\ &= 8 \times 0,25 \times \pi \times 22^2 \end{aligned}$$



$$= 3039,520 \text{ mm}^2 > A_s \text{ perlu} = 2853,034 \text{ mm}^2 \dots \text{(OK)}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{8} = 125,000 \approx 125 \text{ mm}$$

**Jadi digunakan tulangan tarik arah x 8D22**

Perhitungan tulangan tekan

$$A_s \text{ tekan} = 20\% \times A_s \text{ perlu}$$

$$= 20\% \times 2843,034$$

$$= 570,607 \text{ mm}^2$$

Direncanakan menggunakan tul.tekan D19

$$\text{Jumlah tul. (n)} = \frac{A_s \text{ tekan}}{0,25 \times \pi \times d^2} = \frac{570,607}{0,25 \times \pi \times 19^2} = 2,083 \approx 3 \text{ tul.}$$

$$A_s \text{ ada} = n \times 0,25 \times \pi \times 19^2$$

$$= 3 \times 0,25 \times \pi \times 19^2$$

$$= 850,155 \text{ mm}^2 > A_s \text{ tekan} = 570,607 \text{ mm}^2 \dots \text{(OK)}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{3} = 333,33 \approx 250 \text{ mm}$$

**Jadi digunakan tulangan tekan arah x 3D19**

#### 4.6.2.2 Penulangan Poer Arah Y

$$M_u = M_y = P \times m_y$$

$$= 294,069 \times 0,32$$

$$= 94,102 \text{ tm}$$

$$= 94102 \text{ kgm} = 94102 \times 10^4 \text{ Nmm}$$

$$M_u = \frac{94102 \times 10^4}{1,5} = 62734,613 \times 10^4 \text{ Nmm}$$

$$M_n = \frac{M_u}{\phi} = \frac{62734,613 \times 10^4}{0,8} = 78418,267 \times 10^4 \text{ Nmm}$$

$$d_y = 800 - 75 - 22 - (1/2 \times 22) = 692 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$R_n = \frac{M_n}{b \times d^2} = \frac{78418,267 \times 10^4}{1000 \times 692^2} = 1,638 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{\max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,0035$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times 1 -$$

$$\sqrt{1 - \frac{2 \times 1,638 \times 18,824}{400}}$$

$$= 0,0043$$

$\rho_{\min} < \rho_{\text{perlu}}$ , maka yang  $\rho_{\text{perlu}} = 0,0043$

$$\text{As perlu} = \rho_{\text{perlu}} \times b \times d_y$$

$$= 0,0043 \times 1000 \times 692$$

$$= 2951,512 \text{ mm}^2$$

Direncanakan menggunakan tulangan pokok D 22

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d_y^2} = \frac{2951,512}{0,25 \times \pi \times 22^2} = 7,768 \approx 8 \text{ tul.}$$

$$\text{As ada} = n \times 0,25 \times \pi \times 22^2$$

$$= 8 \times 0,25 \times \pi \times 22^2$$

$$= 3039,520 \text{ mm}^2 > \text{As perlu} = 2951,512 \text{ mm}^2 \dots \text{(OK)}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{8} = 125,00 \approx 125 \text{ mm.}$$

**Jadi digunakan tulangan tarik arah y 8D22**

Perhitungan tulangan tekan

$$\begin{aligned}\text{As tekan} &= 20\% \times \text{As perlu} \\ &= 20\% \times 2951,512 \\ &= 590,302 \text{ mm}^2\end{aligned}$$

Direncanakan menggunakan tul.tekan D19

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{590,302}{0,25 \times \pi \times 19^2} = 2,083 \approx 3 \text{ tul.}$$

$$\begin{aligned}\text{As ada} &= n \times 0,25 \times \pi \times 19^2 \\ &= 3 \times 0,25 \times \pi \times 19^2 \\ &= 850,155 \text{ mm}^2 > \text{As tekan} = 590,302 \text{ mm}^2 \dots \text{(OK)}\end{aligned}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{3} = 333,333 \approx 250 \text{ mm}$$

**Jadi digunakan tulangan tekan arah y 3D19**

#### 4.6.2.3 Kontrol Geser Pons (Gaya Geser Dua Arah Sumbu)

- Geser Pons terhadap Kolom

Diketahui :

$$V_u = 283 \text{ ton} = 283000 \text{ kg}$$

Tinggi efektif (d)

$$d = \text{tebal poer} - \text{tebal selimut beton} - \frac{1}{2} \text{ diameter tulangan}$$

terluar

$$= 800 - 75 - \frac{1}{2} \cdot 22$$

$$= 714 \text{ mm}$$

Dimensi kolom (c) = 30/40

- Keliling bidang kritis geser pons (bo)

$$b_o = 4 \cdot [(c_1 + d) + (c_2 + d)]$$

$$= 4 \cdot [(300 + 714) + (400 + 714)]$$

$$= 4256 \text{ mm}$$

- Kuat geser beton maksimum

$$V_c = \frac{\sqrt{f_c'}}{3} \times b_o \times d$$

$$= \frac{\sqrt{25}}{3} \times 4256 \times 714$$

$$= 5064640 \text{ kg}$$

$$\phi V_c = 0,6 \times V_c$$

$$= 0,6 \times 5064640$$

$$= 3038784 \text{ kg}$$

Maka  $V_u = 283000 \text{ kg} < \phi V_c = 3038784 \text{ kg} \dots \dots \dots \text{(OK)}$

Karena  $V_u < \phi V_c$ , maka tidak diperlukan tulangan geser terhadap kolom dan poer aman terhadap geser pons akibat kolom.

#### 4.6.3 Pondasi Sumuran Tipe 3

Mencari beban tiang bor yang menerima beban V dan M

$$P_{max} = \frac{P_u}{n} \pm \frac{M_y \cdot X_{max}}{n_y \cdot \Sigma X^2} \pm \frac{M_x \cdot Y_{max}}{n_x \cdot \Sigma Y^2}$$

Dimana :

$$P_u = 68,069 \text{ ton} \quad ; M_x = 0,02371 \text{ tm}$$

$$n = 1 \text{ tiang} \quad ; M_y = 0,21872 \text{ tm}$$

$$n_y = 1 \text{ tiang}$$

$$n_x = 1 \text{ tiang}$$

$$\Sigma X^2 = \text{Jumlah kuadrat absis tiang}$$

$$= (0^2) + (0^2) + = 0 \text{ m}^2$$

$$\Sigma Y^2 = \text{Jumlah kuadrat ordinat tiang}$$

$$= (0^2) + (0^2) + = 0 \text{ m}^2$$

Sehingga :

$$(x_1 = 0 ; y_1 = 0)$$

$$P_1 = \frac{68,069}{1} \pm \frac{0,02371 \times (0)}{1 \times 0} \pm \frac{0,21872 \times (0)}{1 \times 0}$$

$$= 68,069 \text{ ton}$$

$$P_{\max} = 68,069 \text{ ton} < Q_a \text{ tiang} = 994,940 \text{ ton} \dots\dots\dots(\text{aman})$$

#### Perhitungan Momen

Pada bagian bawah poer diasumsikan sebagai plat jalur jepit pada bagian sisinya. Dari table P2.3 (Pelat : Stiglet/Wipel : 209).

**Tabel 4.10 : Pelat Stiglet/Wipel**

Y/L	0	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1,0
Mye	0,32	0,31	0,30	0,28	0,25	0,21	0,18	0,14	0,09	0,05	0

#### 4.6.3.1 Perhitungan penulangan poer arah X

$$(Y/L) = \frac{0}{0,75+0,75} = 0$$

$$= 0,32$$

$$M_u = M_x = P \times m_y e$$

$$= 68,069 \times 0,32$$

$$= 21,782 \text{ tm}$$

$$= 21782 \text{ kgm} = 21782 \times 10^4 \text{ Nmm}$$

$$M_u = \frac{21782 \times 10^4}{1,5} = 14521,280 \times 10^4 \text{ Nmm}$$

$$M_n = \frac{M_u}{\phi} = \frac{14521,280 \times 10^4}{0,8} = 18151,600 \times 10^4 \text{ Nmm}$$

$$d_x = 800 - 75 - (1/2 \times 22) = 714 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$R_n = \frac{M_n}{b \times d_x^2} = \frac{18151,600 \times 10^4}{1000 \times 714^2} = 0,356 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{\max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,004$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times 1 -$$



$$\sqrt{1 - \frac{2 \times 0,356 \times 18,824}{400}}$$

$$= 0,001$$

$\rho_{\text{perlu}} < \rho_{\text{min}}$ , maka yang  $\rho_{\text{min}} = 0,004$

$$\text{As perlu} = \rho_{\text{min}} \times b \times dx$$

$$= 0,004 \times 1000 \times 714$$

$$= 2499 \text{ mm}^2$$

Direncanakan menggunakan tulangan pokok D 22

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d^2} = \frac{2499}{0,25 \times \pi \times 22^2} = 6,755 \approx 7 \text{ tul.}$$

$$\text{As ada} = n \times 0,25 \times \pi \times 22^2$$

$$= 7 \times 0,25 \times \pi \times 22^2$$

$$= 2659,580 \text{ mm}^2 > \text{As perlu} = 2499 \text{ mm}^2 \dots\dots\dots(\text{OK})$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{7} = 142,875 \approx 150 \text{ mm}$$

**Jadi digunakan tulangan tarik arah x 7D22**

Perhitungan tulangan tekan

$$\begin{aligned}\text{As tekan} &= 20\% \times \text{As perlu} \\ &= 20\% \times 2499 \\ &= 499,800 \text{ mm}^2\end{aligned}$$

Direncanakan menggunakan tul.tekan D19

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{499,800}{0,25 \times \pi \times 19^2} = 1,764 \approx 3 \text{ tul.}$$

$$\begin{aligned}\text{As ada} &= n \times 0,25 \times \pi \times 19^2 \\ &= 3 \times 0,25 \times \pi \times 19^2 \\ &= 850,155 \text{ mm}^2 > \text{As tekan} = 499,800 \text{ mm}^2 \dots \text{(OK)}\end{aligned}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{3} = 333,33 \approx 250 \text{ mm}$$

**Jadi digunakan tulangan tekan arah x 3D19**

#### 4.6.3.2 Penulangan Poer Arah Y

$$\begin{aligned}\text{Mu} = \text{My} &= P \times \text{mye} \\ &= 68,069 \times 0,32\end{aligned}$$

$$= 21,782 \text{ tm}$$

$$= 21782 \text{ kgm} = 21782 \times 10^4 \text{ Nmm}$$

$$M_u = \frac{21782 \times 10^4}{1,5} = 14521,280 \times 10^4 \text{ Nmm}$$

$$M_n = \frac{M_u}{\phi} = \frac{14521,280 \times 10^4}{0,8} = 18151,600 \times 10^4 \text{ Nmm}$$

$$d_y = 800 - 75 - 22 - (\frac{1}{2} \times 22) = 692 \text{ mm}$$

$$b = 1,00 \text{ m} = 1000 \text{ mm}$$

$$R_n = \frac{M_n}{b \times d^2} = \frac{18151,600 \times 10^4}{1000 \times 692^2} = 0,39 \text{ Mpa}$$

$$m = \frac{f_y}{0,85 \times f_c'} = \frac{400}{0,85 \times 25} = 18,824$$

$$\rho_b = \frac{0,85 \times f_c'}{f_y} \times \beta \times \frac{600}{600 + f_y} = \frac{0,85 \times 25}{400} \times 0,85 \times \frac{600}{600 + 400}$$

$$= 0,0271$$

$$\rho_{\max} = 0,75 \times \rho_b = 0,75 \times 0,0271 = 0,0203$$

$$\rho_{\min} = \frac{1,4}{f_y} = \frac{1,4}{400} = 0,004$$

$$\rho_{\text{perlu}} = \frac{1}{m} \times 1 - \sqrt{1 - \frac{2 \times R_n \times m}{f_y}} = \frac{1}{18,824} \times$$

$$1 - \sqrt{1 - \frac{2 \times 0,379 \times 18,824}{400}}$$

$$= 0,001$$

$\rho_{\text{perlu}} < \rho_{\text{min}}$ , maka yang  $\rho_{\text{min}} = 0,004$

$$\text{As perlu} = \rho_{\text{min}} \times b \times d$$

$$= 0,004 \times 1000 \times 692$$

$$= 2422 \text{ mm}^2$$

Direncanakan menggunakan tulangan pokok D 22

$$\text{Jumlah tul. (n)} = \frac{\text{As perlu}}{0,25 \times \pi \times d^2} = \frac{2422}{0,25 \times \pi \times 22^2} = 6,375 \approx 7 \text{ tul.}$$

$$\text{As ada} = n \times 0,25 \times \pi \times 22^2$$

$$= 7 \times 0,25 \times \pi \times 22^2$$

$$= 2659,580 \text{ mm}^2 > \text{As perlu} = 2422 \text{ mm}^2 \dots\dots\dots(\text{OK})$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{7} = 142,875 \approx 150 \text{ mm.}$$

**Jadi digunakan tulangan tarik arah y 7D22**

### Perhitungan tulangan tekan

$$\begin{aligned}\text{As tekan} &= 20\% \times \text{As perlu} \\ &= 20\% \times 2422 \\ &= 484,400 \text{ mm}^2\end{aligned}$$

### Direncanakan menggunakan tul.tekan D19

$$\text{Jumlah tul. (n)} = \frac{\text{As tekan}}{0,25 \times \pi \times d^2} = \frac{484,400}{0,25 \times \pi \times 19^2} = 1,709 \approx 3 \text{ tul.}$$

$$\begin{aligned}\text{As ada} &= n \times 0,25 \times \pi \times 19^2 \\ &= 3 \times 0,25 \times \pi \times 19^2 \\ &= 850,155 \text{ mm}^2 > \text{As tekan} = 484,400 \text{ mm}^2 \dots \text{(OK)}\end{aligned}$$

$$\text{Jarak (s)} = \frac{b}{n} = \frac{1000}{3} = 333,333 \approx 250 \text{ mm}$$

**Jadi digunakan tulangan tekan arah y 3D19**

#### 4.6.3.3 Kontrol Geser Pons (Gaya Geser Dua Arah Sumbu)

- Geser Pons Terhadap Kolom

Diketahui :

$$V_u = 57 \text{ ton} = 57000 \text{ kg}$$

Tinggi efektif (d)

d = tebal poer – tebal selimut beton –  $\frac{1}{2}$  diameter tulangan  
terluar

$$= 800 - 75 - \frac{1}{2} \cdot 22$$

$$= 714 \text{ mm}$$

Dimensi kolom (c) = 30/40

- Keliling bidang kritis geser pons (bo)

$$b_o = 4 \cdot [(c_1 + d) + (c_2 + d)]$$

$$= 4 \cdot [(300 + 714) + (400 + 714)]$$

$$= 4256 \text{ mm}$$

- Kuat geser beton maksimum

$$V_c = \frac{\sqrt{f_c'}}{3} \times b_o \times d$$

$$= \frac{\sqrt{25}}{3} \times 4256 \times 714$$

$$= 5064640 \text{ kg}$$

$$\phi V_c = 0,6 \times V_c$$

$$= 0,6 \times 5064640$$

$$= 3038784 \text{ kg}$$

$$\text{Maka } V_u = 57000 \text{ kg} < \phi V_c = 3038784 \text{ kg} \dots\dots\dots(\text{OK})$$

Karena  $V_u < \phi V_c$ , maka tidak diperlukan tulangan geser terhadap kolom dan poer aman terhadap geser pons akibat kolom.

#### **4.7 Perhitungan Tulangan Pondasi Sumuran**

##### **4.7.1 Perhitungan Tulangan Pondasi Sumuran Tipe 1**

Perhitungan pondasi sumuran diasumsikan seperti perhitungan kolom bulat.

Data Perencanaan :

- $P_u$  = 317,069 ton
- $M_{maks} (My)$  = 101,426 ton
- Mutu beton ( $f_c'$ ) = 25 Mpa
- Mutu baja tulangan ( $f_y$ ) = 400 Mpa
- Diameter luar sumuran = 100 cm = 1000 mm
- Diameter dalam sumuran = 50 cm = 500 mm
- Tebal selimut = 75 mm

1. Tebal efektif selimut beton terpusat tulangan terluar.

Dicoba menggunakan tulangan pokok D22 dan tulangan sengkang D19

$$\begin{aligned}d' &= \text{tebal selimut beton} + \emptyset \text{ sengkang} + \left(\frac{1}{2} D_{\text{tul.pokok}}\right) \\ &= 75 + 19 + \left(\frac{1}{2} 22\right) \\ &= 105 \text{ mm}\end{aligned}$$

$$\begin{aligned}d_{\text{efektif}} &= D_{\text{luar}} - (2 \times d') \\ &= 1000 - (2 \times 105) \\ &= 790 \text{ mm}\end{aligned}$$

2. Luas penampang sumuran ( $A_g$ )

$$\begin{aligned}A_g &= \left(\frac{1}{4} \times \pi \times D_{\text{luar}}^2\right) - \left(\frac{1}{4} \times \pi \times D_{\text{dalam}}^2\right) \\ &= \left(\frac{1}{4} \times \pi \times 1000^2\right) - \left(\frac{1}{4} \times \pi \times 500^2\right) \\ &= 588750 \text{ mm}^2\end{aligned}$$

3. Luas tulangan penampang baja ( $A_{st}$ )

Rencana penulangan dengan perkiraan luas tulangan pokok adalah 3% dari luas tiang.

$$\begin{aligned}\bullet \quad A_{st} &= 3\% \times A_g \\ &= 3\% \times 588750 \\ &= 17662,500 \text{ mm}^2\end{aligned}$$



- Jumlah Tulangan (n)

$$\begin{aligned}
 n &= \frac{A_{st}}{\frac{1}{4} \times \pi \times D_{tul}^2} \\
 &= \frac{17662,500}{\frac{1}{4} \times \pi \times 22^2} \\
 &= 46,49 \approx 47 \text{ buah}
 \end{aligned}$$

- $A_{sada} = n \times \frac{1}{4} \times \pi \times D_{tul}^2$ 

$$\begin{aligned}
 &= 47 \times \frac{1}{4} \times \pi \times 22^2 \\
 &= 17857,180 \text{ mm}^2 > A_{st} = 17662,500 \text{ mm}^2 \text{(OK)}
 \end{aligned}$$

- $A_s = A_{s'} = 0,5 \times A_{st}$ 

$$\begin{aligned}
 &= 0,5 \times 17662,500 \\
 &= 8831,250 \text{ mm}^2
 \end{aligned}$$

- Jarak tulangan pokok (s)

$$\begin{aligned}
 s &= \frac{\pi \times d_{efektif}}{n} \\
 &= \frac{\pi \times 790}{47} = 52,779 \approx 55 \text{ mm}
 \end{aligned}$$

#### 4. Pemeriksaan beban ultimate beton ( $P_{ub}$ ) dan momen ultimate beton ( $M_{ub}$ )

- Tebal penampang segi empat ekuivalen

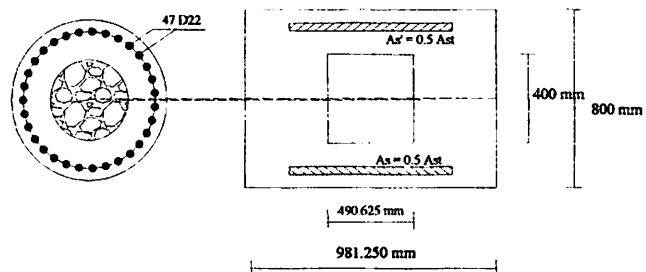
$$\begin{aligned}
 t_{ekl} &= 0,8 \times D_{luar} \\
 &= 0,8 \times 1000 \\
 &= 800 \text{ mm}
 \end{aligned}$$

$$\begin{aligned}
 t_{ek2} &= 0,8 \times D_{dalam} \\
 &= 0,8 \times 500 \\
 &= 400 \text{ mm}
 \end{aligned}$$

- Lebar penampang segi empat ekuivalen

$$\begin{aligned}
 l_{ek1} &= \frac{1/4 \times \pi \times D_{luar}^2}{t_{ek1}} \\
 &= \frac{1/4 \times \pi \times 1000^2}{800} \\
 &= 981,250 \text{ mm}
 \end{aligned}$$

$$\begin{aligned}
 l_{ek2} &= \frac{1/4 \times \pi \times D_{dalam}^2}{t_{ek2}} \\
 &= \frac{1/4 \times \pi \times 500^2}{400} \\
 &= 490,625 \text{ mm}
 \end{aligned}$$



**Gambar 4.23 : Ekuivalen Penampang Bulat ke Segi Empat**

- Pemeriksaan P terhadap beban seimbang

- Jarak tulangan tarik terhadap tepi terluar beton ( $d_b$ )

$$\begin{aligned} d_b &= t_{ekl} - \text{tebal selimut efektif (d')} \\ &= 800 - 105 \\ &= 695 \text{ mm} \end{aligned}$$

- Jarak serat tekan terluar ke garis netral ( $c_b$ )

$$\begin{aligned} c_b &= \frac{600 \times d_b}{600 + f_y} \\ &= \frac{600 \times 695}{600 + 400} \\ &= 417 \text{ mm} \end{aligned}$$

- Lebar daerah tekan ( $a_b$ )

$$\begin{aligned} a_b &= \beta \times c_b \\ &= 0,85 \times 417 \\ &= 354,450 \text{ mm} \end{aligned}$$

- Tegangan tekan tulangan baja ( $f_s'$ )

$$\begin{aligned} f_s' &= \frac{600 \times (c_b - d')}{c_b} \\ &= \frac{600 \times (417 - 105)}{417} \\ &= 448,921 \text{ Mpa} > f_y = 400 \text{ Mpa} \end{aligned}$$

- **Beban ultimate beton ( $P_{ub}$ )**

$$\begin{aligned}
 P_{ub} &= [(0,85 \times f_c' \times a_b \times l_{ek1}) + (A_s' \times f_s') - (A_s \times f_y)] \times 10^{-3} \\
 &= [(0,85 \times 25 \times 354,450 \times 981,250) + (8831,250 \\
 &\quad \times 448,921) - (8831,250 \times 400)] \times 10^{-3} \\
 &= 14887,869 \text{ kN}
 \end{aligned}$$

**Momen ultimate beton ( $M_{ub}$ )**

$$\begin{aligned}
 M_{ub} &= [(0,85 \times f_c' \times l_{ek1} \times a_b \times \{\frac{t_{ek1}}{2} - (\frac{1}{2} \times a_b)\}) + \\
 &\quad (A_s' \times f_s' \times (\frac{1}{2} \times (d_{eff} - d''))) + (A_s \times f_s \times (\frac{1}{2} \\
 &\quad (d_{eff} - d''))] \times 10^{-6} \\
 &= [(0,85 \times 25 \times 918,250 \times 354,450 \times \{\frac{800}{2} - (\frac{1}{2} \times \\
 &\quad 354,450)\}) + (8831,250 \times 400 \times (\frac{1}{2} \times (790 - \\
 &\quad 105)) + (8831,250 \times 400 \times (\frac{1}{2} \times (790 - 105)))] \times \\
 &\quad 10^{-6} \\
 &= 4066,256 \text{ kNm}
 \end{aligned}$$

- **Eksentrisitas beton ( $e_b$ )**

$$\begin{aligned}
 e_b &= \frac{M_{ub}}{P_{ub}} \\
 &= \frac{4066,256}{14887,869} \\
 &= 0,273 \text{ m}
 \end{aligned}$$

- Eksentrisitas beban (e)

$$\begin{aligned}
 e &= \frac{M}{P_{\max}} \\
 &= \frac{101,462}{317,069} \\
 &= 0,320 \text{ m}
 \end{aligned}$$

#### 4.7.1.1 Memeriksa Kekuatan Penampang Pondasi Sumuran

- Rasio penulangan memanjang ( $\rho_s$ )

$$\begin{aligned}
 \rho_s &= \frac{A_{st}}{A_g} \\
 &= \frac{17662,500}{588750} = 0,03
 \end{aligned}$$

- Lebar sumuran efektif (Ds)

$$\begin{aligned}
 D_s &= D_{\text{luar}} - (2 \times d') \\
 &= 1000 - (2 \times 105) \\
 &= 790 \text{ mm}
 \end{aligned}$$

$$\begin{aligned}
 m &= \frac{f_y}{0,85 \times f_c'} \\
 &= \frac{400}{0,85 \times 25} \\
 &= 18,824
 \end{aligned}$$

- Persamaan untuk penampang pondasi tiang bor dengan hancur tarik ( $P_n$ )

$$\begin{aligned}
 P_n &= 0,85 \times f_c \times h^2 \times \\
 &\sqrt{\left(\frac{0,85 \times e}{h} - 0,38\right)^2 + \frac{\rho_s \times m \times D_s}{2,50 \times h} - \left(\frac{0,85 \times e}{h} - 0,38\right)} \\
 &= 0,85 \times 25 \times 1000^2 \times \\
 &\sqrt{\left(\frac{0,85 \times 0,320}{1000} - 0,38\right)^2 + \frac{0,03 \times 18,824 \times 790}{2,50 \times 1000} - \left(\frac{0,85 \times 0,320}{1000} - 0,38\right)} \\
 &= 9143184 \text{ kg} = 91431,84 \text{ ton}
 \end{aligned}$$

- Kekuatan sumuran

$$\begin{aligned}
 \phi P_n &= 0,7 \times P_n \\
 &= 0,7 \times 91431,84 \\
 &= 6400,229 \text{ ton} > P_u = 317,069 \text{ ton}
 \end{aligned}$$

Dengan demikian perencanaan penampang pondasi sumuran memenuhi persyaratan sehingga ukuran dan tulangan dapat digunakan.

#### 4.7.1.2 Perencanaan Tulangan Spiral

Data perencanaan :

$$f_c' = 35 \text{ Mpa}$$

$$f_y = 400 \text{ Mpa}$$

$$D_{tul.} = 12 \text{ mm}$$

Luas penampang kotor sumuran ( $A_g$ )

$$\begin{aligned} A_g &= \left(\frac{1}{4} \times \pi \times d_{luar}^2\right) - \left(\frac{1}{4} \times \pi \times d_{dalam}^2\right) \\ &= \left(\frac{1}{4} \times \pi \times 1000^2\right) - \left(\frac{1}{4} \times \pi \times 500^2\right) \\ &= 588750 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} A_{S \text{ spiral}} &= \frac{1}{4} \times \pi \times D_{tul}^2 \\ &= \frac{1}{4} \times \pi \times 12^2 = 113,04 \text{ mm}^2 \end{aligned}$$

Diameter inti sumuran ( $D_c$ )

$$\begin{aligned} D_c &= D_{tiang} - (2 \times \text{selimut beton}) \\ &= 1000 - (2 \times 75) \\ &= 850 \text{ mm} \end{aligned}$$

Luas penampang inti sumuran ( $A_c$ )

$$\begin{aligned} A_c &= \left(\frac{1}{4} \times \pi \times D_c^2\right) - \left(\frac{1}{4} \times \pi \times D_{\text{dalam}}^2\right) \\ &= \left(\frac{1}{4} \times \pi \times 850^2\right) - \left(\frac{1}{4} \times \pi \times 500^2\right) \\ &= 370913 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} \rho_s &= 0,45 \times \left(\frac{A_g}{A_c} - 1\right) \times \left(\frac{f_{c'}}{f_y}\right) \\ &= 0,45 \times \left(\frac{588750}{370913} - 1\right) \times \left(\frac{35}{400}\right) \\ &= 0,023 \end{aligned}$$

- Jarak antar sengkang spiral

$$\begin{aligned} S_{\text{maks}} &= \frac{4 \times A_{s\text{spiral}} \times (D_c - D_{\text{tul}})}{D_c^2 \times \rho_s} \\ &= \frac{4 \times 283,385 \times (850 - 12)}{850^2 \times 0,023} \\ &= 40,13 \text{ mm} \approx 40 \text{ mm} \end{aligned}$$

Dari perhitungan penulangan pondasi sumuran, maka digunakan tulangan pokok 47D22 dan tulangan spiral Ø12-40.



#### 4.7.2 Perhitungan Tulangan Pondasi Sumuran Tipe 2

Perhitungan pondasi sumuran diasumsikan seperti perhitungan kolom bulat.

Data Perencanaan :

- $P_u$  = 294,069 ton
- $M_{maks}$  (My) = 94,102 ton
- Mutu beton ( $f_c'$ ) = 25 Mpa
- Mutu baja tulangan ( $f_y$ ) = 400 Mpa
- Diameter luar sumuran = 100 cm = 1000 mm
- Diameter dalam sumuran = 50 cm = 500 mm
- Tebal selimut = 75 mm

1. Tebal efektif selimut beton terpusat tulangan terluar.

Dicoba menggunakan tulangan pokok D22 dan tulangan sengkang D19

$$\begin{aligned}d' &= \text{tebal selimut beton} + \emptyset \text{ sengkang} + \left(\frac{1}{2} D_{\text{tul.pokok}}\right) \\ &= 75 + 19 + \left(\frac{1}{2} 22\right) \\ &= 105 \text{ mm}\end{aligned}$$

$$\begin{aligned}d_{\text{efektif}} &= D_{\text{luar}} - (2 \times d') \\ &= 1000 - (2 \times 105)\end{aligned}$$

$$= 790 \text{ mm}$$

2. Luas penampang sumuran ( $A_g$ )

$$\begin{aligned} A_g &= \left(\frac{1}{4} \times \pi \times D_{\text{luar}}^2\right) - \left(\frac{1}{4} \times \pi \times D_{\text{dalam}}^2\right) \\ &= \left(\frac{1}{4} \times \pi \times 1000^2\right) - \left(\frac{1}{4} \times \pi \times 500^2\right) \\ &= 588750 \text{ mm}^2 \end{aligned}$$

3. Luas tulangan penampang baja ( $A_{st}$ )

Rencana penulangan dengan perkiraan luas tulangan pokok adalah 3% dari luas tiang.

- $A_{st} = 3\% \times A_g$   
 $= 3\% \times 588750$   
 $= 17662,500 \text{ mm}^2$

- Jumlah Tulangan ( $n$ )

$$\begin{aligned} n &= \frac{A_{st}}{\frac{1}{4} \times \pi \times D_{\text{tul}}^2} \\ &= \frac{17662,500}{\frac{1}{4} \times \pi \times 22^2} \\ &= 46,49 \approx 47 \text{ buah} \end{aligned}$$

- $A_{sada} = n \times \frac{1}{4} \times \pi \times D_{\text{tul}}^2$   
 $= 47 \times \frac{1}{4} \times \pi \times 22^2$   
 $= 17857,180 \text{ mm}^2 > A_{st} = 17662,500 \text{ mm}^2 \text{(OK)}$

- $A_s = A_s' = 0,5 \times A_{st}$   
 $= 0,5 \times 17662,500$   
 $= 8831,250 \text{ mm}^2$

- Jarak tulangan pokok (s)

$$s = \frac{\pi \times d_{\text{efektif}}}{n}$$

$$= \frac{\pi \times 790}{47} = 52,779 \approx 55 \text{ mm}$$

4. Pemeriksaan beban ultimate beton ( $P_{ub}$ ) dan momen ultimate beton ( $M_{ub}$ )

- Tebal penampang segi empat ekuivalen

$$t_{ek1} = 0,8 \times D_{\text{luar}}$$

$$= 0,8 \times 1000$$

$$= 800 \text{ mm}$$

$$t_{ek2} = 0,8 \times D_{\text{dalam}}$$

$$= 0,8 \times 500$$

$$= 400 \text{ mm}$$

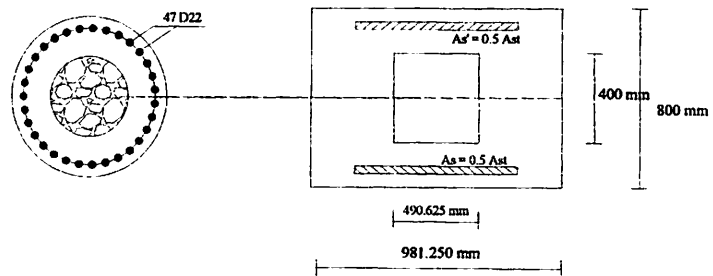
- Lebar penampang segi empat ekuivalen

$$l_{ek1} = \frac{1/4 \times \pi \times D_{\text{luar}}^2}{t_{ek1}}$$

$$= \frac{1/4 \times \pi \times 1000^2}{800}$$

$$= 981,250 \text{ mm}$$

$$\begin{aligned}
 l_{ek2} &= \frac{1/4 \times \pi \times D_{dalam}^2}{t_{ek2}} \\
 &= \frac{1/4 \times \pi \times 500^2}{400} \\
 &= 490,625 \text{ mm}
 \end{aligned}$$



**Gambar 4.24 : Ekvivalen Penampang Bulat ke Segi Empat**

- pemeriksaan P terhadap beban seimbang
  - Jarak tulangan tarik terhadap tepi terluar beton ( $d_b$ )

$$\begin{aligned}
 d_b &= t_{ek1} - \text{tebal selimut efektif (d')} \\
 &= 800 - 105 \\
 &= 695 \text{ mm}
 \end{aligned}$$

- Jarak serat tekan terluar ke garis netral ( $c_b$ )

$$\begin{aligned}
 c_b &= \frac{600 \times d_b}{600 + f_y} \\
 &= \frac{600 \times 695}{600 + 400} \\
 &= 417 \text{ mm}
 \end{aligned}$$

➤ Lebar daerah tekan ( $a_b$ )

$$\begin{aligned} a_b &= \beta \times c_b \\ &= 0,85 \times 417 \\ &= 354,450 \text{ mm} \end{aligned}$$

• Tegangan tekan tulangan baja ( $f_s'$ )

$$\begin{aligned} f_s' &= \frac{600 \times (c_b - d')}{c_b} \\ &= \frac{600 \times (417 - 105)}{417} \\ &= 448,921 \text{ Mpa} > f_y = 400 \text{ Mpa} \end{aligned}$$

• Beban ultimate beton ( $P_{ub}$ )

$$\begin{aligned} P_{ub} &= [(0,85 \times f_c' \times a_b \times l_{ek1}) + (A_s' \times f_s') - (A_s \times f_y)] \times 10^{-3} \\ &= [(0,85 \times 25 \times 354,450 \times 981,250) + (8831,250 \\ &\quad \times 448,921) - (8831,250 \times 400)] \times 10^{-3} \\ &= 14887,869 \text{ kN} \end{aligned}$$

Momen ultimate beton ( $M_{ub}$ )

$$\begin{aligned} M_{ub} &= [(0,85 \times f_c' \times l_{ek1} \times a_b \times \{\frac{l_{ek1}}{2} - (\frac{1}{2} \times a_b)\}) + \\ &\quad (A_s' \times f_s' \times (\frac{1}{2} \times (d_{eff} - d''))) + (A_s \times f_s \times (\frac{1}{2} \\ &\quad (d_{eff} - d'')))] \times 10^{-6} \\ &= [(0,85 \times 25 \times 918,250 \times 354,450 \times \{\frac{800}{2} - (\frac{1}{2} \times \\ &\quad 354,450)\}) + (8831,250 \times 400 \times (\frac{1}{2} \times (790 - \end{aligned}$$

$$105)) + (8831,250 \times 400 \times (1/2 \times (790 - 105))) \times 10^{-6}$$

$$= 4066,256 \text{ kNm}$$

- Eksentrisitas beton ( $e_b$ )

$$e_b = \frac{M_{ub}}{P_{ub}}$$

$$= \frac{4066,256}{14887,869}$$

$$= 0,273 \text{ m}$$

- Eksentrisitas beban ( $e$ )

$$e = \frac{M}{P_{max}}$$

$$= \frac{94,102}{294,069}$$

$$= 0,320 \text{ m}$$

#### 4.7.2.1 Memeriksa Kekuatan Penampang Pondasi Sumuran

- Rasio penulangan memanjang ( $\rho_s$ )

$$\rho_s = \frac{A_{st}}{A_g}$$

$$= \frac{17662,500}{588750}$$

$$= 0,03$$

- Lebar sumuran efektif ( $D_s$ )

$$D_s = D_{luar} - (2 \times d')$$

$$= 1000 - (2 \times 105)$$

$$= 790 \text{ mm}$$

$$m = \frac{f_y}{0,85 \times f_c'}$$

$$= \frac{400}{0,85 \times 25}$$

$$= 18,824$$

- Persamaan untuk penampang pondasi tiang bor dengan hancur tarik ( $P_n$ )

$$P_n = 0,85 \times f_c \times h^2 \times$$

$$\sqrt{\left(\frac{0,85 \times e}{h} - 0,38\right)^2 + \frac{\rho_s \times m \times D_s}{2,50 \times h}} - \left(\frac{0,85 \times e}{h} - 0,38\right)$$

$$= 0,85 \times 25 \times 1000^2 \times$$

$$\sqrt{\left(\frac{0,85 \times 0,320}{1000} - 0,38\right)^2 + \frac{0,03 \times 18,824 \times 790}{2,50 \times 1000}} - \left(\frac{0,85 \times 0,320}{1000} - 0,38\right)$$

$$= 9143184 \text{ kg} = 91431,84 \text{ ton}$$

- Kekuatan sumuran

$$\phi P_n = 0,7 \times P_n$$

$$= 0,7 \times 91431,84$$

$$= 6400,229 \text{ ton} > P_u = 294,069 \text{ ton}$$

Dengan demikian perencanaan penampang pondasi sumuran memenuhi persyaratan sehingga ukuran dan tulangan dapat digunakan.

#### 4.7.2.2 Perencanaan Tulangan Spiral

Data perencanaan :

$$f_c' = 35 \text{ Mpa}$$

$$f_y = 400 \text{ Mpa}$$

$$D_{tul.} = 16 \text{ mm}$$

Luas penampang kotor sumuran ( $A_g$ )

$$\begin{aligned} A_g &= \left(\frac{1}{4} \times \pi \times d_{luar}^2\right) - \left(\frac{1}{4} \times \pi \times d_{dalam}^2\right) \\ &= \left(\frac{1}{4} \times \pi \times 1000^2\right) - \left(\frac{1}{4} \times \pi \times 500^2\right) \\ &= 588750 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} A_{s \text{ spiral}} &= \frac{1}{4} \times \pi \times D_{tul}^2 \\ &= \frac{1}{4} \times \pi \times 12^2 = 113,04 \text{ mm}^2 \end{aligned}$$



Diameter inti sumuran ( $D_c$ )

$$\begin{aligned} D_c &= D_{\text{tiang}} - (2 \times \text{selimut beton}) \\ &= 1000 - (2 \times 75) \\ &= 850 \text{ mm} \end{aligned}$$

Luas penampang inti sumuran ( $A_c$ )

$$\begin{aligned} A_c &= \left(\frac{1}{4} \times \pi \times D_c^2\right) - \left(\frac{1}{4} \times \pi \times D_{\text{dalam}}^2\right) \\ &= \left(\frac{1}{4} \times \pi \times 850^2\right) - \left(\frac{1}{4} \times \pi \times 500^2\right) \\ &= 370913 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} \rho_s &= 0,45 \times \left(\frac{A_g}{A_c} - 1\right) \times \left(\frac{f_{c'}}{f_y}\right) \\ &= 0,45 \times \left(\frac{588750}{370913} - 1\right) \times \left(\frac{35}{400}\right) \\ &= 0,023 \end{aligned}$$

- Jarak antar sengkang spiral

$$\begin{aligned} S_{\text{maks}} &= \frac{4 \times A_{\text{spiral}} \times (D_c - D_{\text{tul}})}{D_c^2 \times \rho_s} \\ &= \frac{4 \times 283,385 \times (850 - 12)}{850^2 \times 0,023} \\ &= 40,13 \text{ mm} \approx 40 \text{ mm} \end{aligned}$$

Dari perhitungan penulangan pondasi sumuran, maka digunakan tulangan pokok 47D22 dan tulangan spiral Ø12-40.

#### 4.7.3 Penulangan Pondasi Sumuran Tipe 3

Perhitungan pondasi sumuran diasumsikan seperti perhitungan kolom bulat.

Data Perencanaan :

- $P_u$  = 68,069 ton
- $M_{maks}$  (My) = 21,782 ton
- Mutu beton ( $f_c'$ ) = 25 Mpa
- Mutu baja tulangan ( $f_y$ ) = 400 Mpa
- Diameter luar sumuran = 100 cm = 1000 mm
- Diameter dalam sumuran = 50 cm = 500 mm
- Tebal selimut = 75 mm

1. Tebal efektif selimut beton terpusat tulangan terluar.

Dicoba menggunakan tulangan pokok D22 dan tulangan sengkang D19

$$\begin{aligned}d' &= \text{tebal selimut beton} + \emptyset \text{ sengkang} + \left(\frac{1}{2} D_{tul.pokok}\right) \\ &= 75 + 19 + \left(\frac{1}{2} 22\right)\end{aligned}$$

$$= 105 \text{ mm}$$

$$\begin{aligned} d_{\text{efektif}} &= D_{\text{luar}} - (2 \times d') \\ &= 1000 - (2 \times 105) \\ &= 790 \text{ mm} \end{aligned}$$

2. Luas penampang sumuran ( $A_g$ )

$$\begin{aligned} A_g &= \left(\frac{1}{4} \times \pi \times D_{\text{luar}}^2\right) - \left(\frac{1}{4} \times \pi \times D_{\text{dalam}}^2\right) \\ &= \left(\frac{1}{4} \times \pi \times 1000^2\right) - \left(\frac{1}{4} \times \pi \times 500^2\right) \\ &= 588750 \text{ mm}^2 \end{aligned}$$

3. Luas tulangan penampang baja ( $A_{st}$ )

Rencana penulangan dengan perkiraan luas tulangan pokok adalah 3% dari luas tiang.

- $A_{st} = 3\% \times A_g$   
 $= 3\% \times 588750$   
 $= 17662,500 \text{ mm}^2$

- Jumlah Tulangan ( $n$ )

$$\begin{aligned} n &= \frac{A_{st}}{\frac{1}{4} \times \pi \times D_{\text{tul}}^2} \\ &= \frac{17662,500}{\frac{1}{4} \times \pi \times 22^2} \\ &= 46,49 \approx 47 \text{ buah} \end{aligned}$$

- $A_{S_{\text{ada}}} = n \times \frac{1}{4} \times \pi \times D_{\text{tul}}^2$   
 $= 47 \times \frac{1}{4} \times \pi \times 22^2$

$$= 17857,180 \text{ mm}^2 > A_{st} = 17662,500 \text{ mm}^2 \text{(OK)}$$

- $A_s = A_{s'} = 0,5 \times A_{st}$   
 $= 0,5 \times 17662,500$   
 $= 8831,250 \text{ mm}^2$

- Jarak tulangan pokok (s)

$$s = \frac{\pi \times d_{\text{efektif}}}{n}$$
$$= \frac{\pi \times 790}{47} = 52,779 \approx 55 \text{ mm}$$

4. Pemeriksaan beban ultimate beton ( $P_{ub}$ ) dan momen ultimate beton ( $M_{ub}$ )

- Tebal penampang segi empat ekuivalen

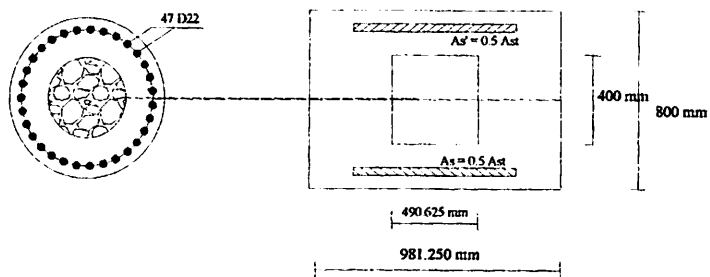
$$t_{ek1} = 0,8 \times D_{\text{luar}}$$
$$= 0,8 \times 1000$$
$$= 800 \text{ mm}$$

$$t_{ek2} = 0,8 \times D_{\text{dalam}}$$
$$= 0,8 \times 500$$
$$= 400 \text{ mm}$$

- Lebar penampang segi empat ekuivalen

$$l_{ek1} = \frac{1/4 \times \pi \times D_{\text{luar}}^2}{t_{ek1}}$$
$$= \frac{1/4 \times \pi \times 1000^2}{800}$$
$$= 981,250 \text{ mm}$$

$$\begin{aligned}
 I_{ek2} &= \frac{1/4 \times \pi \times D_{dalam}^2}{t_{ek2}} \\
 &= \frac{1/4 \times \pi \times 500^2}{400} \\
 &= 490,625 \text{ mm}
 \end{aligned}$$



**Gambar 4.25 : Ekivalen Penampang Bulat ke Segi Empat**

- Pemeriksaan P terhadap beban seimbang
  - Jarak tulangan tarik terhadap tepi terluar beton ( $d_b$ )

$$\begin{aligned}
 d_b &= t_{ek1} - \text{tebal selimut efektif (d')} \\
 &= 800 - 105 \\
 &= 695 \text{ mm}
 \end{aligned}$$

- Jarak serat tekan terluar ke garis netral ( $c_b$ )

$$\begin{aligned}
 c_b &= \frac{600 \times d_b}{600 + f_y} \\
 &= \frac{600 \times 695}{600 + 400} \\
 &= 417 \text{ mm}
 \end{aligned}$$

➤ Lebar daerah tekan ( $a_b$ )

$$\begin{aligned} a_b &= \beta \times c_b \\ &= 0,85 \times 417 \\ &= 354,450 \text{ mm} \end{aligned}$$

• Tegangan tekan tulangan baja ( $f_s'$ )

$$\begin{aligned} f_s' &= \frac{600 \times (c_b - d')}{c_b} \\ &= \frac{600 \times (417 - 105)}{417} \\ &= 448,921 \text{ Mpa} > f_y = 400 \text{ Mpa} \end{aligned}$$

• Beban ultimate beton ( $P_{ub}$ )

$$\begin{aligned} P_{ub} &= [(0,85 \times f_c' \times a_b \times l_{ek1}) + (A_s' \times f_s') - (A_s \times f_y)] \times 10^{-3} \\ &= [(0,85 \times 25 \times 354,450 \times 981,250) + (8831,250 \times 448,921) - (8831,250 \times 400)] \times 10^{-3} \\ &= 14887,869 \text{ kN} \end{aligned}$$

Momen ultimate beton ( $M_{ub}$ )

$$\begin{aligned} M_{ub} &= [(0,85 \times f_c' \times l_{ek1} \times a_b \times \{\frac{l_{ek1}}{2} - (\frac{1}{2} \times a_b)\}) + (A_s' \times f_s' \times (\frac{1}{2} \times (d_{eff} - d''))) + (A_s \times f_s \times (\frac{1}{2} \times (d_{eff} - d'')))] \times 10^{-6} \\ &= [(0,85 \times 25 \times 918,250 \times 354,450 \times \{\frac{800}{2} - (\frac{1}{2} \times 354,450)\}) + (8831,250 \times 400 \times (\frac{1}{2} \times (790 - \end{aligned}$$

$$105)) + (8831,250 \times 400 \times (1/2 \times (790 - 105))) \times 10^{-6}$$

$$= 4066,256 \text{ kNm}$$

- Eksentrisitas beton ( $e_b$ )

$$e_b = \frac{M_{ub}}{P_{ub}}$$

$$= \frac{4066,256}{14887,869}$$

$$= 0,273 \text{ m}$$

- Eksentrisitas beban ( $e$ )

$$e = \frac{M}{P_{max}}$$

$$= \frac{21,872}{68,069}$$

$$= 0,320 \text{ m}$$

#### 4.7.3.1 Memeriksa Kekuatan Penampang Pondasi Sumuran

- Rasio penulangan memanjang ( $\rho_s$ )

$$\rho_s = \frac{A_{st}}{A_g}$$

$$= \frac{17662,500}{588750}$$

$$= 0,03$$

- Lebar sumuran efektif ( $D_s$ )

$$D_s = D_{luar} - (2 \times d')$$

$$= 1000 - (2 \times 105)$$

$$= 790 \text{ mm}$$

$$m = \frac{f_y}{0,85 \times f_c'}$$

$$= \frac{400}{0,85 \times 25}$$

$$= 18,824$$

- Persamaan untuk penampang pondasi tiang bor dengan hancur tarik ( $P_n$ )

$$P_n = 0,85 \times f_c \times h^2 \times$$

$$\sqrt{\left(\frac{0,85 \times e}{h} - 0,38\right)^2 + \frac{\rho_s \times m \times D_s}{2,50 \times h}} - \left(\frac{0,85 \times e}{h} - 0,38\right)$$

$$= 0,85 \times 25 \times 1000^2 \times$$

$$\sqrt{\left(\frac{0,85 \times 0,320}{1000} - 0,38\right)^2 + \frac{0,03 \times 18,824 \times 790}{2,50 \times 1000}} - \left(\frac{0,85 \times 0,320}{1000} - 0,38\right)$$

$$= 9143184 \text{ kg} = 91431,84 \text{ ton}$$

- Kekuatan sumuran

$$\phi P_n = 0,7 \times P_n$$

$$= 0,7 \times 91431,84$$

$$= 6400,229 \text{ ton} > P_u = 68,069 \text{ ton}$$



Dengan demikian perencanaan penampang pondasi sumuran memenuhi persyaratan sehingga ukuran dan tulangan dapat digunakan.

#### 4.7.3.2 Perencanaan Tulangan Spiral

Data perencanaan :

$$f_c' = 35 \text{ Mpa}$$

$$f_y = 400 \text{ Mpa}$$

$$D_{tul.} = 16 \text{ mm}$$

Luas penampang kotor sumuran ( $A_g$ )

$$\begin{aligned} A_g &= \left(\frac{1}{4} \times \pi \times d_{luar}^2\right) - \left(\frac{1}{4} \times \pi \times d_{dalam}^2\right) \\ &= \left(\frac{1}{4} \times \pi \times 1000^2\right) - \left(\frac{1}{4} \times \pi \times 500^2\right) \\ &= 588750 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} A_{s \text{ spiral}} &= \frac{1}{4} \times \pi \times D_{tul}^2 \\ &= \frac{1}{4} \times \pi \times 12^2 = 113,04 \text{ mm}^2 \end{aligned}$$

Diameter inti sumuran ( $D_c$ )

$$D_c = D_{tiang} - (2 \times \text{selimut beton})$$

$$= 1000 - (2 \times 75)$$

$$= 850 \text{ mm}$$

Luas penampang inti sumuran ( $A_c$ )

$$A_c = \left(\frac{1}{4} \times \pi \times D_c^2\right) - \left(\frac{1}{4} \times \pi \times D_{\text{dalam}}^2\right)$$

$$= \left(\frac{1}{4} \times \pi \times 850^2\right) - \left(\frac{1}{4} \times \pi \times 500^2\right)$$

$$= 370913 \text{ mm}^2$$

$$\rho_s = 0,45 \times \left(\frac{A_g}{A_c} - 1\right) \times \left(\frac{f_c'}{f_y}\right)$$

$$= 0,45 \times \left(\frac{588750}{370913} - 1\right) \times \left(\frac{35}{400}\right)$$

$$= 0,023$$

- Jarak antar sengkang spiral

$$S_{\text{maks}} = \frac{4 \times A_{s\text{spiral}} \times (D_c - D_{\text{tul}})}{D_c^2 \times \rho_s}$$

$$= \frac{4 \times 283,385 \times (850 - 12)}{850^2 \times 0,023}$$

$$= 40,13 \text{ mm} \approx 40 \text{ mm}$$

Dari perhitungan penulangan pondasi sumuran, maka digunakan tulangan pokok 47D22 dan tulangan spiral Ø12-40.

**Tabel 4.11 : Hasil Analisa Perhitungan Perencanaan Pondasi Strauss**

No	PERHITUNGAN	SAT	PONDASI STRAUSS		
			TIPE I	TIPE II	TIPE III
1	Ukuran diameter (D)	cm	50	50	50
2	Kedalaman pondasi	m	6,4	6,4	6,4
3	Jumlah tiang	buah	6	6	2
4	Jarak antar tiang (s)	cm	125	125	100
5	Efisiensi kelompok ( $\eta$ )	-	0,717	0,717	0,835
6	Daya dukung 1 tiang ( $Q_{izin}$ )	Kg	74700,60	74700,60	74700,60
7	Daya dukung tiang kelompok ( $Q_{pg}$ )	Kg	321539	321539	124794
8	Tulangan tarik poer arah x	-	6D19-150	6D19-150	6D19-150
9	Tulangan tarik poer arah y	-	6D19-160	11D19-90	6D19-160
10	Tulangan tekan poer arah x	-	5D16-200	5D16-200	5D16-200
11	Tulangan tekan poer arah y	-	5D16-200	3D16-300	5D16-200
12	Tulangan pokok tiang	-	22D19-45	22D19-45	22D19-45
13	Tulangan spiral tiang	-	Ø12-40	Ø12-40	Ø12-40

**Tabel 4.12 : Hasil Analisa Perhitungan Perencanaan Pondasi Sumuran**

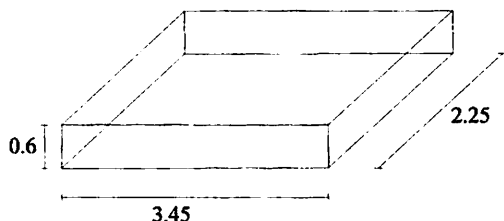
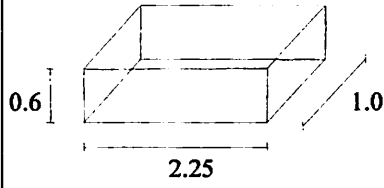
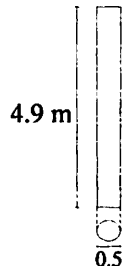
No.	PERHITUNGAN	SAT	PONDASI SUMURAN		
			TIPE I	TIPE II	TIPE III
1	Ukuran diameter luar ( $D_{luar}$ )	cm	100	100	100
2	Ukuran diameter dalam ( $D_{dalam}$ )	cm	50	50	50
3	Kedalaman pondasi	m	6,4	6,4	6,4
4	Jumlah tiang	buah	1	1	1
5	Daya dukung 1 tiang ( $Q_{izin}$ )	ton	392,500	392,500	392,500
6	Tulangan tarik poer arah x	-	8D22-125	8D22-125	7D22-150
7	Tulangan tarik poer arah y	-	9D22-120	8D22-125	7D22-150
8	Tulangan tekan poer arah x	-	3D19-250	3D19-250	3D19-250
9	Tulangan tekan poer arah y	-	3D19-250	3D19-250	3D19-250
10	Tulangan pokok tiang	-	47D22-55	47D22-55	47D22-55
11	Tulangan spiral tiang	-	Ø12-40	Ø12-40	Ø12-40

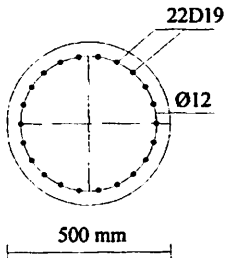
## 4.8 Perhitungan Volume Pondasi Dan Biaya Pengerjaan Pondasi

### 4.8.1 Perhitungan Volume Pondasi

#### 4.8.1.1 Pondasi Tiang Bor (Strauss)

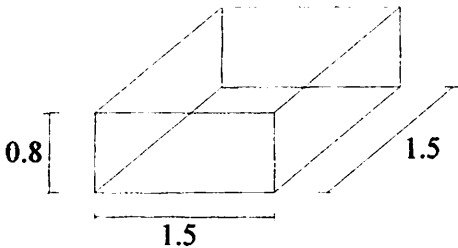
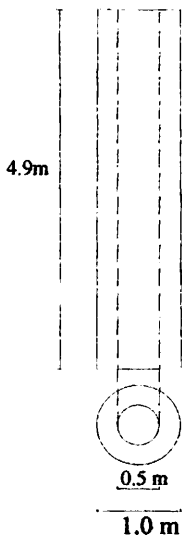
Tabel 4.13 : Perhitungan Volume Pondasi Tiang Bor (Strauss)

No	Uraian	Satuan	Volume
1	<p>Pondasi Tiang Bor</p> <ul style="list-style-type: none"> <li>- Poer Pondasi 1 dan 2</li> </ul>  <p style="text-align: center;">Vol : <math>3,45 \times 2,25 \times 0,6</math></p> <ul style="list-style-type: none"> <li>- Poer Pondasi 3</li> </ul>  <p style="text-align: center;">Vol : <math>2,25 \times 1,00 \times 0,6</math></p> <ul style="list-style-type: none"> <li>- Tiang Pondasi Strauss</li> </ul> 	<p><math>m^3</math></p> <p><math>m^3</math></p>	<p>4,658</p> <p>1,350</p>

	Vol : $\frac{1}{4} \times \pi \times (0,5^2) \times 4,9$	m <sup>3</sup>	0,926
2	<b>Pembesian</b> <ul style="list-style-type: none"> <li>- Tulangan Pondasi</li> </ul>  <ul style="list-style-type: none"> <li>- Tulangan Pokok (ulir) : 22D19</li> <li>- Tulangan Spiral (polos) : 4Ø12</li> <li>- Kawat bendrat</li> <li>- Tulangan Pondasi</li> <li>- Tulangan Pokok (ulir)</li> <li>Tulangan arah x : <ul style="list-style-type: none"> <li>Tekan 5D16</li> <li>Tarik 6D19</li> </ul> </li> <li>Tulangan arah y : <ul style="list-style-type: none"> <li>Tekan 5D16</li> <li>Tarik 6D19</li> </ul> </li> <li>Jumlah volume tul. pokok</li> <li>- Kawat bendrat</li> </ul>	<ul style="list-style-type: none"> <li>Kg</li> <li>Kg</li> <li>Kg</li> <li>Kg</li> <li>Kg</li> <li>Kg</li> <li>Kg</li> <li>Kg</li> <li>Kg</li> </ul>	<ul style="list-style-type: none"> <li>240,394</li> <li>13,054</li> <li>1,200</li> <li>126,980</li> <li>108,840</li> <li>235,820</li> <li>1,200</li> </ul>

#### 4.8.1.2 Pondasi Sumuran (Caisson)

Tabel 4.14 : Perhitungan Volume Pondasi Sumuran

No	Uraian	Satuan	Volume
1	<p>Poer Pondasi Sumuran</p> <ul style="list-style-type: none"> <li>- Poer Pondasi Tipe 1, 2 dan 3</li> </ul>  <p>Vol : <math>1,5 \times 1,5 \times 0,8</math></p> <ul style="list-style-type: none"> <li>- Tiang Pondasi Sumuran</li> </ul>  <p>Vol : <math>\frac{1}{4} \times \pi \times (1,0^2) \times 4,9</math></p> <p>Vol : <math>\frac{1}{4} \times \pi \times (0,5^2) \times 4,9</math></p>	<p><math>m^3</math></p> <p><math>m^3</math></p> <p><math>m^3</math></p>	<p>1,800</p> <p>3,847</p> <p>0,962</p>





## 4.8.2 Perhitungan Biaya Pengerjaan Pondasi

### 4.8.2.1 Pondasi Tiang Bor (Strauss)

Untuk Satu Tiang Pondasi Strauss Tipe 1 dan 2

No	Uraian	Sat	Koef *	Vol	Harga Satuan (Rp)	Jumlah (Rp)
1	<b>Pengerjaan Bored Pile</b>					
	<b>a. Alat Bantu :</b>					
	Crawler Crane	Jam	1.000	0.962	667,500	641,885
	Mesin Bor Pondasi	Jam	1.000	0.962	150,000	144,224
	<b>b. Mobilisasi dan Demobilisasi</b>				3,000,000	3,000,000
	<b>c. Upah Pasang :</b>	hari	1.500	0.962	39,500	56,976
	Pekerja	hari	0.750	0.962	48,500	34,979
Tukang	hari	0.075	0.962	55,500	4,003	
	Operator			<b>a + b + c =</b>	<b>3,882,087</b>	
2	<b>Pembesian Bored Pile / Kg</b>					
	<b>a. Bahan :</b>					
	Tulangan Ulir $f_y = 400$ Mpa	Kg	1.100	240.394	17,700	4,680,471
	Tulangan Polos $f_y = 240$ Mpa	Kg	1.100	13.054	15,600	224,000
	Kawat Beton / Bendrat	Kg	1.100	1.200	20,000	26,400
	<b>b. Upah Pasang :</b>					
	Pekerja Pembesian	hari	1.500	2.546	39,500	150,879
Tukang Besi	hari	0.750	2.546	48,500	92,628	
Mandor Besi	hari	0.075	2.546	70,000	13,396	

	<b>c. Lain-lain</b>					
	Alat Bantu :					
	Tang Pemotong Kawat	buah	0.500	2.546	20,000	25,465
	Beton					
	Alat Pemotong	buah	0.500	2.546	81,000	103,132
	Tulangan					
	Alat Pembekok	buah	0.500	2.546	27,500	35,014
	Tulangan				<b>a + b + c =</b>	<b>5,351,358</b>
<b>3</b>	<b>Pengecoran Bored Pile</b>					
	a. Bahan :					
	Beton Ready Mix K-250	m <sup>3</sup>	1.100	0.962	1,142,150	1,208,150
	b. Upah Pasang :					
	- Pekerja	hari	1.500	0.962	39,500	56,976
	- Tukang	hari	0.750	0.962	48,500	34,979
	- Mandor	hari	0.075	0.962	70,000	5,049
	c. Lain-lain					
	Alat Bantu :					
	- Pipa Tremi dan	unit	0.500	0.962	12,500	6,010
	Cassing Pelindung					
	- Concrete Vibrator	unit	0.500	0.962	50,000	24,041
	- Concrete Pump	unit	0.500	0.962	100,000	48,081
					<b>a + b + c =</b>	<b>1,383,288</b>

Biaya pengerjaan bored pile 1 + 2 + 3 = **Rp 10,616,732**

\*SNI 03-2836-2002 Analisa Biaya Konstruksi Bangunan Gedung

**Pengerjaan Pilecap Pondasi Strauss Tipe 1 dan 2 Per m<sup>3</sup>**

No	Uraian		Koef *	Vol	Harga Satuan (Rp)	Jumlah (Rp)
1	<b>Pengerjaan Pilecap</b>					
	<b>a. Pekerjaan Galian dan Timbunan</b>					
	Pekerja	hari	1.500	6.986	39,500	413,935
	Tukang	hari	0.750	6.986	48,500	254,125
	Mandor	hari	0.075	6.986	70,000	36,678
						<b>704,738</b>
2	<b>Pembesian Pilecap / Kg</b>					
	<b>a. Bahan :</b>					
	Tulangan Ulir fy = 400 Mpa	Kg	1.100	235.820	17,700	4,591,415
	Kawat Beton / Bendrat	Kg	1.100	1.200	20,000	26,400
	<b>b. Upah Pasang :</b>					
	Pekerja Pembesian	hari	1.500	2.370	39,500	140,434
	Tukang Besi	hari	0.750	2.370	48,500	86,216
	Mandor Besi	hari	0.075	2.370	70,000	12,444
	<b>c. Lain-lain</b>					
	<b>Alat Bantu :</b>					
	Tang Pemotong Kawat Beton	buah	0.500	2.370	20,000	23,702
	Alat Pemotong Tulangan	buah	0.500	2.370	81,000	95,993
Alat Pembekok Tulangan	buah	0.500	2.370	27,500	32,590	
				<b>a + b + c =</b>	<b>5,009,195</b>	

3	<b>Pengecoran Pilecap</b>					
	<b>a. Bahan :</b>					
	Beton Ready Mix K-250	m <sup>3</sup>	1.100	4.658	1,142,150	5,851,520
	<b>b. Upah Pasang :</b>					
	- Pekerja	hari	1.500	4.658	39,500	275,957
	- Tukang	hari	0.750	4.658	48,500	169,417
	- Mandor	hari	0.075	4.658	70,000	24,452
	<b>c. Lain-lain</b>					
	<b>Alat Bantu :</b>					
	- Concrete Vibrator	unit	0.500	4.658	50,000	116,438
- Concrete Pump	unit	0.500	4.658	100,000	232,875	
<b>a + b + c =</b>					<b>6,670,658</b>	

**Biaya pengerjaan pilecap 1 + 2 + 3 = Rp 12,232,305**

**Biaya Pengerjaan Pondasi Strauss Tipe 1 dan 2 :**

6 Buah Tiang @ Rp 10,616,732 = Rp 63,700,394

Pilecap = Rp 12,232,305 +

= Rp 75,932,699

**Jumlah Total Biaya Pengerjaan Pondasi Strauss Tipe 1 dan 2 :**

Rp 75,932,699 x 64 = Rp 4,859,692,746

Untuk Satu Tiang Pondasi Strauss Tipe 3

No	Uraian	Sat	Koef *	Vol	Harga Satuan (Rp)	Jumlah (Rp)
1	<b>Pengerjaan Bored Pile</b>					
	<b>a. Alat Bantu :</b>					
	Crawler Crane	Jam	1.000	0.962	667,500	641,885
	Mesin Bor Pondasi	Jam	1.000	0.962	150,000	144,224
	<b>b. Mobilisasi dan Demobilisasi</b>				3,000,000	3,000,000
	<b>c. Upah Pasang :</b>					
	Pekerja	hari	1.500	0.962	39,500	56,976
	Tukang	hari	0.750	0.962	48,500	34,979
Operator	hari	0.075	0.962	55,500	4,003	
				<b>a + b + c =</b>	<b>3,882,087</b>	
2	<b>Pembesian Bored Pile / Kg</b>					
	<b>a. Bahan :</b>					
	Tulangan Ulir $f_y = 400$ Mpa	Kg	1.100	240.394	17,700	4,680,471
	Tulangan Polos $f_y = 240$ Mpa	Kg	1.100	13.054	15,600	224,000
	Kawat Beton / Bendrat	Kg	1.100	1.200	20,000	26,400
	<b>b. Upah Pasang :</b>					
	Pekerja Pembesian	hari	1.500	2.546	39,500	150,879
	Tukang Besi	hari	0.750	2.546	48,500	92,628
	Mandor Besi	hari	0.075	2.546	70,000	13,396
	<b>c. Lain-lain</b>					
<b>Alat Bantu :</b>						
Tang Pemotong Kawat Beton	buah	0.500	2.546	20,000	25,465	
Alat Pemotong	buah	0.500	2.546	81,000	103,132	

	Tulangan Alat Pembekok Tulangan	buah	0.500	2.546	27,500	35,014
					<b>a + b + c =</b>	<b>5,351,358</b>
3	Pengecoran Bored Pile					
	a. Bahan :					
	Beton Ready Mix K-250	m <sup>3</sup>	1.100	0.962	1,142,150	1,208,150
	b. Upah Pasang :					
	- Pekerja	hari	1.500	0.962	39,500	56,976
	- Tukang	hari	0.750	0.962	48,500	34,979
	- Mandor	hari	0.075	0.962	70,000	5,049
	c. Lain-lain					
	Alat Bantu :					
	- Pipa Tremi dan Cassing Pelindung	unit	0.500	0.962	12,500	6,010
	- Concrete Vibrator	unit	0.500	0.962	50,000	24,041
	- Concrete Pump	unit	0.500	0.962	100,000	48,081
					<b>a + b + c =</b>	<b>1,383,288</b>

**Biaya Pengerjaan Bored Pile 1 +2 + 3 = Rp 10,616,732**

**Pengerjaan Pilecap Pondasi Strauss Tipe 3 Per m<sup>3</sup>**

No	Uraian	Sat	Koef *	Vol	Harga Satuan (Rp)	Jumlah (Rp)
1	<b>Pengerjaan Pilecap</b>					
	<b>a. Pekerjaan Galian dan Timbunan</b>					
	Pekerja	hari	1.500	2.025	39,500	119,981
	Tukang	hari	0.750	2.025	48,500	73,659
	Mandor	hari	0.075	2.025	70,000	10,631
						<b>204,272</b>
2	<b>Pembesian Pilecap / Kg</b>					
	<b>a. Bahan :</b>					
	Tulangan Ulir fy = 400 Mpa	Kg	1.100	235.820	17,700	4,680,415
	Kawat Beton / Bendrat	Kg	1.100	1.200	20,000	26,400
	<b>b. Upah Pasang :</b>					
	Pekerja Pembesian	hari	1.500	2.370	39,500	140,434
	Tukang Besi	hari	0.750	2.370	48,500	86,216
	Mandor Besi	hari	0.075	2.370	70,000	12,444
	<b>c. Lain-lain</b>					
	<b>Alat Bantu :</b>					
Tang Pemotong Kawat Beton	buah	0.500	2.370	20,000	23,702	
Alat Pemotong Tulangan	buah	0.500	2.370	81,000	95,993	
Alat Pembekok Tulangan	buah	0.500	2.370	27,500	32,590	
					<b>5,009,195</b>	





#### 4.8.2.2 Pondasi Sumuran (Caisson)

Untuk Satu Tiang Pondasi Sumuran Tipe 1, 2 dan 3

No	Uraian	Sat	Koef *	Vol	Harga Satuan (Rp)	Jumlah (Rp)
1	Pengerjaan Pondasi Sumuran					
	a. Alat Bantu :					
	Crawler Crane	Jam	1.000	3.847	667,500	2,567,873
	Mesin Bor Pondasi	Jam	1.000	3.847	150,000	577,050
	b. Mobilisasi dan Demobilisasi				3,000,000	3,000,000
	c. Upah Pasang :					
	Pekerja	hari	1.500	3.847	39,500	227,935
	Tukang	hari	0.750	3.847	48,500	139,935
	Operator	hari	0.075	3.847	55,500	16,013
				<b>a + b + c =</b>	<b>6,528,805</b>	
2	Pembesian Pondasi Sumuran / Kg					
	a. Bahan :					
	Tulangan Ulir fy = 400 Mpa	Kg	1.100	471.645	17,700	9,182,928
	Tulangan Polos fy = 240 Mpa	Kg	1.100	19.980	15,600	342,875
	Kawat Beton / Bendrat	Kg	1.100	1.200	20,000	26,400
	b. Upah Pasang :					
	Pekerja Pembesian	hari	1.500	4.928	39,500	291,999
	Tukang Besi	hari	0.750	4.928	48,500	179,265
	Mandor Besi	hari	0.075	4.928	70,000	25,873

	c. Lain-lain						
	Alat Bantu :						
	Tang Pemotong	buah	0.500	4.928	20,000	49,283	
	Kawat Beton						
	Alat Pemotong	buah	0.500	4.928	81,000	199,594	
	Tulangan						
	Alat Pembekok	buah	0.500	4.928	27,500	67,763	
	Tulangan				<b>a + b + c =</b>	<b>10,365,962</b>	
3	Pengecoran Pondasi						
	Sumuran						
	a. Bahan :						
	Beton Ready Mix K-250	m <sup>3</sup>	1.100	3.847	1,142,150	4,833,236	
	b. Upah Pasang :						
	- Pekerja	hari	1.500	3.847	39,500	364,696	
	- Tukang	hari	0.750	3.847	48,500	149,264	
	- Mandor	hari	0.075	3.847	70,000	32,046	
	c. Lain-lain						
	Alat Bantu :						
	- Pipa Tremi dan Cassing	unit	0.500	3.847	12,500	24,044	
	Pelindung						
	- Concrete Vibrator	unit	0.500	3.847	50,000	96,175	
	- Concrete Pump	unit	0.500	3.847	100,000	192,350	
					<b>a + b + c =</b>	<b>5,691,810</b>	
4	Beton Siklop D50 cm						
	a. Bahan :						
	Batu Belah 15 cm / 20 cm	m <sup>3</sup>	0.962	0.480	138,400	63,908	
	Semen	Kg	0.962	194.000	1,475	275,276	
	Pasir Beton	m <sup>3</sup>	0.962	0.312	119,175	35,770	

	Kerikil	m <sup>3</sup>	0.962	0.468	137,400	61,860
	b. Upah Pasang :					
	- Pekerja	hari	0.962	3.400	39,500	129,197
	- Tukang Batu	hari	0.962	0.850	48,500	39,658
	- Kepala Tukang	hari	0.962	0.085	52,500	4,293
	- Mandor	hari	0.962	0.170	70,000	11,448
	c. Lain-lain					
	Alat Bantu :					
	- Pipa Tremi dan Cassing Pelindung	unit	0.962	0.500	12,500	6,013
	- Concrete Vibrator	unit	0.962	0.500	50,000	24,050
	- Concrete Pump	unit	0.962	0.500	100,000	48,100
					<b>a + b + e =</b>	<b>699,571</b>

**Pengerjaan Pilecap Pondasi Sumuran Tipe 1, 2 dan 3 Per m<sup>3</sup>**

No	Uraian	Sat	Koef*	Vol	Harga Satuan (Rp)	Jumlah (Rp)
1	Pengerjaan Pilecap					
	a. Pekerjaan Galian dan Timbunan					
	Pekerja	hari	1.500	1.800	39,500	106,650
	Tukang	hari	0.750	1.800	48,500	65,475
	Mandor	hari	0.075	1.800	70,000	9,450
						<b>181,575</b>
2	Pembesian Pilecap / Kg					
	a. Bahan :					
	Tulangan Ulir fy =	Kg	1.100	128.080	17,700	2,493,718

	400 Mpa						
	Kawat Beton /	Kg	1.100	1.200	20,000	26,400	
	Bendrat						
	b. Upah Pasang :						
	Pekerja Pembesian	hari	1.500	1.293	39,500	76,598	
	Tukang Besi	hari	0.750	1.293	48,500	47,026	
	Mandor Besi	hari	0.075	1.293	70,000	6,787	
	c. Lain-lain						
	Alat Bantu :						
	Tang Pemotong	buah	0.500	1.293	20,000	12,928	
	Kawat Beton						
	Alat Pemotong	buah	0.500	1.293	81,000	52,358	
	Tulangan						
	Alat Pembekok	buah	0.500	1.293	27,500	17,776	
	Tulangan				<b>a + b + c =</b>	<b>2,650,529</b>	
3	Pengecoran Pilecap						
	a. Bahan :						
	Beton Ready Mix K-250	m <sup>3</sup>	1.100	1.800	1,142,150	2,261,457	
	b. Upah Pasang :						
	- Pekerja	hari	1.500	1.800	39,500	106,650	
	- Tukang	hari	0.750	1.800	48,500	65,475	
	- Mandor	hari	0.075	1.800	70,000	9,450	
	c. Lain-lain						
	Alat Bantu :						
	- Concrete Vibrator	unit	0.500	1.800	50,000	45,000	
	- Concrete Pump	unit	0.500	1.800	100,000	90,000	
					<b>a + b + c</b>	<b>2,578,032</b>	
					<b>=</b>		

**Biaya Pengerjaan Pilecap 1 + 2 + 3 = Rp 5,410,136**



## BAB V

### KESIMPULAN

#### 5.1 Kesimpulan

Dari hasil analisa perhitungan perencanaan pondasi tiang bor dan pondasi sumuran, perbandingan volume dan biaya pada pembangunan RUSUNAWA di Universitas Negeri Malang, dapat di simpulkan sebagai berikut :

1. Kondisi tanah pada kedalaman 6,40 m cukup keras, sehingga pondasi strauss aman untuk digunakan.
2. Pada perencanaan pondasi strauss ini penurunan tidak diperhitungkan karena ujung bawah tiang pondasi strauss sudah berada pada lapisan tanah keras.
3. Perbandingan biaya pengerjaan pondasi strauss dan pondasi sumuran adalah sebagai berikut :

❖ Untuk biaya pengerjaan pondasi tiang bor :

Tipe pondasi 1 dan 2	: Rp 75,932,699 x 64	= Rp 4,859,692,746
Tipe pondasi 3	: Rp 28,380,455 x 22	= <u>Rp 624,370,015 +</u>
Biaya Total Pengerjaan Pondasi Strauss		= <b>Rp 5,484,062,761</b>

❖ Untuk biaya pengerjaan pondasi sumuran :

Tipe pondasi 1,2 dan 3 : Rp 23,831,656 x 86 = Rp 2,049,522,395

**Biaya Total Pengerjaan Pondasi Sumuran = Rp 2,049,522,395**

4. Dari perhitungan biaya antara pekerjaan Pondasi Tiang Bor (Strauss) dan Pondasi Sumuran (Caisson) dapat disimpulkan bahwa pekerjaan pondasi tiang bor lebih mahal dari pada pondasi sumuran, tapi dilihat dari segi pelaksanaan dilapangan lebih mudah menggunakan pondasi strauss karena dimensi yang digunakan tidak terlalu besar dan bahan isiannya terbuat dari cor beton saja, sedangkan pondasi sumuran dimensinya relatif besar-besar dan isiannya pun terdiri dari dua jenis yaitu : cor beton dan beton cyclop.

## 5.2 Saran

Adapun saran-saran yang dapat diuraikan sebagai dasar pertimbangan dalam merencanakan struktur pondasi dan biaya pengerjaannya antara lain sebagai berikut :

1. Dalam suatu perencanaan pondasi, akan lebih baik dan teliti hasilnya jika parameter tanah yang digunakan adalah dari hasil penyelidikan lapangan dan hasil dari laboratorium bukan hasil dari konversi.
2. Untuk merencanakan pondasi tiang, jarak antar tiang sangat memengaruhi besarnya daya dukung tiang kelompok. Oleh karena itu perencana harus pandai-pandai memodifikasi posisi tiang tersebut, agar didapat daya dukung yang aman dan efisien.
3. Dalam merencanakan pondasi tiang juga harus diperhitungkan menggunakan jenis pondasi yang digunakan, karena akan mempengaruhi jumlah biaya yang diperlukan, waktu pengerjaan dan cara pengerjaannya. Sehingga biaya yang digunakan tidak terlalu mahal dan durasi pelaksanaan tidak terlalu lama.



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STAAD SPACE DXF IMPORT OF ENDHIK17.DXF

START JOB INFORMATION

ENGINEER DATE 06-Oct-10

END JOB INFORMATION

INPUT WIDTH 79

UNIT METER KG

JOINT COORDINATES

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ELEMENT PROPERTY

7982 TO 8336 THICKNESS 0.12  
 8337 TO 8351 8354 TO 8383 THICKNESS 0.12  
 8384 TO 9492 THICKNESS 0.03  
 9493 TO 10628 THICKNESS 0.03  
 10629 TO 10783 THICKNESS 0.03  
 10784 TO 10787 THICKNESS 0.12  
 10788 TO 10851 THICKNESS 0.03  
 10852 TO 10915 THICKNESS 0.03  
 5862 TO 6485 10916 TO 11019 THICKNESS 0.12  
 6486 TO 7109 11020 TO 11123 THICKNESS 0.12  
 7110 TO 7733 8352 8353 11124 TO 11227 THICKNESS 0.12  
 11430 TO 11565 THICKNESS 0.12  
 11566 TO 11689 THICKNESS 0.12  
 11784 TO 11791 THICKNESS 0.12  
 7734 TO 7981 11228 11229 11690 TO 11783 THICKNESS 0.12  
 11792 TO 11831 THICKNESS 0.12  
 11832 TO 11851 THICKNESS 0.12  
 11230 TO 11429 THICKNESS 0.24

DEFINE MATERIAL START

ISOTROPIC CONCRETE

E 2.21467e+009  
 POISSON 0.17  
 DENSITY 2400  
 ALPHA 1e-005  
 DAMP 0.05

ISOTROPIC STEEL

E 2.09042e+010  
 POISSON 0.3  
 DENSITY 7833.41  
 ALPHA 1.2e-005  
 DAMP 0.03

END DEFINE MATERIAL

MEMBER PROPERTY AMERICAN

84 87 116 117 123 124 132 133 151 153 155 156 158 161 TO 163 168 171 174 177 -  
 182 183 188 191 194 195 205 206 208 209 211 212 214 215 217 219 221 222 224 -  
 225 227 228 231 232 234 236 238 240 242 244 245 247 327 330 359 360 366 367 -  
 375 376 388 390 392 393 395 398 TO 400 405 408 411 414 419 420 425 428 431 -  
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 471 473 475 477 479 481 482 484 563 566 595 596 602 603 611 612 624 626 628 -  
 629 631 634 TO 636 641 644 647 650 655 656 661 664 667 668 678 679 681 682 -  
 684 685 687 688 690 692 694 695 697 698 700 701 704 705 707 709 711 713 715 -  
 717 718 720 977 980 TO 984 988 TO 991 995 TO 998 1002 TO 1045 1049 TO 1054 -  
 1064 1066 1068 1071 1073 1075 1078 1080 1082 1084 1086 1088 1090 1092 1094 -  
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 1309 TO 1311 1315 TO 1364 1371 1373 1375 1378 1380 1382 1386 1388 1390 1393 -  
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 1478 TO 1482 1486 TO 1489 1493 TO 1496 PRIS YD 0.4 ZD 0.2  
 5552 TO 5564 5566 5568 PRIS YD 0.4 ZD 0.2  
 1500 TO 1543 1547 TO 1552 1562 1564 1566 1569 1571 1573 1576 1578 1580 1582 -  
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 1879 TO 1921 1925 TO 1930 1940 1942 1944 1947 1949 1951 1954 1956 1958 1960 -  
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 1991 1993 1995 1997 1999 2001 2003 2005 2007 2009 2020 2028 2045 2048 2050 -  
 2051 TO 2054 2058 TO 2061 2065 TO 2068 2072 TO 2121 2128 2130 2132 2135 2137 -  
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 2174 2176 2178 2180 2183 2185 2187 2190 2192 2194 2197 2199 2201 2203 2205 -  
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 606 TO 608 615 TO 617 620 673 676 1143 1145 1188 1190 1287 1289 1466 1641 -  
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 184 186 189 192 196 198 199 207 210 213 216 218 220 223 226 229 230 239 241 -  
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 4293 4300 4301 5674 5675 5677 5678 PRIS YD 0.4 ZD 0.2  
 2361 2362 2395 2399 2401 2405 2410 2413 2415 2419 2421 2424 2427 2466 2468 -  
 2470 2575 2577 2579 2582 2584 2586 2589 2591 2593 2595 2597 2599 2601 2603 -  
 2605 2607 2609 2611 2616 2618 2620 2622 2624 2626 2628 2630 2632 2634 2636 -  
 2638 2640 2642 2644 2662 2750 2752 2754 2757 2759 2761 2763 2765 2767 2769 -  
 2770 2772 2774 2776 2778 2780 2782 2783 2785 2787 2789 2791 2794 2796 2798 -  
 2800 2801 2803 2805 2807 2808 2810 2812 2814 2815 2817 2819 2821 2823 2825 -  
 2827 2829 2831 2833 5738 5740 5742 5744 5745 12837 12839 13400 13418 -  
 13419 PRIS YD 0.5 ZD 0.15  
 139 TO 144 PRIS YD 0.3 ZD 0.2  
 1 2 5 6 9 TO 39 56 TO 83 254 PRIS YD 0.45 ZD 0.3  
 52 TO 55 PRIS YD 0.4 ZD 0.2  
 3 4 7 8 40 TO 51 935 PRIS YD 0.4 ZD 0.3  
 729 732 TO 739 817 818 868 TO 877 880 881 933 1147 TO 1151 1154 1155 1236 -  
 1237 TO 1246 1261 1277 1282 1471 TO 1473 5571 5572 5763 -  
 5764 PRIS YD 0.3 ZD 0.15  
 936 TO 940 942 1192 TO 1197 1199 1235 2312 5575 12748 13344 PRIS YD 0.3 ZD 0.15  
 2313 2314 PRIS YD 0.4 ZD 0.3  
 258 260 262 266 268 270 272 274 276 290 299 323 TO 326 PRIS YD 0.45 ZD 0.3  
 291 TO 298 PRIS YD 0.4 ZD 0.3  
 823 TO 832 837 TO 840 890 892 TO 901 906 907 947 TO 949 951 952 954 955 1656 -  
 1657 1661 1848 1851 2242 TO 2251 5615 5773 TO 5778 5841 TO 5847 -  
 12775 PRIS YD 0.3 ZD 0.15  
 820 822 888 891 2316 2318 PRIS YD 0.3 ZD 0.15  
 527 TO 534 PRIS YD 0.4 ZD 0.3  
 255 TO 257 259 261 263 TO 265 267 269 271 273 275 277 TO 289 300 TO 322 491 -  
 492 TO 526 535 TO 562 PRIS YD 0.45 ZD 0.3  
 844 846 TO 858 860 862 911 913 TO 917 919 TO 922 924 929 930 2035 2039 2226 -  
 2229 2274 2288 TO 2300 5779 5781 5783 5784 PRIS YD 0.3 ZD 0.15  
 958 TO 962 965 966 2034 2320 2322 5655 5848 TO 5854 12804 -  
 12805 PRIS YD 0.3 ZD 0.15  
 780 TO 787 PRIS YD 0.4 ZD 0.3  
 2301 TO 2305 2878 2879 5681 5855 TO 5861 PRIS YD 0.3 ZD 0.15  
 744 TO 779 788 TO 815 PRIS YD 0.45 ZD 0.3  
**MEMBER PROPERTY AMERICAN**  
 5520 TO 5529 13393 TO 13399 13401 PRIS YD 0.4 ZD 0.3  
 332 363 389 391 485 1483 TO 1485 1560 1561 1677 TO 1679 1847 1849 1850 5620 -  
 5622 5624 5632 PRIS YD 0.5 ZD 0.25  
 407 409 1595 1597 1599 1778 1784 1785 1787 1789 1791 2319 2321 -  
 5614 PRIS YD 0.5 ZD 0.15  
 170 172 934 1097 1099 1101 1206 TO 1208 2315 5574 PRIS YD 0.5 ZD 0.15  
 918 923 5780 5782 PRIS YD 0.3 ZD 0.15  
 609 610 670 674 675 677 910 912 927 928 2013 TO 2015 2030 2036 2041 2047 2217 -  
 2218 TO 2219 2371 2372 2431 2435 2436 2647 2660 2669 2836 2837 2861 TO 2874 -  
 4613 4614 4633 4666 TO 4668 5509 TO 5514 5676 5679 5680 -  
 5682 PRIS YD 0.4 ZD 0.2  
**MEMBER PROPERTY AMERICAN**  
 2324 2326 PRIS YD 0.3 ZD 0.15  
 3504 TO 3509 3511 3515 3517 4448 TO 4455 5500 TO 5508 5535 5536 5538 5540 -  
 5542 5543 PRIS YD 0.3 ZD 0.15  
 3522 TO 3527 3529 3533 3535 5032 TO 5037 5039 5546 5548 5550 5551 12834 -  
 12835 PRIS YD 0.3 ZD 0.15  
 5038 PRIS YD 0.3 ZD 0.15  
 5515 TO 5519 PRIS YD 0.3 ZD 0.15  
 3510 3516 5534 5539 PRIS YD 0.3 ZD 0.15  
 3528 3534 12832 12833 PRIS YD 0.3 ZD 0.15  
**MEMBER PROPERTY AMERICAN**  
 12836 12838 12840 13343 PRIS YD 0.4 ZD 0.3  
**MEMBER PROPERTY AMERICAN**  
 740 TO 743 816 835 836 841 842 863 TO 865 882 TO 886 902 903 908 909 925 926 -  
 931 932 941 943 TO 946 950 953 956 957 963 964 967 TO 975 1156 TO 1185 1198 -  
 1200 TO 1205 1209 TO 1227 1231 1247 TO 1260 1262 TO 1276 1278 2230 TO 2241 -  
 2252 TO 2273 2275 TO 2287 2306 TO 2311 5530 5531 5765 TO 5772 5785 TO 5840 -  
 12749 TO 12774 12776 TO 12803 12806 TO 12831 PRIS YD 0.2 ZD 0.15  
**MEMBER PROPERTY AMERICAN**  
 3479 TO 3502 3540 3541 3548 3552 3554 3557 3560 3563 3566 3569 3572 3575 3578 -  
 3581 3584 3587 3590 3593 3596 3599 3602 3605 3608 3611 3614 3617 3620 3623 -  
 3626 3629 3632 3635 3638 3641 3644 3647 3650 3655 3656 4256 4443 TO 4447 -  
 4462 4463 5008 TO 5031 5046 5683 TO 5689 5692 TO 5694 5696 TO 5698 -  
 5700 TO 5702 5704 TO 5706 5708 TO 5710 5713 TO 5715 5717 TO 5719 -  
 5721 TO 5723 5725 TO 5727 5729 TO 5736 PRIS YD 0.3 ZD 0.2  
**MEMBER PROPERTY JAPANESE**  
 2881 TO 2885 2888 TO 2898 2901 TO 2908 2910 TO 2912 2915 TO 2922 2924 TO 2926 -  
 2929 TO 2936 2938 TO 2940 2943 TO 2950 2952 TO 2954 2957 TO 2964 -  
 2966 TO 2968 2971 TO 2978 2980 TO 2982 2985 TO 2992 2994 TO 2996 -

2999 TO 3006 3008 TO 3010 3013 TO 3020 3022 TO 3024 3027 TO 3034 -  
 3036 TO 3038 3041 TO 3048 3050 TO 3052 3055 TO 3062 3064 TO 3066 -  
 3069 TO 3076 3078 TO 3080 3083 TO 3090 3092 TO 3094 3097 TO 3104 -  
 3106 TO 3108 3111 TO 3118 3120 TO 3122 3125 TO 3132 3134 TO 3136 -  
 3139 TO 3146 3148 TO 3150 3153 TO 3160 3162 TO 3164 3167 TO 3174 -  
 3176 TO 3178 3181 TO 3188 3190 TO 3192 3195 TO 3202 3204 TO 3206 -  
 3209 TO 3216 3218 TO 3220 3223 TO 3230 3232 TO 3234 3237 TO 3244 -  
 3246 TO 3248 3251 TO 3258 3260 TO 3262 3265 TO 3272 3274 TO 3276 -  
 3279 TO 3286 3288 TO 3290 3293 TO 3300 3302 TO 3304 3307 TO 3314 -  
 3316 TO 3318 3321 TO 3328 3330 TO 3332 3335 TO 3342 3344 TO 3346 -  
 3349 TO 3356 3358 TO 3360 3363 TO 3370 3372 TO 3374 3377 TO 3384 -  
 3386 TO 3388 3391 TO 3398 3400 TO 3402 3405 TO 3412 3414 TO 3416 -  
 3419 TO 3429 3432 TO 3442 3445 TO 3478 3658 TO 3662 3665 TO 3675 -  
 3678 TO 3685 3687 TO 3689 3692 TO 3699 3701 TO 3703 3706 TO 3713 -  
 3715 TO 3717 3720 TO 3727 3729 TO 3731 3734 TO 3741 3743 TO 3745 -  
 3748 TO 3755 3757 TO 3759 3762 TABLE ST I125X75X6  
 12841 TO 12879 12913 12941 TO 12944 13018 13019 13051 13052 13056 13129 13130 -  
 13194 TO 13202 13237 TO 13240 TABLE ST I125X75X6  
 3763 TO 3769 3771 TO 3773 3776 TO 3783 3785 TO 3787 3790 TO 3797 3799 TO 3801 -  
 3804 TO 3811 3813 TO 3815 3818 TO 3825 3827 TO 3829 3832 TO 3839 -  
 3841 TO 3843 3846 TO 3853 3855 TO 3857 3860 TO 3867 3869 TO 3871 -  
 3874 TO 3881 3883 TO 3885 3888 TO 3895 3897 TO 3899 3902 TO 3909 -  
 3911 TO 3913 3916 TO 3923 3925 TO 3927 3930 TO 3937 3939 TO 3941 -  
 3944 TO 3951 3953 TO 3955 3958 TO 3965 3967 TO 3969 3972 TO 3979 -  
 3981 TO 3983 3986 TO 3993 3995 TO 3997 4000 TO 4007 4009 TO 4011 -  
 4014 TO 4021 4023 TO 4025 4028 TO 4035 4037 TO 4039 4042 TO 4049 -  
 4051 TO 4053 4056 TO 4063 4065 TO 4067 4070 TO 4077 4079 TO 4081 -  
 4084 TO 4091 4093 TO 4095 4098 TO 4105 4107 TO 4109 4112 TO 4119 -  
 4121 TO 4123 4126 TO 4133 4135 TO 4137 4140 TO 4147 4149 TO 4151 -  
 4154 TO 4161 4163 TO 4165 4168 TO 4175 4177 TO 4179 4182 TO 4189 -  
 4191 TO 4193 4196 TO 4206 4209 TO 4219 4222 TO 4255 4302 TO 4442 -  
 4464 TO 4604 4669 TO 5007 5047 TO 5497 12880 TO 12912 12914 TO 12940 12945 -  
 12946 TO 13017 13020 TO 13050 13053 TO 13055 13057 TO 13128 13131 TO 13193 -  
 13203 TO 13236 13241 TO 13272 TABLE ST I125X75X6  
 2880 2886 2887 2899 2900 2909 2913 2914 2923 2927 2928 2937 2941 2942 2951 -  
 2955 2956 2965 2969 2970 2979 2983 2984 2993 2997 2998 3007 3011 3012 3021 -  
 3025 3026 3035 3039 3040 3049 3053 3054 3063 3067 3068 3077 3081 3082 3091 -  
 3095 3096 3105 3109 3110 3119 3123 3124 3133 3137 3138 3147 3151 3152 3161 -  
 3165 3166 3175 3179 3180 3189 3193 3194 3203 3207 3208 3217 3221 3222 3231 -  
 3235 3236 3245 3249 3250 3259 3263 3264 3273 3277 3278 3287 3291 3292 3301 -  
 3305 3306 3315 3319 3320 3329 3333 3334 3343 3347 3348 3357 3361 3362 3371 -  
 3375 3376 3385 3389 3390 3399 3403 3404 3413 3417 3418 3430 3431 3443 3444 -  
 3503 3542 3543 3657 3663 3664 3676 3677 3686 3690 3691 3700 3704 3705 3714 -  
 3718 3719 3728 3732 3733 3742 3746 3747 3756 3760 3761 3770 3774 3775 3784 -  
 3788 3789 3798 3802 3803 3812 3816 3817 3826 3830 3831 3840 3844 3845 3854 -  
 3858 3859 3868 3872 3873 3882 3886 3887 3896 3900 3901 3910 3914 3915 3924 -  
 3928 3929 3938 3942 3943 3952 3956 3957 3966 3970 3971 3980 3984 3985 3994 -  
 3998 3999 4008 4012 4013 4022 4026 4027 4036 4040 4041 4050 4054 4055 4064 -  
 4068 4069 4078 4082 4083 4092 4096 4097 4106 4110 4111 4120 4124 4125 4134 -  
 4138 4139 4148 4152 4153 4162 4166 4167 4176 4180 4181 4190 4194 4195 4207 -  
 4208 4220 4221 4257 TO 4259 TABLE ST H125X60X6  
 3512 TO 3514 3518 TO 3521 3530 TO 3532 3536 TO 3539 4456 TO 4461 5040 TO 5045 -  
 13277 TO 13291 13296 13299 TO 13324 13329 13332 TO 13342 13345 TO 13348 -  
 13353 13354 13361 TO 13364 13369 13370 13380 13381 13388 -  
 13389 TABLE ST I125X75X6  
 5532 5533 5537 5541 5544 5545 5547 5549 TABLE ST H150X75X5  
 13273 13274 13292 TO 13295 13297 13298 TABLE ST I125X75X6  
 13275 13276 13325 TO 13328 13330 13331 TABLE ST I125X75X6  
 MEMBER PROPERTY JAPANESE  
 13365 TO 13368 13371 TO 13374 TABLE ST I125X75X6  
 13349 TO 13352 13355 TO 13358 TABLE ST I125X75X6  
 13359 13360 13385 TO 13387 13390 TO 13392 TABLE ST I125X75X6  
 13375 TO 13379 13382 TO 13384 TABLE ST I125X75X6  
 MEMBER PROPERTY JAPANESE  
 13427 TO 13430 13432 13433 13436 13439 TO 13441 13446 13447 13508 TO 13512 -  
 13514 13516 13518 13520 13522 13524 13526 13528 13534 13536 13538 13540 -  
 13542 TO 13546 13548 TO 13551 13553 13555 13557 13558 13560 13562 13564 -  
 13566 13572 13574 13576 13578 TO 13582 13584 13586 13588 13590 13592 13594 -  
 13596 13598 13600 13604 13606 13608 13610 13612 13614 13616 13618 -  
 13620 PRIS YD 0.3 ZD 0.2  
 13420 TO 13426 13431 13434 13435 13437 13438 13442 TO 13445 13448 13449 13451 -  
 13452 13454 TO 13472 13491 TO 13507 13513 13515 13517 13519 13521 13523 -  
 13525 13527 13529 TO 13533 13535 13537 13539 13541 13547 13552 13554 13556 -  
 13559 13561 13563 13565 13567 TO 13571 13573 13575 13577 13583 13585 13587 -  
 13589 13591 13593 13595 13597 13599 13601 TO 13603 13605 13607 13609 13611 -  
 13613 13615 13617 13619 13621 TO 13623 13638 TO 13643 PRIS YD 0.3 ZD 0.25  
 MEMBER PROPERTY JAPANESE  
 13450 13453 13473 TO 13490 PRIS YD 0.3 ZD 0.2  
 13402 13405 TO 13407 13410 TO 13412 13415 TO 13417 13624 TO 13626 -  
 13627 PRIS YD 0.15 ZD 0.1  
 MEMBER PROPERTY JAPANESE  
 13628 TO 13632 PRIS YD 0.3 ZD 0.2

13633 TO 13637 PRIS YD 0.3 ZD 0.2

## CONSTANTS

MATERIAL CONCRETE MEMB 84 TO 138 145 TO 253 327 330 359 360 366 TO 372 375 -  
 376 TO 381 384 388 390 392 393 395 398 TO 400 405 408 411 414 419 420 425 -  
 428 431 432 437 440 442 443 445 446 448 449 451 452 454 456 458 459 461 462 -  
 464 465 468 469 471 473 475 477 479 481 482 484 563 566 595 596 602 TO 608 -  
 611 TO 617 620 624 626 628 629 631 634 TO 636 641 644 647 650 655 656 661 -  
 664 667 668 673 676 678 679 681 682 684 685 687 688 690 692 694 695 697 698 -  
 700 701 704 705 707 709 711 713 715 717 718 720 727 728 730 731 866 867 878 -  
 879 976 TO 1146 1152 1153 1186 TO 1191 1228 TO 1230 1232 TO 1234 -  
 1279 TO 1281 1283 TO 1470 1475 1478 TO 1482 1486 TO 1489 1493 TO 1496 1500 -  
 1501 TO 1543 1547 TO 1552 1562 1564 1566 1569 1571 1573 1576 1578 1580 1582 -  
 1584 1586 1588 1590 1592 1594 1596 1598 1600 TO 1603 1605 1607 1609 1611 -  
 1613 1615 1617 1619 1621 1623 1625 13403 13404  
 MATERIAL CONCRETE MEMB 1627 1629 1631 1639 1641 TO 1643 1647 1649 TO 1651 -  
 1665 TO 1668 1670 1672 TO 1676 1680 TO 1683 1687 TO 1690 1694 TO 1743 1750 -  
 1752 1754 1757 1759 1761 1765 1767 1769 1772 1774 1776 1779 1781 1783 1786 -  
 1788 1790 1793 1795 1796 1798 1800 1802 1805 1807 1809 1812 1814 1816 1819 -  
 1821 1823 1825 1827 1829 1831 1833 1835 1842 1844 1845 1853 1856  
 MATERIAL CONCRETE MEMB 934 5565 5567 5569 5574 5578 5583 5594 TO 5606 5608 -  
 5610 5618 5623 5658  
 MATERIAL CONCRETE MEMB 1857 TO 1860 1864 TO 1867 1871 TO 1874 1878 TO 1921 -  
 1925 TO 1930 1940 1942 1944 1947 1949 1951 1954 1956 1958 1960 1962 1964 -  
 1966 1968 1970 1972 1974 1976 1978 TO 1981 1983 1985 1987 1989 1991 1993 -  
 1995 1997 1999 2001 2003 2005 2007 2009 2017 2019 TO 2021 2025 2027 TO 2029 -  
 2043 TO 2046 2048 2050 TO 2054 2058 TO 2061 2065 TO 2068 2072 TO 2121 2128 -  
 2130 2132 2135 2137 2139 2143 2145 2147 2150 2152 2154 2157 2159 2161 2164 -  
 2166 2168 2171 2173 2174 2176 2178 2180 2183 2185 2187 2190 2192 2194 2197 -  
 2199 2201 2203 2205 2207 2209 2211 2213 2220 2222 2223 2317 2327 2330 2359 -  
 2360 2364 TO 2370 2373 TO 2379 2382 2386 2388 2390 2391 2393 2396 TO 2398 -  
 2403 2406 2408 2411 2416 2417 2422 2425 2428 2429 2434 2437 TO 2439 2441 -  
 2442 2444 2445 2447 2448 2450 2452 2454 2455 2457 2458 2460 2461 2464 2465 -  
 2467 2469 2471 2473 2475 2477 2478 2480 2488 2491 TO 2495 2499 TO 2502 2506 -  
 2507 TO 2509 2513 TO 2556 2560 TO 2565 2574 2576 2578 2581 2583 2585 2588 -  
 2590 2592 2594 2596 2598 2600 2602 2604 2606 2608 2610 2612 TO 2615 2617 -  
 2619 2621 2623 2625 2627 2629  
 MATERIAL CONCRETE MEMB 2631 2633 2635 2637 2639 2641 2643 2649 2651 TO 2653 -  
 2655 2657 TO 2659 2665 TO 2668 2670 2672 TO 2676 2680 TO 2683 2687 TO 2690 -  
 2694 TO 2743 2749 2751 2753 2756 2758 2760 2764 2766 2768 2771 2773 2775 -  
 2777  
 MATERIAL CONCRETE MEMB 5634 TO 5646 5648 5650 5663 5690 5691 5695 5699 5703 -  
 5707 5711 5712 5716 5720 5724 5728 5737 5739 5741 5748  
 MATERIAL CONCRETE MEMB 1 2 5 6 9 TO 40 42 44 45 47 49 TO 83 139 TO 144 254 -  
 255 TO 326 328 329 331 TO 358 361 TO 365 373 374 382 383 385 TO 387 389 391 -  
 394 396 397 401 TO 404 406 409 410 412 413 415 TO 418 421 TO 424 426 427 -  
 429 430 433 TO 436 438 439 441 444 447 450 453 455 457 460 463 466 467 470 -  
 472 474 476 478 480 483 485 TO 562 564 565 567 TO 594 597 TO 601 609 610 -  
 618 619 621 TO 623 625 627 630 632 633 637 TO 640 642 643 645 646 648 649 -  
 651 TO 654 657 TO 660 662 663 665 666 669 TO 672 674 675 677 680 683 686 -  
 689 691 693 696 699 702 703 706 708 710 712 714 716 719 721 TO 726 729 732 -  
 733 TO 739 744 TO 815 817 TO 834 837 TO 840 843 TO 862 868 TO 877 880 881 -  
 887 TO 901 904 TO 907 910 TO 917 919 TO 922 924 927 TO 930 933 936 TO 940 -  
 942 947 TO 949 951 952 954 955 958 TO 962 965 966 1147 TO 1151 1154 1155 -  
 1192 TO 1197 1199 1235 TO 1246 1261 1277 1282 1471 TO 1474 1476 1477 1483 -  
 1484 TO 1485 1490 TO 1492 1497 TO 1499 1544 TO 1546 1553 TO 1560 1563 1565 -  
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 MATERIAL CONCRETE MEMB 5575 5612 5614 TO 5617 5619 5620 5622 5630 5633 5652 -  
 5655 TO 5657 5659 5660 5662 5670 5673 5773 TO 5779 5781 5783 5784 -  
 5841 TO 5854 12748 12804  
 MATERIAL CONCRETE MEMB 1616 1618 1620 1622 1624 1626 1628 1630 1632 TO 1638 -  
 1640 1644 TO 1646 1648 1652 TO 1664 1669 1671 1677 TO 1679 1684 TO 1686 1691 -  
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 1824 1826 1828 1830 1832 1834 1836 TO 1841 1843 1846 TO 1852 1854 1855 1861 -  
 1862 TO 1863 1868 TO 1870 1875 TO 1877 1922 TO 1924 1931 TO 1939 1941 1943 -  
 1945 1946 1948 1950 1952 1953 1955 1957 1959 1961 1963 1965 1967 1969 1971 -  
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 2056 TO 2057 2062 TO 2064 2069 TO 2071 2122 TO 2127 2129 2131 2133 2134 2136 -  
 2138 2140 TO 2142 2144 2146 2148 2149 2151 2153 2155 2156 2158 2160 2162 -  
 2163 2165 2167 2169 2170 2172 2175 2177 2179 2181 2182 2184 2186 2188 2189 -  
 2191 2193 2195 2196 2198 2200 2202 2204 2206 2208 2210 2212 2214 TO 2219 -  
 2221 2224 TO 2229 2242 TO 2251 2274 2288 TO 2305 2312 TO 2314 2316 2318 2320 -  
 2321 TO 2323 2325 2328 2329 2331 TO 2358 2361 TO 2363 2371 2372 2380 2381 -  
 2383 TO 2385 2387 2389 2392 2394 2395 5607 5609 13344 13408 13409 13413 -  
 13414  
 MATERIAL CONCRETE MEMB 5611 5613 5621 5624 TO 5629 5631 5632 5647 5649 5651 -  
 5653 5654 5661 5664 TO 5669 5671 5672 5746 5751 5762 5855 TO 5861 12775 -  
 12805  
 MATERIAL CONCRETE MEMB 918 923 1206 TO 1208 1561 1787 2315 2319 2324 2326 -



2399 TO 2402 2404 2405 2407 2409 2410 2412 TO 2415 2418 TO 2421 2423 2424 -  
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 2684 TO 2686 2691 TO 2693 2744 TO 2748 2750 2752 2754 2755 2757 2759 2761 -  
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 3504 TO 3511 3515 TO 3517 3522 TO 3529 3533 TO 3535 3544 TO 3547 -  
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 3642 3643 3645 3646 3648 3649 3651 TO 3654 4260 TO 4301 4448 TO 4455 4605 -  
 4606 TO 4668 5032 TO 5039 5498 TO 5521 5524 TO 5529 5534 TO 5536 5538 TO 5540 -  
 5542 13418 13419  
 MATERIAL CONCRETE MEMB 12837 12839  
 MATERIAL CONCRETE MEMB 5543 5546 5548 5550 TO 5564 5566 5568 5570 TO 5572 -  
 5576 5577 5579 TO 5582 5584 TO 5593 5674 TO 5682 5738 5740 5742 TO 5745 5747 -  
 5749 5862 TO 7762  
 MATERIAL CONCRETE MEMB 7763 TO 7773  
 MATERIAL CONCRETE MEMB 5750 5752 7774 TO 8283  
 MATERIAL CONCRETE MEMB 5753 TO 5761 5763 5764 5780 5782 13400  
 MATERIAL CONCRETE MEMB 3 4 7 8 41 43 46 48 740 TO 743 816 835 836 841 842 -  
 863 TO 865 882 TO 886 902 903 908 909 925 926 931 932 935 941 943 TO 946 -  
 950 953 956 957 963 964 967 TO 975 1156 TO 1185 1198 1200 TO 1205 -  
 1209 TO 1227 1231 1247 TO 1260 1262 TO 1276 1278 2230 TO 2241 2252 TO 2273 -  
 2275 TO 2287 2306 TO 2311 3479 TO 3484 3486 TO 3502 3540 3541 3548 3552 3554 -  
 3557 3563 3566 3569 3572 3575 3578 3581 3584 3587 3590 3593 3596 3599 3602 -  
 3605 3608 3611 3614 3617 3620 3623 3626 3629 3632 3635 3638 3641 3644 3647 -  
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 5531 5683 TO 5689 5692 TO 5694 5696 TO 5698 5700 TO 5702 5704 TO 5706 5708 -  
 5709 TO 5710 5713 TO 5715 5717 TO 5719 5721 TO 5723 5725 TO 5727 5729 TO 5736 -  
 5765 TO 5772 5785 TO 5840 12749 TO 12774 12776 TO 12802  
 MATERIAL CONCRETE MEMB 8284 TO 8691 12803 12806 TO 12812 13393 TO 13399 13401  
 MATERIAL CONCRETE MEMB 8692 TO 9853  
 MATERIAL CONCRETE MEMB 9854 TO 10783 12813  
 MATERIAL CONCRETE MEMB 407 5573 10784 TO 11719 12814 TO 12836 12838 12840 -  
 13343 13420 TO 13449 13451 13452 13454 TO 13472 13491 TO 13623  
 MATERIAL CONCRETE MEMB 11720 TO 11851 13402 13405 TO 13407 13410 TO 13412 -  
 13415 TO 13417 13450 13453 13473 TO 13490 13624 TO 13643  
 MATERIAL STEEL MEMB 2880 TO 3478 3503 3512 TO 3514 3518 TO 3521 3530 TO 3532 -  
 3536 TO 3539 3542 3543 3657 TO 4255 4257 TO 4259 4302 TO 4442 4456 TO 4461 -  
 4464 TO 4604 4669 TO 5007 5040 TO 5045 5047 TO 5497 5532 5533 5537 5541 5544 -  
 5545 5547 5549 12841 TO 13342 13345 TO 13392  
 SUPPORTS  
 1 3 5 7 9 11 13 15 17 19 21 23 25 27 29 31 33 35 37 39 41 43 45 47 49 51 53 -  
 55 57 59 61 63 65 67 69 71 73 75 77 79 81 83 85 87 89 91 93 95 97 99 101 -  
 103 105 107 109 111 113 115 117 119 121 123 125 127 129 131 133 135 137 139 -  
 141 143 145 147 149 151 153 155 157 159 161 163 165 333 1040 1042 2018 2019 -  
 3749 TO 3752 FIXED  
 1075 1078 1082 PINNED  
 1077 1080 1084 PINNED  
 MEMBER TRUSS  
 2888 TO 2895 2901 TO 2908 2915 TO 2922 2929 TO 2936 2943 TO 2950 2957 TO 2964 -  
 2971 TO 2978 2985 TO 2992 2999 TO 3006 3013 TO 3020 3027 TO 3034 -  
 3041 TO 3048 3055 TO 3062 3069 TO 3076 3083 TO 3090 3097 TO 3104 -  
 3111 TO 3118 3125 TO 3132 3139 TO 3146 3153 TO 3160 3167 TO 3174 -  
 3181 TO 3188 3195 TO 3202 3209 TO 3216 3223 TO 3230 3237 TO 3244 -  
 3251 TO 3258 3265 TO 3272 3279 TO 3286 3293 TO 3300 3307 TO 3314 -  
 3321 TO 3328 3335 TO 3342 3349 TO 3356 3363 TO 3370 3377 TO 3384 -  
 3391 TO 3398 3405 TO 3412 3419 TO 3426 3432 TO 3439 3445 TO 3452 -  
 3665 TO 3672 3678 TO 3685 3692 TO 3699 3706 TO 3713 3720 TO 3727 -  
 3734 TO 3741 3748 TO 3755 3762 TO 3769 3776 TO 3783 3790 TO 3797 -  
 3804 TO 3811 3818 TO 3825 3832 TO 3839 3846 TO 3853 3860 TO 3867 -  
 3874 TO 3881 3888 TO 3895 3902 TO 3909 3916 TO 3923 3930 TO 3937 -  
 3944 TO 3951 3958 TO 3965 3972 TO 3979 3986 TO 3993 4000 TO 4007 -  
 4014 TO 4021 4028 TO 4035 4042 TO 4049 4056 TO 4063 4070 TO 4077 -  
 4084 TO 4091 4098 TO 4105 4112 TO 4119 4126 TO 4133 4140 TO 4147 -  
 4154 TO 4161 4168 TO 4175 4182 TO 4189 4196 TO 4203 4209 TO 4216 -  
 4222 TO 4229  
 MEMBER TRUSS  
 13349 TO 13352 13355 TO 13358  
 MEMBER TRUSS  
 13365 TO 13368 13371 TO 13374  
 LOAD 1 BEBAN MATI  
 ELEMENT LOAD  
 5862 TO 6485 10916 TO 11019 PR GY -450  
 7982 TO 7989 8006 TO 8037 8102 TO 8111 8128 TO 8159 10786 10787 PR GY -8982.52  
 8224 TO 8231 8256 TO 8271 8288 TO 8303 10784 10785 PR GY -9156.02  
 6486 TO 7109 11020 TO 11123 PR GY -450

7990 TO 7997 8040 TO 8069 8112 TO 8119 8160 TO 8189 PR GY -8982.52  
 8232 TO 8239 8274 TO 8287 8304 TO 8319 PR GY -9156.02  
 7110 TO 7733 11124 TO 11227 PR GY -450  
 7998 TO 8005 8072 TO 8101 8120 TO 8127 8192 TO 8221 PR GY -8982.52  
 8240 TO 8247 8322 TO 8351 PR GY -9156.02  
 7734 TO 7981 8368 8369 11228 11229 11690 TO 11791 PR GY -438  
 8248 TO 8255 8352 TO 8383 PR GY -9156.02  
 \*BEBAN AIR DALAM TANDON  
 11230 TO 11429 PR GY -20000  
 LOAD 2 BEBAN HIDUP  
 ELEMENT LOAD  
 5862 TO 6485 10964 10965 PR GY -250  
 7982 TO 7989 8006 TO 8037 8102 TO 8111 8128 TO 8159 8224 TO 8231 8256 TO 8271 -  
 8288 TO 8303 10784 TO 10787 PR GY -300  
 6486 TO 7109 11020 11021 PR GY -250  
 7990 TO 7997 8038 TO 8069 8112 TO 8119 8160 TO 8191 8232 TO 8239 8272 TO 8287 -  
 8304 TO 8319 PR GY -300  
 7110 TO 7733 11124 11125 PR GY -250  
 8248 TO 8255 8352 TO 8383 PR GY -300  
 7734 TO 7981 11228 11229 PR GY -150  
 LOAD 3 BEBAN GEMPA U-B  
 JOINT LOAD  
 8082 8087 8090 8091 FX 1066.16 FZ 3553.88  
 8083 8086 8092 8094 FX 211.38 FZ 704.61  
 8084 8085 8093 8095 FX 53.52 FZ 178.41  
 8089 FX 642.47 FZ 2141.58  
 8097 8101 8106 8107 FX 2069.61 FZ 6898.71  
 8098 8102 8104 8108 FX 410.33 FZ 1367.77  
 8099 8103 8105 8109 FX 103.9 FZ 346.32  
 8100 FX 878.13 FZ 2927.1  
 8113 8115 8120 8121 FX 3035.55 FZ 10118.5  
 8111 8116 8119 8122 FX 601.84 FZ 2006.13  
 8112 8117 8118 8123 FX 152.39 FZ 507.95  
 8114 FX 1287.97 FZ 4293.24  
 4943 5014 5042 8691 FX 4023.68 FZ 13412.3  
 4931 4982 8689 8690 FX 797.75 FZ 2659.17  
 8684 8685 8687 8688 FX 201.24 FZ 673.31  
 8686 FX 1707.24 FZ 5690.78  
 LOAD 4 BEBAN GEMPA S-T  
 JOINT LOAD  
 8082 8087 8090 8091 FX -1066.16 FZ -3553.88  
 8083 8086 8092 8094 FX -211.38 FZ -704.61  
 8084 8085 8093 8095 FX -53.52 FZ -178.41  
 8089 FX -642.47 FZ -2141.58  
 8097 8101 8106 8107 FX -2069.61 FZ -6898.71  
 8098 8102 8104 8108 FX -410.33 FZ -1367.77  
 8099 8103 8105 8109 FX -103.9 FZ -346.32  
 8100 FX -878.13 FZ -2927.1  
 8113 8115 8120 8121 FX -3035.55 FZ -10118.5  
 8111 8116 8119 8122 FX -601.84 FZ -2006.13  
 8112 8117 8118 8123 FX -152.39 FZ -507.95  
 8114 FX -1287.97 FZ -4293.24  
 4943 5014 5042 8691 FX -4023.68 FZ -13412.3  
 4931 4982 8689 8690 FX -797.75 FZ -2659.17  
 8684 8685 8687 8688 FX -201.24 FZ -673.31  
 8686 FX -1707.24 FZ -5690.78  
 LOAD 5 BEBAN ANGIN U-S  
 ELEMENT LOAD  
 8984 TO 9583 10184 TO 10783 PR 65.433  
 8384 TO 8983 9584 TO 10183 PR -37.39  
 LOAD 6 BEBAN ANGIN B-T  
 ELEMENT LOAD  
 10788 TO 10819 10852 TO 10883 PR 81.85  
 10820 TO 10851 10884 TO 10915 PR 38.976  
 LOAD COMB 7 KOM.1 (1.4 D)  
 1 1.4  
 LOAD COMB 8 KOM.2 (1.2 D + 1.6 L)  
 1 1.2 2 1.6  
 LOAD COMB 9 KOM.3 (1.2 D + 1.0 L + 1.6 ANGIN U-S)  
 1 1.2 2 1.0 5 1.6  
 LOAD COMB 10 KOM.4 (1.2 D + 1.0 L + 1.6 ANGIN B-T)  
 1 1.2 2 1.0 6 1.6  
 LOAD COMB 11 KOM.5 (0.9 D + 1.6 ANGIN U-S)  
 1 0.9 5 1.6  
 LOAD COMB 12 KOM.6 (0.9 D + 1.6 ANGIN B-T)  
 1 0.9 6 1.6  
 LOAD COMB 13 KOM.7 (1.2 D + 1.0 L + 1.0 GEMPA U-B)  
 1 1.2 2 1.0  
 LOAD COMB 14 KOM.8 (1.2 D + 1.0 L - 1.0 GEMPA U-B)  
 1 1.2 2 1.0  
 LOAD COMB 15 KOM.9 (1.2 D + 1.0 L + 1.0 GEMPA S-T)  
 1 1.2 2 1.0

LOAD COMB 16 KOM.10 (1.2 D + 1.0 L - 1.0 GEMPA S-T)  
1 1.2 2 1.0  
LOAD COMB 17 KOM.11 (0.9 D + 1.0 GEMPA U-B)  
1 0.9  
LOAD COMB 18 KOM.12 (0.9 D - 1.0 GEMPA U-B)  
1 0.9  
LOAD COMB 19 KOM.13 (0.9 D + 1.0 GEMPA S-T)  
1 0.9  
LOAD COMB 20 KOM.14 (0.9 D - 1.0 GEMPA S-T)  
1 0.9  
PERFORM ANALYSIS  
LOAD LIST 1 7 TO 20  
FINISH



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Job No	Sheet No	Rev
	1	
Part		
Ref		
By	Date	Chd
	06-Oct-10	
Client	File	Date/Time
	endhik-2 sip 4.std	21-Mar-2011 15:52

## Reactions

Node	LC	Horizontal		FZ	Moment		
		FX (kg)	FY (kg)		MX (kg·m)	MY (kg·m)	MZ (kg·m)
1	1:BEBA MAT	352.767	1.02E 3	-634.698	-1.27E 3	-47.428	-952.373
	2:BEBA HIDL	82.988	3.24E 3	25.012	25.113	-1.901	-136.818
	3:BEBA GEV	-605.671	-5.43E 3	-1.63E 3	-4.03E 3	2.305	1.73E 3
	4:BEBA GEV	605.671	5.43E 3	1.63E 3	4.03E 3	-2.305	-1.73E 3
	5:BEBA ANG	-1.954	-740.658	-195.821	-483.818	0.116	5.817
	6:BEBA ANG	-4.011	-7.381	0.125	0.108	-0.013	11.665
	7:KOM.1 (1.4 L	493.873	1.43E 3	-888.577	-1.77E 3	-66.399	-1.33E 3
	8:KOM.2 (1.2 L	556.100	6.41E 3	-721.617	-1.48E 3	-59.956	-1.36E 3
	9:KOM.3 (1.2 L	503.181	3.28E 3	-1.05E 3	-2.27E 3	-58.630	-1.27E 3
	10:KOM.4 (1.2	499.891	4.45E 3	-736.425	-1.49E 3	-58.837	-1.26E 3
	11:KOM.5 (0.9	314.363	-268.128	-884.542	-1.91E 3	-42.500	-847.828
	12:KOM.6 (0.9	311.073	905.115	-571.028	-1.14E 3	-42.707	-838.472
	13:KOM.7 (1.2	506.308	4.47E 3	-736.625	-1.49E 3	-58.815	-1.28E 3
	14:KOM.8 (1.2	506.308	4.47E 3	-736.625	-1.49E 3	-58.815	-1.28E 3
	15:KOM.9 (1.2	506.308	4.47E 3	-736.625	-1.49E 3	-58.815	-1.28E 3
	16:KOM.10 (1.:	506.308	4.47E 3	-736.625	-1.49E 3	-58.815	-1.28E 3
	17:KOM.11 (0.1	317.490	916.925	-571.228	-1.14E 3	-42.685	-857.135
	18:KOM.12 (0.1	317.490	916.925	-571.228	-1.14E 3	-42.685	-857.135
	19:KOM.13 (0.1	317.490	916.925	-571.228	-1.14E 3	-42.685	-857.135
	20:KOM.14 (0.1	317.490	916.925	-571.228	-1.14E 3	-42.685	-857.135
3	1:BEBA MAT	87.531	-147.360	-541.377	-1.07E 3	41.599	-408.017
	2:BEBA HIDL	-59.015	3.2E 3	27.797	31.072	1.792	93.651
	3:BEBA GEV	-618.327	-4.36E 3	-1.79E 3	-4.36E 3	2.496	1.76E 3
	4:BEBA GEV	618.327	4.36E 3	1.79E 3	4.36E 3	-2.496	-1.76E 3
	5:BEBA ANG	-7.154	-746.593	-210.742	-520.214	0.964	19.084
	6:BEBA ANG	-3.999	6.403	-0.159	-0.177	-0.014	11.640
	7:KOM.1 (1.4 L	94.543	-206.304	-757.927	-1.5E 3	58.239	-571.224
	8:KOM.2 (1.2 L	-29.387	4.94E 3	-605.177	-1.24E 3	52.787	-339.459
	9:KOM.3 (1.2 L	0.576	1.82E 3	-959.042	-2.09E 3	53.254	-365.236
	10:KOM.4 (1.2	5.623	3.03E 3	-622.110	-1.26E 3	51.689	-377.146
	11:KOM.5 (0.9	49.331	-1.33E 3	-824.426	-1.8E 3	38.982	-336.681
	12:KOM.6 (0.9	54.379	-122.379	-487.494	-966.805	37.417	-348.592
	13:KOM.7 (1.2	12.022	3.02E 3	-621.855	-1.26E 3	51.711	-395.770
	14:KOM.8 (1.2	12.022	3.02E 3	-621.855	-1.26E 3	51.711	-395.770
	15:KOM.9 (1.2	12.022	3.02E 3	-621.855	-1.26E 3	51.711	-395.770
	16:KOM.10 (1.:	12.022	3.02E 3	-621.855	-1.26E 3	51.711	-395.770
	17:KOM.11 (0.1	60.778	-132.624	-487.239	-966.521	37.439	-367.215
	18:KOM.12 (0.1	60.778	-132.624	-487.239	-966.521	37.439	-367.215
	19:KOM.13 (0.1	60.778	-132.624	-487.239	-966.521	37.439	-367.215
	20:KOM.14 (0.1	60.778	-132.624	-487.239	-966.521	37.439	-367.215
5	1:BEBA MAT	-114.703	15.4E 3	-38.100	-202.933	1.705	430.483
	2:BEBA HIDL	-9.469	1.37E 3	-4.856	-23.736	0.021	41.213
	3:BEBA GEV	-56.521	311.675	-140.348	-993.536	-1.473	413.952
	4:BEBA GEV	56.521	-311.675	140.348	993.536	1.473	-413.952
	5:BEBA ANG	1.151	52.027	-21.663	-152.889	-0.498	-4.674
	6:BEBA ANG	-0.291	26.245	0.127	0.604	-0.116	2.750
	7:KOM.1 (1.4 L	-160.585	21.6E 3	-53.340	-284.106	2.388	602.676
	8:KOM.2 (1.2 L	-152.794	20.7E 3	-53.489	-281.500	2.081	582.520
	9:KOM.3 (1.2 L	-145.271	20E 3	-85.235	-511.880	1.271	550.314



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Part		
Ref		
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Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

### Reactions Cont...

Node	L/C	Horizontal		Vertical	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	10:KOM.4 (1.2	-147.578	19.9E 3	-50.372	-266.291	1.882	562.192
	11:KOM.5 (0.9	-101.391	14E 3	-68.950	-427.263	0.738	379.957
	12:KOM.6 (0.9	-103.698	13.9E 3	-34.086	-181.674	1.349	391.834
	13:KOM.7 (1.2	-147.113	19.9E 3	-50.575	-267.257	2.068	557.792
	14:KOM.8 (1.2	-147.113	19.9E 3	-50.575	-267.257	2.068	557.792
	15:KOM.9 (1.2	-147.113	19.9E 3	-50.575	-267.257	2.068	557.792
	16:KOM.10 (1.	-147.113	19.9E 3	-50.575	-267.257	2.068	557.792
	17:KOM.11 (0.9	-103.233	13.9E 3	-34.290	-182.640	1.535	387.435
	18:KOM.12 (0.9	-103.233	13.9E 3	-34.290	-182.640	1.535	387.435
	19:KOM.13 (0.9	-103.233	13.9E 3	-34.290	-182.640	1.535	387.435
	20:KOM.14 (0.9	-103.233	13.9E 3	-34.290	-182.640	1.535	387.435
7	1:BEBAN MAT	-203.151	22.8E 3	9.935	21.065	-0.070	845.640
	2:BEBAN HIDL	-9.751	1.39E 3	3.932	17.244	-0.078	42.633
	3:BEBAN GEV	-71.755	456.066	-139.156	-987.980	-1.215	480.634
	4:BEBAN GEV	71.755	-456.066	139.156	987.980	1.215	-480.634
	5:BEBAN ANG	-2.072	9.542	-21.550	-152.366	-0.496	9.419
	6:BEBAN ANG	-0.297	26.733	-0.122	-0.564	0.112	2.782
	7:KOM.1 (1.4 [	-284.411	31.9E 3	13.909	29.491	-0.099	1.18E 3
	8:KOM.2 (1.2 [	-259.383	29.6E 3	18.214	52.869	-0.209	1.08E 3
	9:KOM.3 (1.2 [	-256.848	28.8E 3	-18.626	-201.263	-0.956	1.07E 3
	10:KOM.4 (1.2	-254.007	28.8E 3	15.660	41.619	0.017	1.06E 3
	11:KOM.5 (0.9	-186.152	20.5E 3	-25.539	-224.827	-0.857	776.146
	12:KOM.6 (0.9	-183.311	20.6E 3	8.747	18.055	0.116	765.528
	13:KOM.7 (1.2	-253.532	28.8E 3	15.854	42.522	-0.162	1.06E 3
	14:KOM.8 (1.2	-253.532	28.8E 3	15.854	42.522	-0.162	1.06E 3
	15:KOM.9 (1.2	-253.532	28.8E 3	15.854	42.522	-0.162	1.06E 3
	16:KOM.10 (1.	-253.532	28.8E 3	15.854	42.522	-0.162	1.06E 3
	17:KOM.11 (0.9	-182.836	20.5E 3	8.942	18.958	-0.063	761.076
	18:KOM.12 (0.9	-182.836	20.5E 3	8.942	18.958	-0.063	761.076
	19:KOM.13 (0.9	-182.836	20.5E 3	8.942	18.958	-0.063	761.076
	20:KOM.14 (0.9	-182.836	20.5E 3	8.942	18.958	-0.063	761.076
9	1:BEBAN MAT	174.769	349.178	-112.397	-447.412	29.372	-674.566
	2:BEBAN HIDL	-89.365	3.23E 3	-50.401	-83.385	0.310	93.323
	3:BEBAN GEV	-519.320	5.48E 3	-1.73E 3	-4.28E 3	3.689	1.51E 3
	4:BEBAN GEV	519.320	-5.48E 3	1.73E 3	4.28E 3	-3.689	-1.51E 3
	5:BEBAN ANG	8.821	892.759	-210.054	-519.047	0.790	-22.255
	6:BEBAN ANG	-4.066	6.262	0.268	0.431	0.004	11.802
	7:KOM.1 (1.4 [	244.676	488.849	-157.355	-626.377	41.121	-944.393
	8:KOM.2 (1.2 [	98.739	5.59E 3	-215.518	-670.310	35.743	-660.162
	9:KOM.3 (1.2 [	154.472	5.08E 3	-521.363	-1.45E 3	36.820	-751.785
	10:KOM.4 (1.2	133.852	3.66E 3	-184.847	-619.590	35.563	-697.273
	11:KOM.5 (0.9	171.406	1.74E 3	-437.243	-1.23E 3	27.698	-642.718
	12:KOM.6 (0.9	150.786	324.280	-100.727	-401.981	26.441	-588.226
	13:KOM.7 (1.2	140.358	3.65E 3	-185.277	-620.279	35.557	-716.156
	14:KOM.8 (1.2	140.358	3.65E 3	-185.277	-620.279	35.557	-716.156
	15:KOM.9 (1.2	140.358	3.65E 3	-185.277	-620.279	35.557	-716.156
	16:KOM.10 (1.	140.358	3.65E 3	-185.277	-620.279	35.557	-716.156
	17:KOM.11 (0.9	157.292	314.260	-101.157	-402.671	26.435	-607.110
	18:KOM.12 (0.9	157.292	314.260	-101.157	-402.671	26.435	-607.110



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Part		
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Client	File endhik-2 slp 4.std	Date/Time 21-Mar-2011 15:52

## Reactions Cont...

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	19:KOM.13 (0.)	157.292	314.260	-101.157	-402.671	26.435	-607.110
	20:KOM.14 (0.)	157.292	314.260	-101.157	-402.671	26.435	-607.110
11	1:BEBAN MAT	90.775	1.63E 3	-141.197	-548.478	-36.824	-291.050
	2:BEBAN HIDL	75.906	3.28E 3	-52.239	-88.310	-0.684	-118.719
	3:BEBAN GEV	-557.998	4.19E 3	-1.68E 3	-4.1E 3	7.253	1.59E 3
	4:BEBAN GEV	557.998	-4.19E 3	1.68E 3	4.1E 3	-7.253	-1.59E 3
	5:BEBAN ANG	1.146	881.213	-195.185	-482.610	0.406	-3.385
	6:BEBAN ANG	-4.077	-7.117	-0.274	-0.459	0.004	11.824
	7:KOM.1 (1.4 [	127.085	2.29E 3	-197.676	-767.869	-51.554	-407.470
	8:KOM.2 (1.2 [	230.380	7.21E 3	-253.019	-799.470	-45.283	-539.211
	9:KOM.3 (1.2 [	186.669	6.65E 3	-533.972	-1.52E 3	-44.222	-473.395
	10:KOM.4 (1.2	178.314	5.23E 3	-222.114	-747.219	-44.867	-449.061
	11:KOM.5 (0.9	83.531	2.88E 3	-439.374	-1.27E 3	-32.492	-267.361
	12:KOM.6 (0.9	75.175	1.46E 3	-127.516	-494.365	-33.136	-243.027
	13:KOM.7 (1.2	184.836	5.24E 3	-221.675	-746.484	-44.873	-467.979
	14:KOM.8 (1.2	184.836	5.24E 3	-221.675	-746.484	-44.873	-467.979
	15:KOM.9 (1.2	184.836	5.24E 3	-221.675	-746.484	-44.873	-467.979
	16:KOM.10 (1.)	184.836	5.24E 3	-221.675	-746.484	-44.873	-467.979
	17:KOM.11 (0.)	81.698	1.47E 3	-127.077	-493.630	-33.142	-261.945
	18:KOM.12 (0.)	81.698	1.47E 3	-127.077	-493.630	-33.142	-261.945
	19:KOM.13 (0.)	81.698	1.47E 3	-127.077	-493.630	-33.142	-261.945
	20:KOM.14 (0.)	81.698	1.47E 3	-127.077	-493.630	-33.142	-261.945
13	1:BEBAN MAT	289.625	22.8E 3	8.045	4.148	-2.888	-1.46E 3
	2:BEBAN HIDL	11.946	1.38E 3	3.882	16.704	-0.076	-58.503
	3:BEBAN GEV	-53.714	-72.361	-133.280	-942.702	6.433	397.970
	4:BEBAN GEV	53.714	72.361	133.280	942.702	-6.433	-397.970
	5:BEBAN ANG	1.672	-20.104	-20.485	-144.682	1.361	-7.758
	6:BEBAN ANG	-0.204	19.896	-0.053	-0.252	0.112	2.350
	7:KOM.1 (1.4 [	405.474	31.9E 3	11.263	5.808	-4.043	-2.05E 3
	8:KOM.2 (1.2 [	366.663	29.6E 3	15.866	31.705	-3.586	-1.85E 3
	9:KOM.3 (1.2 [	362.171	28.7E 3	-19.240	-209.809	-1.363	-1.83E 3
	10:KOM.4 (1.2	359.169	28.8E 3	13.453	21.280	-3.361	-1.81E 3
	11:KOM.5 (0.9	263.337	20.5E 3	-25.536	-227.758	-0.421	-1.33E 3
	12:KOM.6 (0.9	260.335	20.5E 3	7.157	3.331	-2.419	-1.31E 3
	13:KOM.7 (1.2	359.495	28.7E 3	13.537	21.683	-3.541	-1.81E 3
	14:KOM.8 (1.2	359.495	28.7E 3	13.537	21.683	-3.541	-1.81E 3
	15:KOM.9 (1.2	359.495	28.7E 3	13.537	21.683	-3.541	-1.81E 3
	16:KOM.10 (1.)	359.495	28.7E 3	13.537	21.683	-3.541	-1.81E 3
	17:KOM.11 (0.)	260.662	20.5E 3	7.241	3.734	-2.599	-1.32E 3
	18:KOM.12 (0.)	260.662	20.5E 3	7.241	3.734	-2.599	-1.32E 3
	19:KOM.13 (0.)	260.662	20.5E 3	7.241	3.734	-2.599	-1.32E 3
	20:KOM.14 (0.)	260.662	20.5E 3	7.241	3.734	-2.599	-1.32E 3
15	1:BEBAN MAT	203.203	15.7E 3	-42.223	-230.297	-3.939	-1.06E 3
	2:BEBAN HIDL	11.734	1.36E 3	-6.057	-24.995	-0.096	-57.815
	3:BEBAN GEV	-67.385	41.054	-131.939	-936.474	6.045	463.549
	4:BEBAN GEV	67.385	-41.054	131.939	936.474	-6.045	-463.549
	5:BEBAN ANG	-1.206	72.639	-20.625	-145.333	1.358	6.038
	6:BEBAN ANG	-0.194	20.749	0.044	0.189	-0.117	2.299
	7:KOM.1 (1.4 [	284.484	21.9E 3	-59.113	-322.416	-5.514	-1.48E 3



Software licensed to Snow Panther [LZO]

Job No	Sheet No <b>4</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
File endhik-2 sip 4.std		Date/Time 21-Mar-2011 15:52

Job Title	Ref
Cilent	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	LC	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	8:KOM.2 (1.2 [	262.618	21E 3	-58.759	-316.349	-4.881	-1.36E 3
	9:KOM.3 (1.2 [	253.649	20.3E 3	-88.725	-533.885	-2.650	-1.32E 3
	10:KOM.4 (1.2	255.267	20.2E 3	-55.655	-301.049	-5.009	-1.32E 3
	11:KOM.5 (0.9	180.953	14.2E 3	-71.001	-439.801	-1.372	-943.077
	12:KOM.6 (0.9	182.572	14.1E 3	-37.931	-206.965	-3.731	-949.060
	13:KOM.7 (1.2	255.578	20.2E 3	-55.725	-301.352	-4.823	-1.33E 3
	14:KOM.8 (1.2	255.578	20.2E 3	-55.725	-301.352	-4.823	-1.33E 3
	15:KOM.9 (1.2	255.578	20.2E 3	-55.725	-301.352	-4.823	-1.33E 3
	16:KOM.10 (1.:	255.578	20.2E 3	-55.725	-301.352	-4.823	-1.33E 3
	17:KOM.11 (0.!	182.882	14.1E 3	-38.001	-207.268	-3.545	-952.738
	18:KOM.12 (0.!	182.882	14.1E 3	-38.001	-207.268	-3.545	-952.738
	19:KOM.13 (0.!	182.882	14.1E 3	-38.001	-207.268	-3.545	-952.738
	20:KOM.14 (0.!	182.882	14.1E 3	-38.001	-207.268	-3.545	-952.738
17	1:BEBAN MAT	310.054	10.9E 3	-95.221	-378.943	-43.340	-878.757
	2:BEBAN HIDU	1.573	6.76E 3	105.178	145.763	-1.630	-18.224
	3:BEBAN GEV	-716.783	-5.37E 3	-1.68E 3	-4.11E 3	1.140	1.89E 3
	4:BEBAN GEV	716.783	5.37E 3	1.68E 3	4.11E 3	-1.140	-1.89E 3
	5:BEBAN ANG	-1.270	-628.374	-198.471	-487.717	-0.100	4.526
	6:BEBAN ANG	-4.847	-2.658	-0.060	-0.141	-0.009	12.883
	7:KOM.1 (1.4 [	434.075	15.2E 3	-133.309	-530.520	-60.675	-1.23E 3
	8:KOM.2 (1.2 [	374.582	23.9E 3	54.020	-221.511	-54.616	-1.08E 3
	9:KOM.3 (1.2 [	371.606	18.8E 3	-326.641	-1.09E 3	-53.798	-1.07E 3
	10:KOM.4 (1.2	365.883	19.8E 3	-9.183	-309.195	-53.651	-1.05E 3
	11:KOM.5 (0.9	277.017	8.78E 3	-403.253	-1.12E 3	-39.166	-783.639
	12:KOM.6 (0.9	271.293	9.78E 3	-85.795	-341.274	-39.019	-770.268
	13:KOM.7 (1.2	373.638	19.8E 3	-9.087	-308.969	-53.638	-1.07E 3
	14:KOM.8 (1.2	373.638	19.8E 3	-9.087	-308.969	-53.638	-1.07E 3
	15:KOM.9 (1.2	373.638	19.8E 3	-9.087	-308.969	-53.638	-1.07E 3
	16:KOM.10 (1.:	373.638	19.8E 3	-9.087	-308.969	-53.638	-1.07E 3
	17:KOM.11 (0.!	279.049	9.79E 3	-85.698	-341.049	-39.006	-790.881
	18:KOM.12 (0.!	279.049	9.79E 3	-85.698	-341.049	-39.006	-790.881
	19:KOM.13 (0.!	279.049	9.79E 3	-85.698	-341.049	-39.006	-790.881
	20:KOM.14 (0.!	279.049	9.79E 3	-85.698	-341.049	-39.006	-790.881
19	1:BEBAN MAT	35.817	11.3E 3	-450.138	-911.115	-36.588	-225.480
	2:BEBAN HIDU	-5.811	6.8E 3	-123.953	-190.826	-0.733	-0.691
	3:BEBAN GEV	-675.460	4.95E 3	-1.69E 3	-4.12E 3	4.529	1.77E 3
	4:BEBAN GEV	675.460	-4.95E 3	1.69E 3	4.12E 3	-4.529	-1.77E 3
	5:BEBAN ANG	0.380	890.126	-198.971	-488.804	0.204	-1.926
	6:BEBAN ANG	-4.919	-2.391	-0.066	-0.153	-0.002	13.050
	7:KOM.1 (1.4 [	50.143	15.9E 3	-630.193	-1.28E 3	-51.223	-315.672
	8:KOM.2 (1.2 [	33.683	24.5E 3	-738.490	-1.4E 3	-45.078	-271.682
	9:KOM.3 (1.2 [	37.745	21.8E 3	-982.472	-2.07E 3	-44.312	-274.348
	10:KOM.4 (1.2	29.299	20.4E 3	-664.224	-1.28E 3	-44.642	-250.387
	11:KOM.5 (0.9	32.810	11.6E 3	-723.478	-1.6E 3	-32.602	-206.013
	12:KOM.6 (0.9	24.365	10.2E 3	-405.230	-820.248	-32.932	-182.051
	13:KOM.7 (1.2	37.169	20.4E 3	-664.119	-1.28E 3	-44.638	-271.267
	14:KOM.8 (1.2	37.169	20.4E 3	-664.119	-1.28E 3	-44.638	-271.267
	15:KOM.9 (1.2	37.169	20.4E 3	-664.119	-1.28E 3	-44.638	-271.267
	16:KOM.10 (1.:	37.169	20.4E 3	-664.119	-1.28E 3	-44.638	-271.267



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Job No	Sheet No <b>5</b>	Rev
Part		
Ref		
By	Date <b>06-Oct-10</b>	Chd
Client	File <b>endhik-2 sip 4.std</b>	Date/Time <b>21-Mar-2011 15:52</b>

**Reactions Cont...**

Node	L/C	Horizontal		Vertical	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg'm)	MY (kg'm)	MZ (kg'm)
	17:KOM.11 (0.1	32.235	10.2E 3	-405.124	-820.004	-32.929	-202.932
	18:KOM.12 (0.1	32.235	10.2E 3	-405.124	-820.004	-32.929	-202.932
	19:KOM.13 (0.1	32.235	10.2E 3	-405.124	-820.004	-32.929	-202.932
	20:KOM.14 (0.1	32.235	10.2E 3	-405.124	-820.004	-32.929	-202.932
21	1:BEAN MAT	298.207	11.3E 3	59.739	-83.316	-33.390	-840.593
	2:BEAN HIDL	11.465	6.61E 3	117.943	167.245	-1.336	-31.341
	3:BEAN GEM	-719.293	-5.96E 3	-1.72E 3	-4.17E 3	0.582	1.89E 3
	4:BEAN GEM	719.293	5.96E 3	1.72E 3	4.17E 3	-0.582	-1.89E 3
	5:BEAN ANG	-1.170	-751.753	-201.952	-492.358	-0.288	4.065
	6:BEAN ANG	-4.886	-0.006	-0.064	-0.133	-0.006	12.947
	7:KOM.1 (1.4 Γ	417.490	15.8E 3	83.635	-116.642	-46.746	-1.18E 3
	8:KOM.2 (1.2 Γ	376.192	24.1E 3	260.396	167.613	-42.205	-1.06E 3
	9:KOM.3 (1.2 Γ	367.442	19E 3	-133.494	-720.506	-41.865	-1.03E 3
	10:KOM.4 (1.2	361.496	20.2E 3	189.528	67.054	-41.414	-1.02E 3
	11:KOM.5 (0.9	266.515	8.96E 3	-269.358	-862.756	-30.512	-750.029
	12:KOM.6 (0.9	260.569	10.2E 3	53.663	-75.196	-30.061	-735.818
	13:KOM.7 (1.2	369.313	20.2E 3	189.630	67.266	-41.404	-1.04E 3
	14:KOM.8 (1.2	369.313	20.2E 3	189.630	67.266	-41.404	-1.04E 3
	15:KOM.9 (1.2	369.313	20.2E 3	189.630	67.266	-41.404	-1.04E 3
	16:KOM.10 (1.:	369.313	20.2E 3	189.630	67.266	-41.404	-1.04E 3
	17:KOM.11 (0.1	268.387	10.2E 3	53.765	-74.984	-30.051	-756.534
	18:KOM.12 (0.1	268.387	10.2E 3	53.765	-74.984	-30.051	-756.534
	19:KOM.13 (0.1	268.387	10.2E 3	53.765	-74.984	-30.051	-756.534
	20:KOM.14 (0.1	268.387	10.2E 3	53.765	-74.984	-30.051	-756.534
23	1:BEAN MAT	76.333	12.1E 3	-458.977	-864.962	-27.987	-309.135
	2:BEAN HIDL	6.464	6.66E 3	-132.075	-201.925	-0.275	-18.957
	3:BEAN GEM	-684.649	5.93E 3	-1.72E 3	-4.18E 3	2.157	1.78E 3
	4:BEAN GEM	684.649	-5.93E 3	1.72E 3	4.18E 3	-2.157	-1.78E 3
	5:BEAN ANG	-0.090	1.02E 3	-202.874	-494.467	-0.147	-0.772
	6:BEAN ANG	-4.957	0.281	-0.040	-0.109	-0.004	13.111
	7:KOM.1 (1.4 Γ	106.886	16.9E 3	-642.568	-1.21E 3	-39.182	-432.788
	8:KOM.2 (1.2 Γ	101.943	25.2E 3	-762.092	-1.36E 3	-34.025	-401.292
	9:KOM.3 (1.2 Γ	97.920	22.8E 3	-1.01E 3	-2.03E 3	-34.096	-391.153
	10:KOM.4 (1.2	90.133	21.2E 3	-682.910	-1.24E 3	-33.866	-368.940
	11:KOM.5 (0.9	68.555	12.5E 3	-737.678	-1.57E 3	-25.424	-279.455
	12:KOM.6 (0.9	60.768	10.9E 3	-413.143	-778.639	-25.195	-257.243
	13:KOM.7 (1.2	98.064	21.2E 3	-682.847	-1.24E 3	-33.860	-389.918
	14:KOM.8 (1.2	98.064	21.2E 3	-682.847	-1.24E 3	-33.860	-389.918
	15:KOM.9 (1.2	98.064	21.2E 3	-682.847	-1.24E 3	-33.860	-389.918
	16:KOM.10 (1.:	98.064	21.2E 3	-682.847	-1.24E 3	-33.860	-389.918
	17:KOM.11 (0.1	68.700	10.9E 3	-413.079	-778.465	-25.188	-278.221
	18:KOM.12 (0.1	68.700	10.9E 3	-413.079	-778.465	-25.188	-278.221
	19:KOM.13 (0.1	68.700	10.9E 3	-413.079	-778.465	-25.188	-278.221
	20:KOM.14 (0.1	68.700	10.9E 3	-413.079	-778.465	-25.188	-278.221
25	1:BEAN MAT	215.112	11E 3	-27.276	-235.587	37.876	-631.433
	2:BEAN HIDL	6.983	6.69E 3	118.333	166.684	1.601	-16.743
	3:BEAN GEM	-744.023	-5.24E 3	-1.77E 3	-4.32E 3	5.159	1.94E 3
	4:BEAN GEM	744.023	5.24E 3	1.77E 3	4.32E 3	-5.159	-1.94E 3
	5:BEAN ANG	-9.258	-669.134	-210.972	-518.281	1.183	22.373





Software licensed to Snow Panther (LZO)

Job No	Sheet No <b>6</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
File	endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

Job Title	Ref
Client	File

**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	6:BEAN ANG	-4.828	1.947	0.025	0.071	-0.009	12.846
	7:KOM.1 (1.4 E	301.157	15.3E 3	-38.186	-329.822	53.026	-884.007
	8:KOM.2 (1.2 E	269.308	24.2E 3	156.601	-16.010	48.012	-784.509
	9:KOM.3 (1.2 E	250.305	19E 3	-251.953	-945.270	48.944	-738.666
	10:KOM.4 (1.2	257.394	20E 3	85.642	-115.907	47.037	-753.909
	11:KOM.5 (0.9	178.788	8.8E 3	-362.103	-1.04E 3	35.981	-532.493
	12:KOM.6 (0.9	185.877	9.87E 3	-24.508	-211.915	34.074	-547.736
	13:KOM.7 (1.2	265.118	20E 3	85.602	-116.021	47.052	-774.463
	14:KOM.8 (1.2	265.118	20E 3	85.602	-116.021	47.052	-774.463
	15:KOM.9 (1.2	265.118	20E 3	85.602	-116.021	47.052	-774.463
	16:KOM.10 (1.:	265.118	20E 3	85.602	-116.021	47.052	-774.463
	17:KOM.11 (0.:	193.601	9.87E 3	-24.548	-212.029	34.088	-568.290
	18:KOM.12 (0.:	193.601	9.87E 3	-24.548	-212.029	34.088	-568.290
	19:KOM.13 (0.:	193.601	9.87E 3	-24.548	-212.029	34.088	-568.290
	20:KOM.14 (0.:	193.601	9.87E 3	-24.548	-212.029	34.088	-568.290
27	1:BEAN MAT	317.769	11.2E 3	-431.231	-843.203	28.863	-867.143
	2:BEAN HIDL	6.489	6.92E 3	-133.984	-204.192	0.264	-16.111
	3:BEAN GEV	-618.649	5.56E 3	-1.77E 3	-4.32E 3	4.974	1.65E 3
	4:BEAN GEV	618.649	-5.56E 3	1.77E 3	4.32E 3	-4.974	-1.65E 3
	5:BEAN ANG	10.834	931.781	-211.277	-518.932	1.038	-25.531
	6:BEAN ANG	-4.901	1.810	0.060	0.126	-0.002	13.019
	7:KOM.1 (1.4 E	444.877	15.7E 3	-603.723	-1.18E 3	40.128	-1.21E 3
	8:KOM.2 (1.2 E	391.705	24.5E 3	-731.851	-1.34E 3	34.817	-1.07E 3
	9:KOM.3 (1.2 E	405.146	21.8E 3	-989.504	-2.05E 3	36.319	-1.1E 3
	10:KOM.4 (1.2	379.970	20.4E 3	-651.365	-1.22E 3	34.856	-1.04E 3
	11:KOM.5 (0.9	303.326	11.6E 3	-726.150	-1.59E 3	27.456	-821.278
	12:KOM.6 (0.9	278.150	10.1E 3	-388.011	-758.681	25.793	-759.599
	13:KOM.7 (1.2	387.812	20.4E 3	-651.461	-1.22E 3	34.859	-1.06E 3
	14:KOM.8 (1.2	387.812	20.4E 3	-651.461	-1.22E 3	34.859	-1.06E 3
	15:KOM.9 (1.2	387.812	20.4E 3	-651.461	-1.22E 3	34.859	-1.06E 3
	16:KOM.10 (1.:	387.812	20.4E 3	-651.461	-1.22E 3	34.859	-1.06E 3
	17:KOM.11 (0.:	285.992	10.1E 3	-388.107	-758.883	25.796	-780.429
	18:KOM.12 (0.:	285.992	10.1E 3	-388.107	-758.883	25.796	-780.429
	19:KOM.13 (0.:	285.992	10.1E 3	-388.107	-758.883	25.796	-780.429
	20:KOM.14 (0.:	285.992	10.1E 3	-388.107	-758.883	25.796	-780.429
29	1:BEAN MAT	287.510	11.4E 3	118.481	53.077	-23.807	-804.963
	2:BEAN HIDL	10.793	6.54E 3	121.747	174.666	-0.725	-28.670
	3:BEAN GEV	-712.988	-5.85E 3	-1.73E 3	-4.17E 3	0.019	1.88E 3
	4:BEAN GEV	712.988	5.85E 3	1.73E 3	4.17E 3	-0.019	-1.88E 3
	5:BEAN ANG	-0.730	-779.061	-202.147	-491.964	-0.360	3.398
	6:BEAN ANG	-4.884	0.081	-0.023	-0.064	-0.004	12.951
	7:KOM.1 (1.4 E	402.514	15.9E 3	165.873	74.308	-33.329	-1.13E 3
	8:KOM.2 (1.2 E	362.281	24.1E 3	336.972	343.159	-29.728	-1.01E 3
	9:KOM.3 (1.2 E	354.637	18.9E 3	-59.512	-548.783	-29.869	-989.189
	10:KOM.4 (1.2	347.990	20.2E 3	263.887	238.256	-29.299	-973.904
	11:KOM.5 (0.9	257.591	8.88E 3	-216.803	-739.373	-22.002	-719.030
	12:KOM.6 (0.9	250.944	10.2E 3	106.596	47.666	-21.432	-703.746
	13:KOM.7 (1.2	355.805	20.2E 3	263.924	238.359	-29.293	-994.625
	14:KOM.8 (1.2	355.805	20.2E 3	263.924	238.359	-29.293	-994.625



Software licensed to Snow Panther (LZO)

Job No	Sheet No <b>7</b>	Rev
Part		
Ref		
By	Date: 06-Oct-10	Chd
Client	File: endhik-2 sip 4.std	Date/Time: 21-Mar-2011 15:52

## Reactions Cont...

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	15:KOM.9 (1.2	355.805	20.2E 3	263.924	238.359	-29.293	-994.625
	16:KOM.10 (1.2	355.805	20.2E 3	263.924	238.359	-29.293	-994.625
	17:KOM.11 (0.1	258.759	10.2E 3	106.633	47.769	-21.426	-724.466
	18:KOM.12 (0.1	258.759	10.2E 3	106.633	47.769	-21.426	-724.466
	19:KOM.13 (0.1	258.759	10.2E 3	106.633	47.769	-21.426	-724.466
	20:KOM.14 (0.1	258.759	10.2E 3	106.633	47.769	-21.426	-724.466
31	1:BEBAN MAT	109.607	12.3E 3	-411.593	-750.204	-21.618	-378.681
	2:BEBAN HIDU	7.409	6.59E 3	-133.651	-203.745	-0.316	-19.958
	3:BEBAN GEV	-690.639	5.88E 3	-1.74E 3	-4.2E 3	0.386	1.8E 3
	4:BEBAN GEV	690.639	-5.88E 3	1.74E 3	4.2E 3	-0.386	-1.8E 3
	5:BEBAN ANG	-0.612	1.04E 3	-203.271	-494.517	-0.320	0.136
	6:BEBAN ANG	-4.953	0.355	-0.060	-0.131	-0.005	13.107
	7:KOM.1 (1.4 [	153.449	17.2E 3	-576.231	-1.05E 3	-30.266	-530.153
	8:KOM.2 (1.2 [	143.362	25.3E 3	-707.753	-1.23E 3	-26.448	-486.350
	9:KOM.3 (1.2 [	137.958	23E 3	-952.796	-1.9E 3	-26.770	-474.158
	10:KOM.4 (1.2	131.012	21.4E 3	-627.659	-1.1E 3	-26.266	-453.403
	11:KOM.5 (0.9	97.667	12.7E 3	-695.667	-1.47E 3	-19.968	-340.596
	12:KOM.6 (0.9	90.722	11.1E 3	-370.531	-675.392	-19.464	-319.641
	13:KOM.7 (1.2	138.937	21.4E 3	-627.563	-1.1E 3	-26.258	-474.375
	14:KOM.8 (1.2	138.937	21.4E 3	-627.563	-1.1E 3	-26.258	-474.375
	15:KOM.9 (1.2	138.937	21.4E 3	-627.563	-1.1E 3	-26.258	-474.375
	16:KOM.10 (1.2	138.937	21.4E 3	-627.563	-1.1E 3	-26.258	-474.375
	17:KOM.11 (0.1	98.646	11.1E 3	-370.434	-675.183	-19.456	-340.813
	18:KOM.12 (0.1	98.646	11.1E 3	-370.434	-675.183	-19.456	-340.813
	19:KOM.13 (0.1	98.646	11.1E 3	-370.434	-675.183	-19.456	-340.813
	20:KOM.14 (0.1	98.646	11.1E 3	-370.434	-675.183	-19.456	-340.813
33	1:BEBAN MAT	263.045	10.9E 3	89.214	43.964	-14.794	-752.904
	2:BEBAN HIDU	11.752	5.66E 3	72.106	102.692	-0.078	-28.367
	3:BEBAN GEV	-701.360	-5.41E 3	-1.72E 3	-4.16E 3	-2.197	1.86E 3
	4:BEBAN GEV	701.360	5.41E 3	1.72E 3	4.16E 3	2.197	-1.86E 3
	5:BEBAN ANG	-0.902	-781.841	-200.488	-488.789	-0.360	3.875
	6:BEBAN ANG	-4.822	0.299	-0.016	-0.047	-0.003	12.863
	7:KOM.1 (1.4 [	368.263	15.3E 3	124.899	61.550	-20.711	-1.05E 3
	8:KOM.2 (1.2 [	334.457	22.1E 3	222.426	217.384	-17.877	-948.872
	9:KOM.3 (1.2 [	325.962	17.5E 3	-141.618	-626.413	-18.406	-925.652
	10:KOM.4 (1.2	319.690	18.7E 3	179.138	155.573	-17.835	-911.271
	11:KOM.5 (0.9	235.296	8.55E 3	-240.488	-742.494	-13.890	-671.413
	12:KOM.6 (0.9	229.025	9.8E 3	80.268	39.492	-13.319	-657.033
	13:KOM.7 (1.2	327.405	18.7E 3	179.162	155.649	-17.830	-931.852
	14:KOM.8 (1.2	327.405	18.7E 3	179.162	155.649	-17.830	-931.852
	15:KOM.9 (1.2	327.405	18.7E 3	179.162	155.649	-17.830	-931.852
	16:KOM.10 (1.2	327.405	18.7E 3	179.162	155.649	-17.830	-931.852
	17:KOM.11 (0.1	236.740	9.8E 3	80.292	39.568	-13.314	-677.613
	18:KOM.12 (0.1	236.740	9.8E 3	80.292	39.568	-13.314	-677.613
	19:KOM.13 (0.1	236.740	9.8E 3	80.292	39.568	-13.314	-677.613
	20:KOM.14 (0.1	236.740	9.8E 3	80.292	39.568	-13.314	-677.613
35	1:BEBAN MAT	123.431	11.8E 3	-304.696	-559.786	-15.580	-417.098
	2:BEBAN HIDU	9.330	5.71E 3	-82.730	-128.718	-0.480	-21.961
	3:BEBAN GEV	-683.891	5.46E 3	-1.73E 3	-4.19E 3	-2.537	1.79E 3



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Part		
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Job Title

Client

**Reactions Cont...**

Node	LC	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	4:BEAN GEV	683.891	-5.46E 3	1.73E 3	4.19E 3	2.537	-1.79E 3
	5:BEAN ANG	-0.313	1.05E 3	-201.637	-491.409	-0.366	-0.585
	6:BEAN ANG	-4.887	0.543	-0.049	-0.105	-0.005	13.011
	7:KOM.1 (1.4 [	172.804	16.6E 3	-426.574	-783.701	-21.812	-583.937
	8:KOM.2 (1.2 [	163.045	23.4E 3	-498.004	-877.691	-19.485	-535.655
	9:KOM.3 (1.2 [	156.947	21.6E 3	-770.984	-1.59E 3	-19.782	-523.414
	10:KOM.4 (1.2	149.629	19.9E 3	-448.445	-800.630	-19.184	-501.660
	11:KOM.5 (0.9	110.588	12.3E 3	-596.845	-1.29E 3	-14.608	-376.324
	12:KOM.6 (0.9	103.269	10.7E 3	-274.308	-503.976	-14.030	-354.570
	13:KOM.7 (1.2	157.447	19.9E 3	-448.365	-800.461	-19.176	-522.478
	14:KOM.8 (1.2	157.447	19.9E 3	-448.365	-800.461	-19.176	-522.478
	15:KOM.9 (1.2	157.447	19.9E 3	-448.365	-800.461	-19.176	-522.478
	16:KOM.10 (1.:	157.447	19.9E 3	-448.365	-800.461	-19.176	-522.478
	17:KOM.11 (0.:	111.088	10.7E 3	-274.226	-503.807	-14.022	-375.388
	18:KOM.12 (0.:	111.088	10.7E 3	-274.226	-503.807	-14.022	-375.388
	19:KOM.13 (0.:	111.088	10.7E 3	-274.226	-503.807	-14.022	-375.388
	20:KOM.14 (0.:	111.088	10.7E 3	-274.226	-503.807	-14.022	-375.388
37	1:BEAN MAT	230.389	10.5E 3	88.659	62.333	-7.472	-693.669
	2:BEAN HIDL	8.347	6.41E 3	96.238	137.699	0.420	-22.306
	3:BEAN GEV	-698.511	-5.82E 3	-1.7E 3	-4.12E 3	-4.303	1.85E 3
	4:BEAN GEV	698.511	5.82E 3	1.7E 3	4.12E 3	4.303	-1.85E 3
	5:BEAN ANG	-1.247	-769.052	-198.347	-484.951	-0.310	4.849
	6:BEAN ANG	-4.815	-0.490	-0.003	-0.024	-0.003	12.852
	7:KOM.1 (1.4 [	322.544	14.8E 3	124.122	87.267	-10.460	-971.136
	8:KOM.2 (1.2 [	289.822	22.9E 3	260.372	295.118	-8.293	-868.091
	9:KOM.3 (1.2 [	282.818	17.8E 3	-114.726	-563.422	-9.041	-846.949
	10:KOM.4 (1.2	277.110	19.1E 3	202.625	212.460	-8.550	-834.144
	11:KOM.5 (0.9	205.355	8.26E 3	-237.563	-719.821	-7.220	-616.543
	12:KOM.6 (0.9	199.646	9.49E 3	79.789	56.062	-6.729	-603.738
	13:KOM.7 (1.2	284.814	19.1E 3	202.629	212.499	-8.546	-854.708
	14:KOM.8 (1.2	284.814	19.1E 3	202.629	212.499	-8.546	-854.708
	15:KOM.9 (1.2	284.814	19.1E 3	202.629	212.499	-8.546	-854.708
	16:KOM.10 (1.:	284.814	19.1E 3	202.629	212.499	-8.546	-854.708
	17:KOM.11 (0.:	207.350	9.49E 3	79.793	56.100	-6.725	-624.302
	18:KOM.12 (0.:	207.350	9.49E 3	79.793	56.100	-6.725	-624.302
	19:KOM.13 (0.:	207.350	9.49E 3	79.793	56.100	-6.725	-624.302
	20:KOM.14 (0.:	207.350	9.49E 3	79.793	56.100	-6.725	-624.302
39	1:BEAN MAT	122.303	11.3E 3	-252.753	-459.646	-10.265	-431.754
	2:BEAN HIDL	6.264	6.45E 3	-106.013	-161.640	-0.623	-16.896
	3:BEAN GEV	-681.619	5.74E 3	-1.71E 3	-4.15E 3	-4.248	1.79E 3
	4:BEAN GEV	681.619	-5.74E 3	1.71E 3	4.15E 3	4.248	-1.79E 3
	5:BEAN ANG	0.266	1.03E 3	-199.297	-487.302	-0.303	-2.137
	6:BEAN ANG	-4.876	-0.320	-0.043	-0.088	-0.004	12.993
	7:KOM.1 (1.4 [	171.224	15.9E 3	-353.854	-643.504	-14.371	-604.455
	8:KOM.2 (1.2 [	156.785	23.9E 3	-472.924	-810.198	-13.316	-545.138
	9:KOM.3 (1.2 [	153.453	21.7E 3	-728.191	-1.49E 3	-13.426	-536.419
	10:KOM.4 (1.2	145.225	20E 3	-409.386	-713.355	-12.948	-514.211
	11:KOM.5 (0.9	110.499	11.8E 3	-546.353	-1.19E 3	-9.724	-391.997
	12:KOM.6 (0.9	102.271	10.2E 3	-227.547	-413.821	-9.245	-367.789



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Part		
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By	Date 06-Oct-10	Chd
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**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	13:KOM.7 (1.2	153.027	20E 3	-409.316	-713.214	-12.942	-535.000
	14:KOM.8 (1.2	153.027	20E 3	-409.316	-713.214	-12.942	-535.000
	15:KOM.9 (1.2	153.027	20E 3	-409.316	-713.214	-12.942	-535.000
	16:KOM.10 (1.2	153.027	20E 3	-409.316	-713.214	-12.942	-535.000
	17:KOM.11 (0.1	110.073	10.2E 3	-227.478	-413.681	-9.239	-388.578
	18:KOM.12 (0.1	110.073	10.2E 3	-227.478	-413.681	-9.239	-388.578
	19:KOM.13 (0.1	110.073	10.2E 3	-227.478	-413.681	-9.239	-388.578
	20:KOM.14 (0.1	110.073	10.2E 3	-227.478	-413.681	-9.239	-388.578
41	1:BEBAN MAT	240.814	6.28E 3	-75.546	-169.876	-0.632	-703.358
	2:BEBAN HIDU	5.878	6.59E 3	95.066	135.025	0.527	-18.662
	3:BEBAN GEV	-706.900	-5.37E 3	-1.63E 3	-4.02E 3	-4.659	1.87E 3
	4:BEBAN GEV	706.900	5.37E 3	1.63E 3	4.02E 3	4.659	-1.87E 3
	5:BEBAN ANG	-2.647	-694.286	-192.182	-475.329	-0.306	7.737
	6:BEBAN ANG	-4.814	0.069	0.062	0.074	-0.005	12.850
	7:KOM.1 (1.4 [	337.140	8.79E 3	-105.764	-237.826	-0.885	-984.701
	8:KOM.2 (1.2 [	298.382	18.1E 3	61.451	12.189	0.085	-873.888
	9:KOM.3 (1.2 [	290.620	13E 3	-303.048	-829.353	-0.720	-850.311
	10:KOM.4 (1.2	287.153	14.1E 3	4.510	-68.708	-0.239	-842.131
	11:KOM.5 (0.9	212.498	4.54E 3	-375.450	-913.415	-1.058	-620.643
	12:KOM.6 (0.9	209.030	5.85E 3	-67.893	-152.770	-0.577	-612.462
	13:KOM.7 (1.2	294.855	14.1E 3	4.411	-68.826	-0.231	-862.691
	14:KOM.8 (1.2	294.855	14.1E 3	4.411	-68.826	-0.231	-862.691
	15:KOM.9 (1.2	294.855	14.1E 3	4.411	-68.826	-0.231	-862.691
	16:KOM.10 (1.2	294.855	14.1E 3	4.411	-68.826	-0.231	-862.691
	17:KOM.11 (0.1	216.733	5.85E 3	-67.991	-152.888	-0.569	-633.022
	18:KOM.12 (0.1	216.733	5.85E 3	-67.991	-152.888	-0.569	-633.022
	19:KOM.13 (0.1	216.733	5.85E 3	-67.991	-152.888	-0.569	-633.022
	20:KOM.14 (0.1	216.733	5.85E 3	-67.991	-152.888	-0.569	-633.022
43	1:BEBAN MAT	153.511	7.03E 3	-63.214	-168.481	-4.825	-482.250
	2:BEBAN HIDU	3.652	6.63E 3	-104.663	-168.623	-0.420	-13.134
	3:BEBAN GEV	-669.353	5.36E 3	-1.66E 3	-4.06E 3	-2.089	1.76E 3
	4:BEBAN GEV	669.353	-5.36E 3	1.66E 3	4.06E 3	2.089	-1.76E 3
	5:BEBAN ANG	2.111	916.742	-192.723	-477.269	-0.099	-5.953
	6:BEBAN ANG	-4.873	0.016	-0.088	-0.146	-0.000	12.984
	7:KOM.1 (1.4 [	214.916	9.84E 3	-88.499	-235.874	-6.755	-689.150
	8:KOM.2 (1.2 [	190.057	19E 3	-243.317	-455.974	-6.462	-611.715
	9:KOM.3 (1.2 [	191.243	16.5E 3	-488.877	-1.12E 3	-6.388	-613.359
	10:KOM.4 (1.2	180.070	15.1E 3	-180.661	-361.034	-6.211	-583.060
	11:KOM.5 (0.9	141.537	7.79E 3	-365.250	-915.263	-4.500	-452.550
	12:KOM.6 (0.9	130.384	6.32E 3	-57.034	-151.867	-4.343	-422.251
	13:KOM.7 (1.2	187.866	15.1E 3	-180.519	-360.801	-6.210	-603.834
	14:KOM.8 (1.2	187.866	15.1E 3	-180.519	-360.801	-6.210	-603.834
	15:KOM.9 (1.2	187.866	15.1E 3	-180.519	-360.801	-6.210	-603.834
	16:KOM.10 (1.2	187.866	15.1E 3	-180.519	-360.801	-6.210	-603.834
	17:KOM.11 (0.1	138.160	6.32E 3	-56.892	-151.633	-4.342	-443.025
	18:KOM.12 (0.1	138.160	6.32E 3	-56.892	-151.633	-4.342	-443.025
	19:KOM.13 (0.1	138.160	6.32E 3	-56.892	-151.633	-4.342	-443.025
	20:KOM.14 (0.1	138.160	6.32E 3	-56.892	-151.633	-4.342	-443.025
45	1:BEBAN MAT	254.953	7.4E 3	73.980	48.652	1.359	-731.451



Software (licensed to Snow Panther (LZO))

Job No	Sheet No <b>10</b>	Rev
Part		
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**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	2:BEAN HIDL	8.106	6.74E 3	96.821	136.949	0.108	-23.066
	3:BEAN GEV	-726.872	-5.14E 3	-1.7E 3	-4.12E 3	-2.743	1.91E 3
	4:BEAN GEV	726.872	5.14E 3	1.7E 3	4.12E 3	2.743	-1.91E 3
	5:BEAN ANG	-5.035	-673.488	-200.468	-487.123	0.417	13.015
	6:BEAN ANG	-4.819	-1.388	-0.010	-0.027	-0.041	12.853
	7:KOM.1 (1.4 I	356.934	10.4E 3	103.572	65.313	1.903	-1.02E 3
	8:KOM.2 (1.2 I	318.913	19.7E 3	243.690	275.101	1.804	-914.646
	9:KOM.3 (1.2 I	305.993	14.5E 3	-135.152	-586.466	2.406	-879.982
	10:KOM.4 (1.2	306.339	15.6E 3	185.582	192.888	1.673	-880.242
	11:KOM.5 (0.9	221.401	5.58E 3	-254.167	-737.411	1.891	-637.481
	12:KOM.6 (0.9	221.747	6.66E 3	66.566	41.943	1.157	-637.741
	13:KOM.7 (1.2	314.049	15.6E 3	185.597	192.932	1.739	-900.807
	14:KOM.8 (1.2	314.049	15.6E 3	185.597	192.932	1.739	-900.807
	15:KOM.9 (1.2	314.049	15.6E 3	185.597	192.932	1.739	-900.807
	16:KOM.10 (1.	314.049	15.6E 3	185.597	192.932	1.739	-900.807
	17:KOM.11 (0.9	229.457	6.66E 3	66.582	41.987	1.223	-658.306
	18:KOM.12 (0.9	229.457	6.66E 3	66.582	41.987	1.223	-658.306
	19:KOM.13 (0.9	229.457	6.66E 3	66.582	41.987	1.223	-658.306
	20:KOM.14 (0.9	229.457	6.66E 3	66.582	41.987	1.223	-658.306
47	1:BEAN MAT	175.350	8.33E 3	-226.296	-402.494	-3.019	-536.493
	2:BEAN HIDL	4.927	6.77E 3	-108.080	-163.307	-0.071	-15.530
	3:BEAN GEV	-653.351	4.92E 3	-1.72E 3	-4.16E 3	6.112	1.72E 3
	4:BEAN GEV	653.351	-4.92E 3	1.72E 3	4.16E 3	-6.112	-1.72E 3
	5:BEAN ANG	4.133	942.932	-202.569	-491.629	0.128	-10.351
	6:BEAN ANG	-4.874	-1.177	-0.021	-0.043	0.038	12.980
	7:KOM.1 (1.4 I	245.490	11.7E 3	-316.814	-563.491	-4.226	-751.090
	8:KOM.2 (1.2 I	218.303	20.8E 3	-444.484	-744.283	-3.736	-668.640
	9:KOM.3 (1.2 I	221.959	18.3E 3	-703.746	-1.43E 3	-3.489	-675.883
	10:KOM.4 (1.2	207.549	16.8E 3	-379.669	-646.368	-3.633	-638.553
	11:KOM.5 (0.9	164.427	9E 3	-527.777	-1.15E 3	-2.512	-499.405
	12:KOM.6 (0.9	150.017	7.49E 3	-203.700	-362.313	-2.656	-462.075
	13:KOM.7 (1.2	215.347	16.8E 3	-379.635	-646.299	-3.694	-659.322
	14:KOM.8 (1.2	215.347	16.8E 3	-379.635	-646.299	-3.694	-659.322
	15:KOM.9 (1.2	215.347	16.8E 3	-379.635	-646.299	-3.694	-659.322
	16:KOM.10 (1.	215.347	16.8E 3	-379.635	-646.299	-3.694	-659.322
	17:KOM.11 (0.9	157.815	7.5E 3	-203.666	-362.244	-2.717	-482.844
	18:KOM.12 (0.9	157.815	7.5E 3	-203.666	-362.244	-2.717	-482.844
	19:KOM.13 (0.9	157.815	7.5E 3	-203.666	-362.244	-2.717	-482.844
	20:KOM.14 (0.9	157.815	7.5E 3	-203.666	-362.244	-2.717	-482.844
49	1:BEAN MAT	275.249	10.5E 3	130.814	138.103	-3.282	-774.664
	2:BEAN HIDL	10.373	6.75E 3	99.243	141.201	-0.640	-27.919
	3:BEAN GEV	-748.015	-5.07E 3	-1.73E 3	-4.18E 3	4.109	1.96E 3
	4:BEAN GEV	748.015	5.07E 3	1.73E 3	4.18E 3	-4.109	-1.96E 3
	5:BEAN ANG	-7.706	-662.892	-203.738	-493.723	1.135	18.671
	6:BEAN ANG	-4.815	1.023	-0.016	-0.033	-0.041	12.847
	7:KOM.1 (1.4 I	385.349	14.7E 3	183.139	193.345	-4.594	-1.08E 3
	8:KOM.2 (1.2 I	346.896	23.4E 3	315.765	391.646	-4.962	-974.266
	9:KOM.3 (1.2 I	328.343	18.3E 3	-69.761	-483.032	-2.761	-927.641
	10:KOM.4 (1.2	332.968	19.4E 3	256.193	306.873	-4.644	-936.960



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Job No	Sheet No <b>11</b>	Rev
Part		
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### Reactions Cont...

Node	L/C	Horizontal		Vertical	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	11:KOM.5 (0.9)	235.395	8.39E 3	-208.248	-665.664	-1.137	-667.323
	12:KOM.6 (0.9)	240.021	9.46E 3	117.707	124.241	-3.020	-676.643
	13:KOM.7 (1.2)	340.672	19.4E 3	256.219	306.925	-4.578	-957.515
	14:KOM.8 (1.2)	340.672	19.4E 3	256.219	306.925	-4.578	-957.515
	15:KOM.9 (1.2)	340.672	19.4E 3	256.219	306.925	-4.578	-957.515
	16:KOM.10 (1.2)	340.672	19.4E 3	256.219	306.925	-4.578	-957.515
	17:KOM.11 (0.9)	247.724	9.45E 3	117.732	124.293	-2.954	-697.197
	18:KOM.12 (0.9)	247.724	9.45E 3	117.732	124.293	-2.954	-697.197
	19:KOM.13 (0.9)	247.724	9.45E 3	117.732	124.293	-2.954	-697.197
	20:KOM.14 (0.9)	247.724	9.45E 3	117.732	124.293	-2.954	-697.197
51	1:BEBA MAT	191.372	11.3E 3	-267.882	-461.997	-0.984	-572.012
	2:BEBA HIDU	5.717	6.79E 3	-109.785	-166.106	0.364	-17.359
	3:BEBA GEV	-631.753	5.16E 3	-1.73E 3	-4.18E 3	10.295	1.68E 3
	4:BEBA GEV	631.753	-5.16E 3	1.73E 3	4.18E 3	-10.295	-1.68E 3
	5:BEBA ANG	6.660	946.727	-204.054	-494.378	0.525	-15.763
	6:BEBA ANG	-4.873	1.163	-0.002	-0.010	0.037	12.981
	7:KOM.1 (1.4 E)	267.920	15.8E 3	-375.034	-646.796	-1.377	-800.817
	8:KOM.2 (1.2 E)	238.794	24.4E 3	-497.113	-820.167	-0.599	-714.189
	9:KOM.3 (1.2 E)	246.019	21.8E 3	-757.729	-1.51E 3	0.023	-728.994
	10:KOM.4 (1.2)	227.566	20.3E 3	-431.245	-720.518	-0.759	-683.004
	11:KOM.5 (0.9)	182.891	11.6E 3	-567.580	-1.21E 3	-0.046	-540.032
	12:KOM.6 (0.9)	164.437	10.1E 3	-241.096	-415.813	-0.827	-494.042
	13:KOM.7 (1.2)	235.363	20.3E 3	-431.242	-720.503	-0.817	-703.774
	14:KOM.8 (1.2)	235.363	20.3E 3	-431.242	-720.503	-0.817	-703.774
	15:KOM.9 (1.2)	235.363	20.3E 3	-431.242	-720.503	-0.817	-703.774
	16:KOM.10 (1.2)	235.363	20.3E 3	-431.242	-720.503	-0.817	-703.774
	17:KOM.11 (0.9)	172.234	10.1E 3	-241.093	-415.797	-0.885	-514.811
	18:KOM.12 (0.9)	172.234	10.1E 3	-241.093	-415.797	-0.885	-514.811
	19:KOM.13 (0.9)	172.234	10.1E 3	-241.093	-415.797	-0.885	-514.811
	20:KOM.14 (0.9)	172.234	10.1E 3	-241.093	-415.797	-0.885	-514.811
53	1:BEBA MAT	278.251	10.8E 3	156.962	184.398	-2.164	-783.593
	2:BEBA HIDU	11.812	6.6E 3	106.788	153.858	-0.898	-30.571
	3:BEBA GEV	-754.919	-6.04E 3	-1.72E 3	-4.18E 3	10.086	1.97E 3
	4:BEBA GEV	754.919	6.04E 3	1.72E 3	4.18E 3	-10.086	-1.97E 3
	5:BEBA ANG	-8.818	-786.757	-200.934	-491.884	1.192	21.056
	6:BEBA ANG	-4.811	0.697	-0.032	-0.053	-0.004	12.843
	7:KOM.1 (1.4 E)	389.551	15.1E 3	219.747	258.157	-3.029	-1.1E 3
	8:KOM.2 (1.2 E)	352.800	23.5E 3	359.215	467.451	-4.033	-889.225
	9:KOM.3 (1.2 E)	331.604	18.3E 3	-26.351	-411.878	-1.588	-837.192
	10:KOM.4 (1.2)	338.016	19.6E 3	295.091	375.050	-3.501	-950.333
	11:KOM.5 (0.9)	236.317	8.47E 3	-180.228	-621.056	-0.040	-671.543
	12:KOM.6 (0.9)	242.729	9.73E 3	141.215	165.873	-1.954	-684.664
	13:KOM.7 (1.2)	345.713	19.6E 3	295.142	375.136	-3.494	-970.882
	14:KOM.8 (1.2)	345.713	19.6E 3	295.142	375.136	-3.494	-970.882
	15:KOM.9 (1.2)	345.713	19.6E 3	295.142	375.136	-3.494	-970.882
	16:KOM.10 (1.2)	345.713	19.6E 3	295.142	375.136	-3.494	-970.882
	17:KOM.11 (0.9)	250.426	9.72E 3	141.266	165.958	-1.947	-705.233
	18:KOM.12 (0.9)	250.426	9.72E 3	141.266	165.958	-1.947	-705.233
	19:KOM.13 (0.9)	250.426	9.72E 3	141.266	165.958	-1.947	-705.233



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Job No	Sheet No <b>12</b>	Rev
Part		
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By	Date 06-Oct-10	Chd
Client	File endhik-2 slp 4.std	Date/Time 21-Mar-2011 15:52

## Reactions Cont...

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	20:KOM.14 (0.1	250.426	9.72E 3	141.286	165.958	-1.947	-705.233
55	1:BEAN MAT	205.196	11.6E 3	-288.190	-498.976	5.469	-808.848
	2:BEAN HIDL	6.479	6.64E 3	-116.407	-176.991	0.734	-18.720
	3:BEAN GEV	-614.266	6.1E 3	-1.71E 3	-4.16E 3	9.679	1.64E 3
	4:BEAN GEV	614.266	-6.1E 3	1.71E 3	4.16E 3	-9.679	-1.64E 3
	5:BEAN ANG	8.931	1.03E 3	-200.345	-490.549	1.052	-20.803
	6:BEAN ANG	-4.871	0.826	0.026	0.037	-0.001	12.983
	7:KOM.1 (1.4 I	288.675	16.3E 3	-403.466	-698.566	7.656	-852.387
	8:KOM.2 (1.2 I	257.803	24.6E 3	-532.080	-881.957	7.737	-760.570
	9:KOM.3 (1.2 I	268.204	22.2E 3	-782.788	-1.56E 3	8.980	-782.622
	10:KOM.4 (1.2	246.121	20.6E 3	-462.195	-775.703	7.294	-728.566
	11:KOM.5 (0.9	199.866	12.1E 3	-579.924	-1.23E 3	6.605	-581.248
	12:KOM.6 (0.9	177.783	10.5E 3	-259.330	-449.018	4.919	-527.191
	13:KOM.7 (1.2	253.915	20.6E 3	-462.236	-775.763	7.297	-749.338
	14:KOM.8 (1.2	253.915	20.6E 3	-462.236	-775.763	7.297	-749.338
	15:KOM.9 (1.2	253.915	20.6E 3	-462.236	-775.763	7.297	-749.338
	16:KOM.10 (1.:	253.915	20.6E 3	-462.236	-775.763	7.297	-749.338
	17:KOM.11 (0.1	185.577	10.5E 3	-259.371	-449.078	4.922	-547.963
	18:KOM.12 (0.1	185.577	10.5E 3	-259.371	-449.078	4.922	-547.963
	19:KOM.13 (0.1	185.577	10.5E 3	-259.371	-449.078	4.922	-547.963
	20:KOM.14 (0.1	185.577	10.5E 3	-259.371	-449.078	4.922	-547.963
57	1:BEAN MAT	276.385	10.9E 3	154.343	179.203	3.776	-776.864
	2:BEAN HIDL	7.111	6.35E 3	101.306	147.216	-0.509	-23.015
	3:BEAN GEV	-755.304	-5.96E 3	-1.75E 3	-4.25E 3	10.552	1.97E 3
	4:BEAN GEV	755.304	5.96E 3	1.75E 3	4.25E 3	-10.552	-1.97E 3
	5:BEAN ANG	-9.291	-799.490	-205.040	-500.168	1.246	22.150
	6:BEAN ANG	-4.811	0.514	-0.010	-0.018	-0.003	12.842
	7:KOM.1 (1.4 I	386.939	15.3E 3	216.081	250.884	5.286	-1.09E 3
	8:KOM.2 (1.2 I	343.039	23.3E 3	347.302	450.589	3.717	-969.060
	9:KOM.3 (1.2 I	323.907	18.2E 3	-41.545	-438.010	6.017	-919.811
	10:KOM.4 (1.2	331.076	19.5E 3	286.503	362.231	4.018	-934.703
	11:KOM.5 (0.9	233.881	8.55E 3	-189.154	-638.987	5.393	-663.737
	12:KOM.6 (0.9	241.050	9.83E 3	138.894	161.254	3.394	-678.630
	13:KOM.7 (1.2	338.773	19.5E 3	286.518	362.259	4.022	-955.251
	14:KOM.8 (1.2	338.773	19.5E 3	286.518	362.259	4.022	-955.251
	15:KOM.9 (1.2	338.773	19.5E 3	286.518	362.259	4.022	-955.251
	16:KOM.10 (1.:	338.773	19.5E 3	286.518	362.259	4.022	-955.251
	17:KOM.11 (0.1	248.746	9.83E 3	138.909	161.283	3.398	-699.177
	18:KOM.12 (0.1	248.746	9.83E 3	138.909	161.283	3.398	-699.177
	19:KOM.13 (0.1	248.746	9.83E 3	138.909	161.283	3.398	-699.177
	20:KOM.14 (0.1	248.746	9.83E 3	138.909	161.283	3.398	-699.177
59	1:BEAN MAT	226.080	11.7E 3	-301.836	-531.983	8.623	-653.312
	2:BEAN HIDL	1.723	6.39E 3	-110.668	-169.813	0.546	-11.226
	3:BEAN GEV	-605.405	6.04E 3	-1.74E 3	-4.23E 3	11.067	1.62E 3
	4:BEAN GEV	605.405	-6.04E 3	1.74E 3	4.23E 3	-11.067	-1.62E 3
	5:BEAN ANG	10.548	1.06E 3	-204.304	-498.522	1.317	-24.461
	6:BEAN ANG	-4.875	0.573	0.018	0.034	-0.004	12.989
	7:KOM.1 (1.4 I	316.512	16.4E 3	-422.711	-744.776	12.072	-914.636
	8:KOM.2 (1.2 I	274.053	24.3E 3	-539.392	-910.081	11.221	-801.935



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Rev

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Part

Ref

By

Date 06-Oct-10

Chd

Client

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Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	LC	Horizontal		Vertical	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	9:KOM.3 (1.2 I	289.896	22.2E 3	-799.878	-1.81E 3	13.001	-834.337
	10:KOM.4 (1.2	265.220	20.5E 3	-472.963	-808.138	10.887	-774.417
	11:KOM.5 (0.9	220.348	12.3E 3	-598.629	-1.28E 3	9.867	-627.118
	12:KOM.6 (0.9	195.673	10.6E 3	-271.715	-478.730	7.754	-567.197
	13:KOM.7 (1.2	273.019	20.5E 3	-472.991	-808.193	10.893	-795.200
	14:KOM.8 (1.2	273.019	20.5E 3	-472.991	-808.193	10.893	-795.200
	15:KOM.9 (1.2	273.019	20.5E 3	-472.991	-808.193	10.893	-795.200
	16:KOM.10 (1.:	273.019	20.5E 3	-472.991	-808.193	10.893	-795.200
	17:KOM.11 (0.!	203.472	10.6E 3	-271.743	-478.785	7.760	-587.980
	18:KOM.12 (0.!	203.472	10.6E 3	-271.743	-478.785	7.760	-587.980
	19:KOM.13 (0.!	203.472	10.6E 3	-271.743	-478.785	7.760	-587.980
	20:KOM.14 (0.!	203.472	10.6E 3	-271.743	-478.785	7.760	-587.980
61	1:BEAN MAT	266.684	11.2E 3	140.522	145.175	11.225	-753.461
	2:BEAN HIDL	9.024	5.77E 3	85.209	124.144	0.099	-24.235
	3:BEAN GEV	-752.683	-5.6E 3	-1.78E 3	-4.3E 3	8.393	1.97E 3
	4:BEAN GEV	752.683	5.6E 3	1.78E 3	4.3E 3	-8.393	-1.97E 3
	5:BEAN ANG	-9.825	-810.754	-208.077	-506.974	1.287	22.910
	6:BEAN ANG	-4.807	-0.168	-0.000	0.001	-0.003	12.835
	7:KOM.1 (1.4 I	373.358	15.7E 3	196.731	203.245	15.715	-1.05E 3
	8:KOM.2 (1.2 I	334.460	22.7E 3	304.960	372.841	13.629	-942.929
	9:KOM.3 (1.2 I	313.646	18E 3	-79.088	-512.804	15.828	-891.732
	10:KOM.4 (1.2	321.353	19.3E 3	253.835	298.355	13.564	-807.862
	11:KOM.5 (0.9	224.816	8.81E 3	-206.453	-680.501	12.162	-641.459
	12:KOM.6 (0.9	232.324	10.1E 3	126.470	130.660	10.097	-657.579
	13:KOM.7 (1.2	329.045	19.3E 3	253.835	298.355	13.569	-928.388
	14:KOM.8 (1.2	329.045	19.3E 3	253.835	298.355	13.569	-928.388
	15:KOM.9 (1.2	329.045	19.3E 3	253.835	298.355	13.569	-928.388
	16:KOM.10 (1.:	329.045	19.3E 3	253.835	298.355	13.569	-928.388
	17:KOM.11 (0.!	240.016	10.1E 3	126.470	130.658	10.103	-678.115
	18:KOM.12 (0.!	240.016	10.1E 3	126.470	130.658	10.103	-678.115
	19:KOM.13 (0.!	240.016	10.1E 3	126.470	130.658	10.103	-678.115
	20:KOM.14 (0.!	240.016	10.1E 3	126.470	130.658	10.103	-678.115
63	1:BEAN MAT	245.848	12E 3	-325.129	-584.556	12.261	-698.771
	2:BEAN HIDL	4.117	5.82E 3	-94.790	-147.352	0.237	-13.640
	3:BEAN GEV	-602.286	5.6E 3	-1.77E 3	-4.29E 3	9.515	1.62E 3
	4:BEAN GEV	602.286	-5.6E 3	1.77E 3	4.29E 3	-9.515	-1.62E 3
	5:BEAN ANG	11.496	1.08E 3	-207.605	-505.927	1.459	-26.674
	6:BEAN ANG	-4.875	-0.201	0.024	0.053	-0.005	12.990
	7:KOM.1 (1.4 I	344.187	16.8E 3	-455.181	-818.378	17.166	-978.279
	8:KOM.2 (1.2 I	301.605	23.7E 3	-541.819	-937.230	15.093	-860.349
	9:KOM.3 (1.2 I	317.528	22E 3	-817.113	-1.88E 3	17.285	-894.844
	10:KOM.4 (1.2	291.335	20.2E 3	-484.906	-848.734	14.944	-831.381
	11:KOM.5 (0.9	239.657	12.5E 3	-624.784	-1.34E 3	13.370	-671.573
	12:KOM.6 (0.9	213.464	10.8E 3	-292.578	-526.015	11.028	-608.110
	13:KOM.7 (1.2	299.135	20.2E 3	-484.945	-848.819	14.951	-852.165
	14:KOM.8 (1.2	299.135	20.2E 3	-484.945	-848.819	14.951	-852.165
	15:KOM.9 (1.2	299.135	20.2E 3	-484.945	-848.819	14.951	-852.165
	16:KOM.10 (1.:	299.135	20.2E 3	-484.945	-848.819	14.951	-852.165
	17:KOM.11 (0.!	221.263	10.8E 3	-292.616	-526.100	11.035	-628.894





Software licensed to Snow Panther [LZO]

Job No

Sheet No

14

Rev

Part

Job Title

Ref

By

Date 06-Oct-10

Chd

Client

File endhik-2 sip 4.std

Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal			Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	18:KOM.12 (0.!	221.263	10.8E 3	-292.616	-526.100	11.035	-628.894
	19:KOM.13 (0.!	221.263	10.8E 3	-292.616	-526.100	11.035	-628.894
	20:KOM.14 (0.!	221.263	10.8E 3	-292.616	-526.100	11.035	-628.894
65	1:BEBA MAT	255.025	11.2E 3	108.984	70.548	19.819	-723.027
	2:BEBA HIDL	11.597	6.38E 3	101.019	148.406	0.788	-26.102
	3:BEBA GEN	-748.143	-6.04E 3	-1.79E 3	-4.33E 3	5.737	1.95E 3
	4:BEBA GEN	748.143	6.04E 3	1.79E 3	4.33E 3	-5.737	-1.95E 3
	5:BEBA ANG	-9.758	-813.000	-211.082	-513.768	1.293	23.258
	6:BEBA ANG	-4.800	0.124	0.011	0.025	-0.005	12.820
	7:KOM.1 (1.4 Γ	357.035	15.7E 3	152.577	98.767	27.746	-1.01E 3
	8:KOM.2 (1.2 Γ	324.585	23.6E 3	292.410	318.907	25.041	-809.396
	9:KOM.3 (1.2 Γ	302.015	18.5E 3	-105.932	-590.965	26.637	-856.521
	10:KOM.4 (1.2	309.948	19.8E 3	231.817	231.103	24.561	-873.223
	11:KOM.5 (0.9	213.910	8.77E 3	-239.645	-758.535	19.905	-613.511
	12:KOM.6 (0.9	221.843	10.1E 3	98.103	63.533	17.829	-630.213
	13:KOM.7 (1.2	317.627	19.8E 3	231.799	231.063	24.569	-893.735
	14:KOM.8 (1.2	317.627	19.8E 3	231.799	231.063	24.569	-893.735
	15:KOM.9 (1.2	317.627	19.8E 3	231.799	231.063	24.569	-893.735
	16:KOM.10 (1.:	317.627	19.8E 3	231.799	231.063	24.569	-893.735
	17:KOM.11 (0.!	229.523	10.1E 3	98.085	63.493	17.837	-650.724
	18:KOM.12 (0.!	229.523	10.1E 3	98.085	63.493	17.837	-650.724
	19:KOM.13 (0.!	229.523	10.1E 3	98.085	63.493	17.837	-650.724
	20:KOM.14 (0.!	229.523	10.1E 3	98.085	63.493	17.837	-650.724
67	1:BEBA MAT	269.969	11.9E 3	-355.693	-654.790	16.573	-752.404
	2:BEBA HIDL	7.514	6.42E 3	-111.315	-171.579	-0.053	-17.548
	3:BEBA GEN	-602.780	6.07E 3	-1.78E 3	-4.33E 3	7.336	1.62E 3
	4:BEBA GEN	602.780	-6.07E 3	1.79E 3	4.33E 3	-7.336	-1.62E 3
	5:BEBA ANG	11.923	1.07E 3	-211.027	-513.663	1.484	-27.677
	6:BEBA ANG	-4.870	0.058	0.031	0.072	-0.004	12.982
	7:KOM.1 (1.4 Γ	377.956	16.7E 3	-497.970	-916.706	23.202	-1.05E 3
	8:KOM.2 (1.2 Γ	335.985	24.6E 3	-604.935	-1.06E 3	19.803	-930.961
	9:KOM.3 (1.2 Γ	350.553	22.4E 3	-875.790	-1.78E 3	22.210	-964.716
	10:KOM.4 (1.2	323.684	20.7E 3	-538.096	-957.213	19.828	-899.661
	11:KOM.5 (0.9	262.049	12.4E 3	-657.767	-1.41E 3	17.291	-721.447
	12:KOM.6 (0.9	235.180	10.7E 3	-320.073	-589.196	14.909	-656.392
	13:KOM.7 (1.2	331.476	20.7E 3	-538.146	-957.327	19.835	-920.433
	14:KOM.8 (1.2	331.476	20.7E 3	-538.146	-957.327	19.835	-920.433
	15:KOM.9 (1.2	331.476	20.7E 3	-538.146	-957.327	19.835	-920.433
	16:KOM.10 (1.:	331.476	20.7E 3	-538.146	-957.327	19.835	-920.433
	17:KOM.11 (0.!	242.972	10.7E 3	-320.123	-589.311	14.916	-677.163
	18:KOM.12 (0.!	242.972	10.7E 3	-320.123	-589.311	14.916	-677.163
	19:KOM.13 (0.!	242.972	10.7E 3	-320.123	-589.311	14.916	-677.163
	20:KOM.14 (0.!	242.972	10.7E 3	-320.123	-589.311	14.916	-677.163
69	1:BEBA MAT	224.160	11.2E 3	58.331	-48.394	28.702	-661.746
	2:BEBA HIDL	-1.365	6.45E 3	96.535	137.811	1.416	-5.667
	3:BEBA GEN	-742.284	-6.17E 3	-1.78E 3	-4.35E 3	5.143	1.94E 3
	4:BEBA GEN	742.284	6.17E 3	1.79E 3	4.35E 3	-5.143	-1.94E 3
	5:BEBA ANG	-9.367	-786.038	-212.660	-518.458	1.272	22.709
	6:BEBA ANG	-4.797	-0.134	0.032	0.066	-0.007	12.809



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Job No	Sheet No <b>15</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

### Reactions Cont...

Node	L/C	Horizontal		Vertical	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	7:KOM.1 (1.4 [	313.825	15.8E 3	81.663	-67.752	40.183	-926.444
	8:KOM.2 (1.2 [	266.809	23.7E 3	224.453	162.424	36.707	-803.162
	9:KOM.3 (1.2 [	252.640	18.6E 3	-173.725	-749.794	37.893	-763.427
	10:KOM.4 (1.2	259.953	19.8E 3	166.584	79.844	35.847	-779.267
	11:KOM.5 (0.9	186.757	8.79E 3	-287.759	-873.087	27.867	-559.238
	12:KOM.6 (0.9	184.069	10E 3	52.550	-43.449	25.821	-575.076
	13:KOM.7 (1.2	267.628	19.8E 3	166.532	79.738	35.858	-799.762
	14:KOM.8 (1.2	267.628	19.8E 3	166.532	79.738	35.858	-799.762
	15:KOM.9 (1.2	267.628	19.8E 3	166.532	79.738	35.858	-799.762
	16:KOM.10 (1.:	267.628	19.8E 3	166.532	79.738	35.858	-799.762
	17:KOM.11 (0.!	201.744	10E 3	52.498	-43.555	25.832	-595.571
	18:KOM.12 (0.!	201.744	10E 3	52.498	-43.555	25.832	-595.571
	19:KOM.13 (0.!	201.744	10E 3	52.498	-43.555	25.832	-595.571
	20:KOM.14 (0.!	201.744	10E 3	52.498	-43.555	25.832	-595.571
71	1:BEAN MAT	279.621	11.7E 3	-392.562	-742.393	21.173	-787.493
	2:BEAN HIDL	-3.951	6.48E 3	-108.409	-167.023	-0.219	-0.484
	3:BEAN GEV	-608.926	6.17E 3	-1.8E 3	-4.35E 3	6.444	1.63E 3
	4:BEAN GEV	608.926	-6.17E 3	1.8E 3	4.35E 3	-6.444	-1.63E 3
	5:BEAN ANG	11.434	1.05E 3	-212.941	-519.113	1.372	-26.882
	6:BEAN ANG	-4.870	-0.243	0.029	0.076	-0.003	12.979
	7:KOM.1 (1.4 [	391.489	16.4E 3	-549.586	-1.04E 3	29.643	-1.1E 3
	8:KOM.2 (1.2 [	329.223	24.4E 3	-644.528	-1.16E 3	25.058	-945.757
	9:KOM.3 (1.2 [	349.887	22.2E 3	-920.188	-1.89E 3	27.384	-988.487
	10:KOM.4 (1.2	323.801	20.6E 3	-579.435	-1.06E 3	25.184	-924.710
	11:KOM.5 (0.9	269.953	12.2E 3	-694.010	-1.5E 3	21.251	-751.755
	12:KOM.6 (0.9	243.866	10.6E 3	-353.258	-668.032	19.051	-687.978
	13:KOM.7 (1.2	331.593	20.6E 3	-579.483	-1.06E 3	25.189	-945.476
	14:KOM.8 (1.2	331.593	20.6E 3	-579.483	-1.06E 3	25.189	-945.476
	15:KOM.9 (1.2	331.593	20.6E 3	-579.483	-1.06E 3	25.189	-945.476
	16:KOM.10 (1.:	331.593	20.6E 3	-579.483	-1.06E 3	25.189	-945.476
	17:KOM.11 (0.!	251.659	10.6E 3	-353.305	-668.154	19.056	-708.744
	18:KOM.12 (0.!	251.659	10.6E 3	-353.305	-668.154	19.056	-708.744
	19:KOM.13 (0.!	251.659	10.6E 3	-353.305	-668.154	19.056	-708.744
	20:KOM.14 (0.!	251.659	10.6E 3	-353.305	-668.154	19.056	-708.744
73	1:BEAN MAT	-253.918	86.8E 3	1.86E 3	2.36E 3	-52.681	156.160
	2:BEAN HIDL	54.781	8.99E 3	80.689	106.065	-1.865	-87.229
	3:BEAN GEV	-743.800	-3.16E 3	-2.11E 3	-4.72E 3	3.578	1.92E 3
	4:BEAN GEV	743.800	3.16E 3	2.11E 3	4.72E 3	-3.578	-1.92E 3
	5:BEAN ANG	-12.665	-161.811	-242.275	-551.485	0.263	21.099
	6:BEAN ANG	-4.433	-0.088	-0.625	-0.985	-0.010	12.328
	7:KOM.1 (1.4 [	-355.485	121E 3	2.8E 3	3.31E 3	-73.753	218.623
	8:KOM.2 (1.2 [	-217.051	119E 3	2.36E 3	3E 3	-66.201	47.824
	9:KOM.3 (1.2 [	-270.185	113E 3	1.92E 3	2.06E 3	-64.661	133.921
	10:KOM.4 (1.2	-257.013	113E 3	2.31E 3	2.94E 3	-65.098	119.887
	11:KOM.5 (0.9	-248.791	77.8E 3	1.28E 3	1.24E 3	-46.992	174.302
	12:KOM.6 (0.9	-235.619	78.1E 3	1.67E 3	2.12E 3	-47.429	160.269
	13:KOM.7 (1.2	-249.920	113E 3	2.31E 3	2.94E 3	-65.082	100.162
	14:KOM.8 (1.2	-249.920	113E 3	2.31E 3	2.94E 3	-65.082	100.162
	15:KOM.9 (1.2	-249.920	113E 3	2.31E 3	2.94E 3	-65.082	100.162



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Job No	Sheet No <b>16</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal			Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	16:KOM.10 (1.:	-249.920	113E 3	2.31E 3	2.94E 3	-65.082	100.162
	17:KOM.11 (0.:	-228.526	78.1E 3	1.67E 3	2.12E 3	-47.413	140.544
	18:KOM.12 (0.:	-228.526	78.1E 3	1.67E 3	2.12E 3	-47.413	140.544
	19:KOM.13 (0.:	-228.526	78.1E 3	1.67E 3	2.12E 3	-47.413	140.544
	20:KOM.14 (0.:	-228.526	78.1E 3	1.67E 3	2.12E 3	-47.413	140.544
75	1:BEBAN MAT	680.761	84.5E 3	1.81E 3	2.35E 3	47.116	-1.5E 3
	2:BEBAN HIDL	-39.517	8.92E 3	80.371	107.536	1.732	42.741
	3:BEBAN GEV	-568.396	-350.054	-2.07E 3	-4.78E 3	3.444	1.66E 3
	4:BEBAN GEV	568.396	350.054	2.07E 3	4.78E 3	-3.444	-1.68E 3
	5:BEBAN ANG	8.060	-215.623	-259.943	-591.920	0.880	-8.170
	6:BEBAN ANG	-4.387	1.870	0.675	1.039	-0.010	12.255
	7:KOM.1 (1.4 [	953.066	118E 3	2.53E 3	3.29E 3	65.962	-2.1E 3
	8:KOM.2 (1.2 [	753.686	116E 3	2.3E 3	2.99E 3	59.310	-1.73E 3
	9:KOM.3 (1.2 [	790.292	110E 3	1.83E 3	1.98E 3	59.678	-1.77E 3
	10:KOM.4 (1.2	770.377	110E 3	2.25E 3	2.93E 3	58.255	-1.74E 3
	11:KOM.5 (0.9	625.591	75.7E 3	1.21E 3	1.17E 3	43.812	-1.36E 3
	12:KOM.6 (0.9	605.666	76E 3	1.63E 3	2.11E 3	42.388	-1.33E 3
	13:KOM.7 (1.2	777.396	110E 3	2.25E 3	2.92E 3	58.271	-1.76E 3
	14:KOM.8 (1.2	777.396	110E 3	2.25E 3	2.92E 3	58.271	-1.76E 3
	15:KOM.9 (1.2	777.396	110E 3	2.25E 3	2.92E 3	58.271	-1.76E 3
	16:KOM.10 (1.:	777.396	110E 3	2.25E 3	2.92E 3	58.271	-1.76E 3
	17:KOM.11 (0.:	612.685	76E 3	1.63E 3	2.11E 3	42.404	-1.35E 3
	18:KOM.12 (0.:	612.685	76E 3	1.63E 3	2.11E 3	42.404	-1.35E 3
	19:KOM.13 (0.:	612.685	76E 3	1.63E 3	2.11E 3	42.404	-1.35E 3
	20:KOM.14 (0.:	612.685	76E 3	1.63E 3	2.11E 3	42.404	-1.35E 3
77	1:BEBAN MAT	-94.984	86.3E 3	-2.58E 3	-4.11E 3	-50.147	-218.002
	2:BEBAN HIDL	58.492	8.92E 3	-101.037	-159.702	-1.410	-98.003
	3:BEBAN GEV	-589.650	890.005	-1.96E 3	-4.51E 3	6.481	1.64E 3
	4:BEBAN GEV	589.650	-890.005	1.96E 3	4.51E 3	-6.481	-1.64E 3
	5:BEBAN ANG	7.842	387.749	-243.548	-553.102	0.478	-11.340
	6:BEBAN ANG	-4.463	-1.115	0.470	0.625	-0.000	12.385
	7:KOM.1 (1.4 [	-132.977	121E 3	-3.61E 3	-5.75E 3	-70.205	-305.203
	8:KOM.2 (1.2 [	-20.394	118E 3	-3.25E 3	-5.19E 3	-62.432	-418.408
	9:KOM.3 (1.2 [	-42.941	113E 3	-3.58E 3	-5.98E 3	-60.821	-377.749
	10:KOM.4 (1.2	-62.630	112E 3	-3.19E 3	-5.09E 3	-61.586	-339.789
	11:KOM.5 (0.9	-72.938	78.3E 3	-2.71E 3	-4.58E 3	-44.367	-214.346
	12:KOM.6 (0.9	-92.627	77.7E 3	-2.32E 3	-3.7E 3	-45.132	-176.386
	13:KOM.7 (1.2	-55.489	112E 3	-3.19E 3	-5.09E 3	-61.586	-359.606
	14:KOM.8 (1.2	-55.489	112E 3	-3.19E 3	-5.09E 3	-61.586	-359.606
	15:KOM.9 (1.2	-55.489	112E 3	-3.19E 3	-5.09E 3	-61.586	-359.606
	16:KOM.10 (1.:	-55.489	112E 3	-3.19E 3	-5.09E 3	-61.586	-359.606
	17:KOM.11 (0.:	-85.485	77.7E 3	-2.32E 3	-3.7E 3	-45.132	-196.202
	18:KOM.12 (0.:	-85.485	77.7E 3	-2.32E 3	-3.7E 3	-45.132	-196.202
	19:KOM.13 (0.:	-85.485	77.7E 3	-2.32E 3	-3.7E 3	-45.132	-196.202
	20:KOM.14 (0.:	-85.485	77.7E 3	-2.32E 3	-3.7E 3	-45.132	-196.202
79	1:BEBAN MAT	-3.56E 3	92E 3	118.893	-49.748	6.839	5.1E 3
	2:BEBAN HIDL	-183.129	7.59E 3	1.340	-5.722	0.823	262.818
	3:BEBAN GEV	-216.883	-756.166	-1.8E 3	-4.09E 3	-11.552	911.191
	4:BEBAN GEV	216.883	756.166	1.8E 3	4.09E 3	11.552	-911.191



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Job No	Sheet No <b>17</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	5:BEAN ANG	33.527	-215.737	-214.242	-488.642	-1.231	-46.101
	6:BEAN ANG	-3.445	28.278	0.041	0.012	-0.014	9.174
	7:KOM.1 (1.4 I	-4.99E 3	129E 3	166.451	-69.647	9.575	7.14E 3
	8:KOM.2 (1.2 I	-4.57E 3	123E 3	144.817	-68.852	9.524	6.54E 3
	9:KOM.3 (1.2 I	-4.4E 3	118E 3	-198.774	-847.247	7.060	6.31E 3
	10:KOM.4 (1.2	-4.46E 3	118E 3	144.078	-65.399	9.008	6.39E 3
	11:KOM.5 (0.9	-3.15E 3	82.5E 3	-235.783	-826.600	4.185	4.51E 3
	12:KOM.6 (0.9	-3.21E 3	82.9E 3	107.070	-44.753	6.133	4.6E 3
	13:KOM.7 (1.2	-4.46E 3	118E 3	144.012	-65.419	9.030	6.38E 3
	14:KOM.8 (1.2	-4.46E 3	118E 3	144.012	-65.419	9.030	6.38E 3
	15:KOM.9 (1.2	-4.46E 3	118E 3	144.012	-65.419	9.030	6.38E 3
	16:KOM.10 (1.	-4.46E 3	118E 3	144.012	-65.419	9.030	6.38E 3
	17:KOM.11 (0.!	-3.21E 3	82.8E 3	107.004	-44.773	6.155	4.59E 3
	18:KOM.12 (0.!	-3.21E 3	82.8E 3	107.004	-44.773	6.155	4.59E 3
	19:KOM.13 (0.!	-3.21E 3	82.8E 3	107.004	-44.773	6.155	4.59E 3
	20:KOM.14 (0.!	-3.21E 3	82.8E 3	107.004	-44.773	6.155	4.59E 3
81	1:BEAN MAT	33.939	-2.26E 3	-37.760	-210.513	-2.748	-269.532
	2:BEAN HIDL	2.154	-49.446	-3.124	-15.997	-0.250	-12.955
	3:BEAN GEV	-52.709	-461.912	-120.257	-879.928	6.855	395.177
	4:BEAN GEV	52.709	461.912	120.257	879.928	-6.855	-395.177
	5:BEAN ANG	0.447	-1.897	-18.779	-136.281	1.151	-1.652
	6:BEAN ANG	-0.659	-23.147	-0.112	-0.541	0.178	4.470
	7:KOM.1 (1.4 I	47.515	-3.16E 3	-52.865	-294.718	-3.847	-377.345
	8:KOM.2 (1.2 I	44.173	-2.79E 3	-50.310	-278.211	-3.697	-344.166
	9:KOM.3 (1.2 I	43.595	-2.76E 3	-78.483	-486.661	-1.706	-339.037
	10:KOM.4 (1.2	41.826	-2.8E 3	-48.616	-269.477	-3.262	-329.241
	11:KOM.5 (0.9	31.260	-2.04E 3	-64.031	-407.510	-0.632	-245.222
	12:KOM.6 (0.9	29.491	-2.07E 3	-34.164	-190.326	-2.188	-235.426
	13:KOM.7 (1.2	42.881	-2.76E 3	-48.436	-268.612	-3.547	-336.393
	14:KOM.8 (1.2	42.881	-2.76E 3	-48.436	-268.612	-3.547	-336.393
	15:KOM.9 (1.2	42.881	-2.76E 3	-48.436	-268.612	-3.547	-336.393
	16:KOM.10 (1.	42.881	-2.76E 3	-48.436	-268.612	-3.547	-336.393
	17:KOM.11 (0.!	30.545	-2.03E 3	-33.984	-189.461	-2.473	-242.579
	18:KOM.12 (0.!	30.545	-2.03E 3	-33.984	-189.461	-2.473	-242.579
	19:KOM.13 (0.!	30.545	-2.03E 3	-33.984	-189.461	-2.473	-242.579
	20:KOM.14 (0.!	30.545	-2.03E 3	-33.984	-189.461	-2.473	-242.579
83	1:BEAN MAT	-3.2E 3	98.5E 3	-678.518	-1.22E 3	25.752	4.34E 3
	2:BEAN HIDL	-157.471	7.57E 3	-21.512	-39.700	0.065	217.896
	3:BEAN GEV	-765.728	2.82E 3	-1.82E 3	-4.12E 3	24.836	1.69E 3
	4:BEAN GEV	765.728	-2.82E 3	1.82E 3	4.12E 3	-24.836	-1.69E 3
	5:BEAN ANG	-36.901	273.367	-214.744	-490.085	3.123	54.756
	6:BEAN ANG	-3.355	27.954	-0.168	-0.296	0.009	9.033
	7:KOM.1 (1.4 I	-4.47E 3	138E 3	-949.925	-1.71E 3	36.053	6.07E 3
	8:KOM.2 (1.2 I	-4.09E 3	130E 3	-848.640	-1.53E 3	31.006	5.55E 3
	9:KOM.3 (1.2 I	-4.05E 3	126E 3	-1.18E 3	-2.29E 3	35.963	5.51E 3
	10:KOM.4 (1.2	-4E 3	126E 3	-836.001	-1.5E 3	30.981	5.44E 3
	11:KOM.5 (0.9	-2.93E 3	89.1E 3	-954.256	-1.88E 3	28.173	3.99E 3
	12:KOM.6 (0.9	-2.88E 3	88.7E 3	-610.934	-1.1E 3	23.191	3.92E 3
	13:KOM.7 (1.2	-3.99E 3	126E 3	-835.733	-1.5E 3	30.967	5.42E 3



Software licensed to Snow Panther [L20]

Job No	Sheet No <b>18</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal		Horizontal FZ (kg)	Moment		
		FX (kg)	FY (kg)		MX (kg·m)	MY (kg·m)	MZ (kg·m)
	14:KOM.8 (1.2	-3.99E 3	126E 3	-835.733	-1.5E 3	30.967	5.42E 3
	15:KOM.9 (1.2	-3.99E 3	126E 3	-835.733	-1.5E 3	30.967	5.42E 3
	16:KOM.10 (1.	-3.99E 3	126E 3	-835.733	-1.5E 3	30.967	5.42E 3
	17:KOM.11 (0.!	-2.88E 3	88.7E 3	-610.666	-1.1E 3	23.177	3.9E 3
	18:KOM.12 (0.!	-2.88E 3	88.7E 3	-610.666	-1.1E 3	23.177	3.9E 3
	19:KOM.13 (0.!	-2.88E 3	88.7E 3	-610.666	-1.1E 3	23.177	3.9E 3
	20:KOM.14 (0.!	-2.88E 3	88.7E 3	-610.666	-1.1E 3	23.177	3.9E 3
85	1:BEAN MAT	32.239	-2.54E 3	7.785	1.832	-0.461	-262.346
	2:BEAN HIDL	2.097	-48.382	2.027	8.012	0.078	-12.591
	3:BEAN GEM	-57.380	-197.188	-122.803	-891.745	4.519	414.981
	4:BEAN GEM	57.380	197.188	122.803	891.745	-4.519	-414.981
	5:BEAN ANG	-0.682	-12.886	-18.775	-136.261	1.140	3.205
	6:BEAN ANG	-0.660	-23.107	0.104	0.479	-0.183	4.478
	7:KOM.1 (1.4 [	45.135	-3.56E 3	10.900	2.565	-0.645	-367.285
	8:KOM.2 (1.2 [	42.042	-3.13E 3	12.585	15.019	-0.428	-334.961
	9:KOM.3 (1.2 [	39.692	-3.12E 3	-18.670	-207.806	1.350	-322.278
	10:KOM.4 (1.2	39.727	-3.13E 3	11.536	10.977	-0.767	-320.242
	11:KOM.5 (0.9	27.924	-2.31E 3	-23.033	-216.368	1.410	-230.984
	12:KOM.6 (0.9	27.959	-2.32E 3	7.173	2.415	-0.707	-228.947
	13:KOM.7 (1.2	40.784	-3.1E 3	11.369	10.211	-0.475	-327.406
	14:KOM.8 (1.2	40.784	-3.1E 3	11.369	10.211	-0.475	-327.406
	15:KOM.9 (1.2	40.784	-3.1E 3	11.369	10.211	-0.475	-327.406
	16:KOM.10 (1.	40.784	-3.1E 3	11.369	10.211	-0.475	-327.406
	17:KOM.11 (0.!	29.015	-2.29E 3	7.007	1.649	-0.415	-236.112
	18:KOM.12 (0.!	29.015	-2.29E 3	7.007	1.649	-0.415	-236.112
	19:KOM.13 (0.!	29.015	-2.29E 3	7.007	1.649	-0.415	-236.112
	20:KOM.14 (0.!	29.015	-2.29E 3	7.007	1.649	-0.415	-236.112
87	1:BEAN MAT	-182.730	3.1E 3	-79.627	-462.878	-37.130	42.627
	2:BEAN HIDL	9.170	1.13E 3	0.249	-11.403	-1.428	-21.746
	3:BEAN GEM	-404.305	-4.31E 3	-1.26E 3	-3.31E 3	5.284	1.18E 3
	4:BEAN GEM	404.305	4.31E 3	1.26E 3	3.31E 3	-5.284	-1.18E 3
	5:BEAN ANG	-1.508	-397.190	-142.443	-383.529	0.598	4.650
	6:BEAN ANG	-2.664	-19.094	-0.406	-0.667	-0.016	8.000
	7:KOM.1 (1.4 [	-255.822	4.34E 3	-111.477	-648.030	-51.982	59.678
	8:KOM.2 (1.2 [	-204.605	5.53E 3	-95.153	-573.700	-46.837	16.359
	9:KOM.3 (1.2 [	-212.520	4.22E 3	-323.212	-1.18E 3	-45.025	36.846
	10:KOM.4 (1.2	-214.368	4.82E 3	-95.953	-567.925	-46.006	42.208
	11:KOM.5 (0.9	-166.870	2.15E 3	-299.574	-1.03E 3	-32.460	45.803
	12:KOM.6 (0.9	-168.719	2.76E 3	-72.314	-417.658	-33.442	51.164
	13:KOM.7 (1.2	-210.106	4.85E 3	-95.303	-566.858	-45.981	29.406
	14:KOM.8 (1.2	-210.106	4.85E 3	-95.303	-566.858	-45.981	29.406
	15:KOM.9 (1.2	-210.106	4.85E 3	-95.303	-566.858	-45.981	29.406
	16:KOM.10 (1.	-210.106	4.85E 3	-95.303	-566.858	-45.981	29.406
	17:KOM.11 (0.!	-164.457	2.79E 3	-71.664	-416.591	-33.417	38.364
	18:KOM.12 (0.!	-164.457	2.79E 3	-71.664	-416.591	-33.417	38.364
	19:KOM.13 (0.!	-164.457	2.79E 3	-71.664	-416.591	-33.417	38.364
	20:KOM.14 (0.!	-164.457	2.79E 3	-71.664	-416.591	-33.417	38.364
89	1:BEAN MAT	-205.214	3.06E 3	-586.276	-1.21E 3	-42.870	123.110
	2:BEAN HIDL	10.486	1.14E 3	-23.138	-45.947	-1.083	-22.691



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Job No	Sheet No <b>19</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	3:BEAN GEV	-380.842	491.699	-1.17E 3	-3.16E 3	10.099	1.11E 3
	4:BEAN GEV	380.842	-491.699	1.17E 3	3.16E 3	-10.099	-1.11E 3
	5:BEAN ANG	0.059	409.022	-142.770	-383.619	0.778	-0.951
	6:BEAN ANG	-2.689	-19.011	0.286	0.366	0.009	8.059
	7:KOM.1 (1.4 I	-287.300	4.28E 3	-820.786	-1.69E 3	-60.018	172.354
	8:KOM.2 (1.2 I	-229.479	5.49E 3	-740.552	-1.53E 3	-53.177	111.426
	9:KOM.3 (1.2 I	-235.677	5.46E 3	-955.101	-2.11E 3	-51.283	123.518
	10:KOM.4 (1.2	-240.073	4.78E 3	-726.211	-1.5E 3	-52.513	137.936
	11:KOM.5 (0.9	-184.599	3.41E 3	-756.080	-1.7E 3	-37.339	109.277
	12:KOM.6 (0.9	-188.995	2.72E 3	-527.190	-1.09E 3	-38.565	123.694
	13:KOM.7 (1.2	-235.771	4.81E 3	-726.669	-1.5E 3	-52.527	125.040
	14:KOM.8 (1.2	-235.771	4.81E 3	-726.669	-1.5E 3	-52.527	125.040
	15:KOM.9 (1.2	-235.771	4.81E 3	-726.669	-1.5E 3	-52.527	125.040
	16:KOM.10 (1.	-235.771	4.81E 3	-726.669	-1.5E 3	-52.527	125.040
	17:KOM.11 (0.!	-184.693	2.75E 3	-527.648	-1.09E 3	-38.583	110.799
	18:KOM.12 (0.!	-184.693	2.75E 3	-527.648	-1.09E 3	-38.583	110.799
	19:KOM.13 (0.!	-184.693	2.75E 3	-527.648	-1.09E 3	-38.583	110.799
	20:KOM.14 (0.!	-184.693	2.75E 3	-527.648	-1.09E 3	-38.583	110.799
91	1:BEAN MAT	46.475	-2.77E 3	-30.953	-169.801	-1.496	-320.877
	2:BEAN HIDL	0.022	-65.452	-2.856	-14.400	-0.025	-3.029
	3:BEAN GEV	-58.609	161.230	-130.389	-946.722	0.658	423.608
	4:BEAN GEV	58.609	-161.230	130.389	946.722	-0.658	-423.608
	5:BEAN ANG	-0.922	3.832	-19.957	-144.789	-0.094	4.977
	6:BEAN ANG	-0.878	41.124	0.292	1.373	0.181	5.492
	7:KOM.1 (1.4 I	65.065	-3.88E 3	-43.334	-237.722	-2.094	-449.227
	8:KOM.2 (1.2 I	55.805	-3.43E 3	-41.713	-226.801	-1.835	-389.898
	9:KOM.3 (1.2 I	54.317	-3.38E 3	-71.930	-449.823	-1.970	-380.117
	10:KOM.4 (1.2	54.387	-3.32E 3	-39.532	-215.965	-1.531	-379.294
	11:KOM.5 (0.9	40.353	-2.49E 3	-59.788	-384.483	-1.496	-280.826
	12:KOM.6 (0.9	40.422	-2.43E 3	-27.391	-150.624	-1.058	-280.002
	13:KOM.7 (1.2	55.792	-3.39E 3	-39.999	-218.161	-1.820	-388.080
	14:KOM.8 (1.2	55.792	-3.39E 3	-39.999	-218.161	-1.820	-388.080
	15:KOM.9 (1.2	55.792	-3.39E 3	-39.999	-218.161	-1.820	-388.080
	16:KOM.10 (1.	55.792	-3.39E 3	-39.999	-218.161	-1.820	-388.080
	17:KOM.11 (0.!	41.827	-2.49E 3	-27.857	-152.821	-1.346	-288.789
	18:KOM.12 (0.!	41.827	-2.49E 3	-27.857	-152.821	-1.346	-288.789
	19:KOM.13 (0.!	41.827	-2.49E 3	-27.857	-152.821	-1.346	-288.789
	20:KOM.14 (0.!	41.827	-2.49E 3	-27.857	-152.821	-1.346	-288.789
93	1:BEAN MAT	3.91E 3	93.6E 3	173.577	66.992	-18.342	-6.11E 3
	2:BEAN HIDL	194.625	7.55E 3	3.727	-0.890	-1.340	-295.045
	3:BEAN GEV	-787.480	-3.12E 3	-1.9E 3	-4.32E 3	22.584	1.72E 3
	4:BEAN GEV	787.480	3.12E 3	1.9E 3	4.32E 3	-22.584	-1.72E 3
	5:BEAN ANG	-38.307	-261.427	-227.509	-518.818	2.787	55.628
	6:BEAN ANG	-3.404	-20.128	-0.042	-0.031	-0.013	9.110
	7:KOM.1 (1.4 I	5.48E 3	131E 3	243.008	93.788	-25.678	-8.55E 3
	8:KOM.2 (1.2 I	5.01E 3	124E 3	214.256	78.966	-24.153	-7.8E 3
	9:KOM.3 (1.2 I	4.83E 3	119E 3	-151.994	-750.608	-18.890	-7.53E 3
	10:KOM.4 (1.2	4.88E 3	120E 3	211.952	79.450	-23.370	-7.61E 3
	11:KOM.5 (0.9	3.46E 3	83.8E 3	-207.794	-769.818	-12.048	-5.41E 3



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Job No	Sheet No <b>20</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

## Reactions Cont...

Node	L/C	Horizontal		Vertical	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	12:KOM.6 (0.9	3.52E 3	84.2E 3	156.152	60.242	-16.528	-5.48E 3
	13:KOM.7 (1.2	4.89E 3	120E 3	212.020	79.500	-23.349	-7.62E 3
	14:KOM.8 (1.2	4.89E 3	120E 3	212.020	79.500	-23.349	-7.62E 3
	15:KOM.9 (1.2	4.89E 3	120E 3	212.020	79.500	-23.349	-7.62E 3
	16:KOM.10 (1.:	4.89E 3	120E 3	212.020	79.500	-23.349	-7.62E 3
	17:KOM.11 (0.!	3.52E 3	84.2E 3	156.220	60.292	-16.507	-5.49E 3
	18:KOM.12 (0.!	3.52E 3	84.2E 3	156.220	60.292	-16.507	-5.49E 3
	19:KOM.13 (0.!	3.52E 3	84.2E 3	156.220	60.292	-16.507	-5.49E 3
	20:KOM.14 (0.!	3.52E 3	84.2E 3	156.220	60.292	-16.507	-5.49E 3
95	1:BEBAN MAT	46.930	-3.06E 3	7.569	9.876	-0.182	-320.089
	2:BEBAN HIDL	-0.004	-63.738	2.040	8.431	-0.103	-2.805
	3:BEBAN GEV	-51.135	404.822	-127.657	-933.982	-1.190	384.565
	4:BEBAN GEV	51.135	-404.822	127.657	933.982	1.190	-384.565
	5:BEBAN ANG	0.824	-16.877	-19.940	-144.710	-0.104	-4.071
	6:BEBAN ANG	-0.879	41.106	-0.286	-1.332	-0.185	5.498
	7:KOM.1 (1.4 [	65.701	-4.29E 3	10.597	13.827	-0.255	-448.124
	8:KOM.2 (1.2 [	56.309	-3.78E 3	12.347	25.341	-0.384	-388.594
	9:KOM.3 (1.2 [	57.630	-3.77E 3	-20.781	-211.253	-0.488	-393.424
	10:KOM.4 (1.2	54.905	-3.67E 3	10.665	18.152	-0.618	-378.113
	11:KOM.5 (0.9	43.555	-2.78E 3	-25.092	-222.646	-0.330	-294.593
	12:KOM.6 (0.9	40.830	-2.89E 3	6.354	6.758	-0.460	-279.282
	13:KOM.7 (1.2	56.312	-3.74E 3	11.123	20.282	-0.322	-386.911
	14:KOM.8 (1.2	56.312	-3.74E 3	11.123	20.282	-0.322	-386.911
	15:KOM.9 (1.2	56.312	-3.74E 3	11.123	20.282	-0.322	-386.911
	16:KOM.10 (1.:	56.312	-3.74E 3	11.123	20.282	-0.322	-386.911
	17:KOM.11 (0.!	42.237	-2.76E 3	6.812	8.889	-0.164	-288.080
	18:KOM.12 (0.!	42.237	-2.76E 3	6.812	8.889	-0.164	-288.080
	19:KOM.13 (0.!	42.237	-2.76E 3	6.812	8.889	-0.164	-288.080
	20:KOM.14 (0.!	42.237	-2.76E 3	6.812	8.889	-0.164	-288.080
97	1:BEBAN MAT	3.47E 3	99.5E 3	-645.382	-1.13E 3	-42.135	-5.2E 3
	2:BEBAN HIDL	165.782	7.51E 3	-20.785	-37.453	-0.724	-243.533
	3:BEBAN GEV	-134.489	988.405	-1.89E 3	-4.31E 3	-11.656	743.187
	4:BEBAN GEV	134.489	-988.405	1.89E 3	4.31E 3	11.656	-743.187
	5:BEBAN ANG	41.424	309.309	-227.851	-520.014	-1.864	-62.992
	6:BEBAN ANG	-3.318	-19.817	0.127	0.219	0.007	8.977
	7:KOM.1 (1.4 [	4.86E 3	139E 3	-903.535	-1.59E 3	-58.989	-7.27E 3
	8:KOM.2 (1.2 [	4.43E 3	131E 3	-807.715	-1.42E 3	-51.721	-6.63E 3
	9:KOM.3 (1.2 [	4.4E 3	127E 3	-1.16E 3	-2.23E 3	-54.270	-6.58E 3
	10:KOM.4 (1.2	4.33E 3	127E 3	-795.041	-1.4E 3	-51.275	-6.46E 3
	11:KOM.5 (0.9	3.19E 3	90.1E 3	-945.405	-1.85E 3	-40.905	-4.78E 3
	12:KOM.6 (0.9	3.12E 3	89.5E 3	-580.841	-1.02E 3	-37.910	-4.66E 3
	13:KOM.7 (1.2	4.33E 3	127E 3	-795.244	-1.4E 3	-51.287	-6.48E 3
	14:KOM.8 (1.2	4.33E 3	127E 3	-795.244	-1.4E 3	-51.287	-6.48E 3
	15:KOM.9 (1.2	4.33E 3	127E 3	-795.244	-1.4E 3	-51.287	-6.48E 3
	16:KOM.10 (1.:	4.33E 3	127E 3	-795.244	-1.4E 3	-51.287	-6.48E 3
	17:KOM.11 (0.!	3.13E 3	89.6E 3	-580.844	-1.02E 3	-37.922	-4.68E 3
	18:KOM.12 (0.!	3.13E 3	89.6E 3	-580.844	-1.02E 3	-37.922	-4.68E 3
	19:KOM.13 (0.!	3.13E 3	89.6E 3	-580.844	-1.02E 3	-37.922	-4.68E 3
	20:KOM.14 (0.!	3.13E 3	89.6E 3	-580.844	-1.02E 3	-37.922	-4.68E 3



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Job No	Sheet No <b>21</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
99	1:BEAN MAT	439.542	-33.716	-87.546	-420.621	32.832	-934.307
	2:BEAN HIDL	-0.769	1.05E 3	0.355	-9.395	1.301	-6.400
	3:BEAN GEN	-402.232	-497.875	-1.24E 3	-3.37E 3	-2.278	1.18E 3
	4:BEAN GEN	402.232	497.875	1.24E 3	3.37E 3	2.278	-1.18E 3
	5:BEAN ANG	-3.030	-390.566	-153.690	-413.842	0.286	7.955
	6:BEAN ANG	-2.643	17.002	0.401	0.644	-0.016	7.966
	7:KOM.1 (1.4 E	615.359	-47.202	-122.584	-588.870	45.965	-1.31E 3
	8:KOM.2 (1.2 E	526.220	1.64E 3	-104.487	-519.778	41.479	-1.13E 3
	9:KOM.3 (1.2 E	521.834	386.035	-350.604	-1.18E 3	41.157	-1.11E 3
	10:KOM.4 (1.2	522.452	1.04E 3	-104.058	-513.111	40.673	-1.11E 3
	11:KOM.5 (0.9	390.740	-655.249	-324.695	-1.04E 3	30.007	-828.148
	12:KOM.6 (0.9	391.359	-3.141	-78.149	-377.529	29.523	-828.131
	13:KOM.7 (1.2	526.681	1.01E 3	-104.700	-514.141	40.699	-1.13E 3
	14:KOM.8 (1.2	526.681	1.01E 3	-104.700	-514.141	40.699	-1.13E 3
	15:KOM.9 (1.2	526.681	1.01E 3	-104.700	-514.141	40.699	-1.13E 3
	16:KOM.10 (1.;	526.681	1.01E 3	-104.700	-514.141	40.699	-1.13E 3
	17:KOM.11 (0.!	395.588	-30.344	-78.791	-378.559	29.549	-840.876
	18:KOM.12 (0.!	395.588	-30.344	-78.791	-378.559	29.549	-840.876
	19:KOM.13 (0.!	395.588	-30.344	-78.791	-378.559	29.549	-840.876
	20:KOM.14 (0.!	395.588	-30.344	-78.791	-378.559	29.549	-840.876
101	1:BEAN MAT	380.350	-164.039	-482.569	-999.153	35.951	-804.388
	2:BEAN HIDL	-8.110	1.06E 3	-19.454	-38.285	0.800	4.092
	3:BEAN GEN	-349.024	4.35E 3	-1.34E 3	-3.52E 3	-0.089	1.05E 3
	4:BEAN GEN	349.024	-4.35E 3	1.34E 3	3.52E 3	0.089	-1.05E 3
	5:BEAN ANG	4.893	413.721	-154.304	-414.481	0.159	-11.540
	6:BEAN ANG	-2.670	16.981	-0.311	-0.420	0.009	8.029
	7:KOM.1 (1.4 E	532.491	-229.654	-675.597	-1.4E 3	50.331	-1.13E 3
	8:KOM.2 (1.2 E	446.645	1.5E 3	-610.210	-1.26E 3	44.421	-958.718
	9:KOM.3 (1.2 E	458.139	1.52E 3	-845.424	-1.9E 3	44.195	-979.638
	10:KOM.4 (1.2	448.038	888.256	-599.036	-1.24E 3	43.955	-948.327
	11:KOM.5 (0.9	350.144	514.318	-681.199	-1.56E 3	32.610	-742.413
	12:KOM.6 (0.9	338.043	-120.465	-434.810	-899.909	32.370	-711.102
	13:KOM.7 (1.2	450.311	861.087	-598.537	-1.24E 3	43.941	-961.174
	14:KOM.8 (1.2	450.311	861.087	-598.537	-1.24E 3	43.941	-961.174
	15:KOM.9 (1.2	450.311	861.087	-598.537	-1.24E 3	43.941	-961.174
	16:KOM.10 (1.;	450.311	861.087	-598.537	-1.24E 3	43.941	-961.174
	17:KOM.11 (0.!	342.315	-147.635	-434.312	-899.237	32.356	-723.949
	18:KOM.12 (0.!	342.315	-147.635	-434.312	-899.237	32.356	-723.949
	19:KOM.13 (0.!	342.315	-147.635	-434.312	-899.237	32.356	-723.949
	20:KOM.14 (0.!	342.315	-147.635	-434.312	-899.237	32.356	-723.949
103	1:BEAN MAT	31.734	4.314	-14.699	-40.657	-8.811	-91.368
	2:BEAN HIDL	1.037	-30.683	2.024	1.427	-0.210	-2.978
	3:BEAN GEN	-84.757	535.048	-492.616	-1.15E 3	26.072	240.759
	4:BEAN GEN	84.757	-535.048	492.616	1.15E 3	-26.072	-240.759
	5:BEAN ANG	0.719	75.506	-59.198	-138.216	-0.041	-2.010
	6:BEAN ANG	-0.660	-0.605	-0.002	-0.006	0.191	1.876
	7:KOM.1 (1.4 E	44.427	6.040	-20.578	-56.920	-12.336	-127.915
	8:KOM.2 (1.2 E	39.739	-43.916	-14.401	-46.506	-10.909	-114.406
	9:KOM.3 (1.2 E	40.267	95.303	-110.331	-268.507	-10.849	-115.836





Software licensed to Snow Panther [LZO]

Job No

Sheet No

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Rev

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Part

Ref

By

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Chd

Client

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Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	10:KOM.4 (1.2	38.061	-26.475	-15.618	-47.371	-10.477	-109.617
	11:KOM.5 (0.9	29.710	124.692	-107.946	-257.737	-7.996	-85.448
	12:KOM.6 (0.9	27.504	2.914	-13.232	-36.601	-7.624	-79.229
	13:KOM.7 (1.2	39.117	-25.506	-15.615	-47.362	-10.783	-112.620
	14:KOM.8 (1.2	39.117	-25.506	-15.615	-47.362	-10.783	-112.620
	15:KOM.9 (1.2	39.117	-25.506	-15.615	-47.362	-10.783	-112.620
	16:KOM.10 (1.	39.117	-25.506	-15.615	-47.362	-10.783	-112.620
	17:KOM.11 (0.1	28.560	3.883	-13.229	-36.592	-7.930	-82.231
	18:KOM.12 (0.1	28.560	3.883	-13.229	-36.592	-7.930	-82.231
	19:KOM.13 (0.1	28.560	3.883	-13.229	-36.592	-7.930	-82.231
	20:KOM.14 (0.1	28.560	3.883	-13.229	-36.592	-7.930	-82.231
105	1:BEBAN MAT	31.731	-71.757	-13.049	-37.894	-9.891	-91.371
	2:BEBAN HIDL	1.035	-33.194	2.022	1.344	-0.297	-2.976
	3:BEBAN GEV	-84.717	688.999	-493.837	-1.16E 3	24.603	240.689
	4:BEBAN GEV	84.717	-688.999	493.837	1.16E 3	-24.603	-240.689
	5:BEBAN ANG	0.724	74.073	-59.311	-138.534	-0.204	-2.019
	6:BEBAN ANG	-0.660	0.611	-0.003	-0.008	0.191	1.876
	7:KOM.1 (1.4 E	44.423	-100.460	-18.269	-53.051	-13.848	-127.920
	8:KOM.2 (1.2 E	39.732	-139.219	-12.423	-43.322	-12.345	-114.407
	9:KOM.3 (1.2 E	40.269	-0.786	-108.534	-265.783	-12.493	-115.852
	10:KOM.4 (1.2	38.055	-118.324	-13.642	-44.141	-11.861	-109.620
	11:KOM.5 (0.9	29.716	53.936	-106.641	-255.758	-9.229	-85.465
	12:KOM.6 (0.9	27.501	-63.603	-11.750	-34.117	-8.596	-79.232
	13:KOM.7 (1.2	39.111	-119.302	-13.637	-44.129	-12.167	-112.622
	14:KOM.8 (1.2	39.111	-119.302	-13.637	-44.129	-12.167	-112.622
	15:KOM.9 (1.2	39.111	-119.302	-13.637	-44.129	-12.167	-112.622
	16:KOM.10 (1.	39.111	-119.302	-13.637	-44.129	-12.167	-112.622
	17:KOM.11 (0.1	28.558	-64.581	-11.744	-34.104	-8.902	-82.234
	18:KOM.12 (0.1	28.558	-64.581	-11.744	-34.104	-8.902	-82.234
	19:KOM.13 (0.1	28.558	-64.581	-11.744	-34.104	-8.902	-82.234
	20:KOM.14 (0.1	28.558	-64.581	-11.744	-34.104	-8.902	-82.234
107	1:BEBAN MAT	42.403	-124.378	-23.641	-48.415	11.813	-121.181
	2:BEBAN HIDL	1.486	-36.870	-4.829	-7.958	0.418	-4.224
	3:BEBAN GEV	-99.388	-520.622	-493.296	-1.16E 3	-27.611	281.656
	4:BEBAN GEV	99.388	520.622	493.296	1.16E 3	27.611	-281.656
	5:BEBAN ANG	-1.086	-73.165	-59.052	-138.104	-0.152	3.030
	6:BEBAN ANG	-0.653	0.599	-0.002	-0.004	-0.190	1.856
	7:KOM.1 (1.4 E	59.365	-174.130	-33.097	-67.782	16.538	-169.654
	8:KOM.2 (1.2 E	53.262	-208.247	-36.095	-70.831	14.844	-152.177
	9:KOM.3 (1.2 E	50.633	-303.188	-127.681	-287.024	14.350	-144.793
	10:KOM.4 (1.2	51.328	-185.166	-33.200	-66.063	14.289	-146.872
	11:KOM.5 (0.9	38.425	-229.004	-115.760	-264.541	10.388	-104.214
	12:KOM.6 (0.9	37.118	-110.982	-21.279	-43.580	10.327	-106.093
	13:KOM.7 (1.2	52.370	-186.125	-33.198	-66.057	14.593	-149.642
	14:KOM.8 (1.2	52.370	-186.125	-33.198	-66.057	14.593	-149.642
	15:KOM.9 (1.2	52.370	-186.125	-33.198	-66.057	14.593	-149.642
	16:KOM.10 (1.	52.370	-186.125	-33.198	-66.057	14.593	-149.642
	17:KOM.11 (0.1	38.163	-111.941	-21.276	-43.574	10.632	-109.063
	18:KOM.12 (0.1	38.163	-111.941	-21.276	-43.574	10.632	-109.063



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Job No	Sheet No <b>23</b>	Rev
Part		
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By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal		Horizontal FZ (kg)	Moment		
		FX (kg)	Vertical FY (kg)		MX (kg·m)	MY (kg·m)	MZ (kg·m)
	19:KOM.13 (0.!	38.163	-111.941	-21.276	-43.574	10.632	-109.063
	20:KOM.14 (0.!	38.163	-111.941	-21.276	-43.574	10.632	-109.063
109	1:BEAN MAT	42.355	-31.498	-24.670	-52.104	10.397	-121.100
	2:BEAN HIDL	1.481	-33.610	-4.927	-8.288	0.238	-4.216
	3:BEAN GEV	-99.361	-701.437	-489.843	-1.15E 3	-25.899	281.603
	4:BEAN GEV	99.361	701.437	489.843	1.15E 3	25.899	-281.603
	5:BEAN ANG	-1.079	-75.046	-58.607	-136.973	0.041	3.018
	6:BEAN ANG	-0.653	-0.607	-0.005	-0.012	-0.190	1.856
	7:KOM.1 (1.4 I	59.297	-44.098	-34.539	-72.945	14.555	-169.540
	8:KOM.2 (1.2 I	53.197	-91.574	-37.487	-75.785	12.856	-152.066
	9:KOM.3 (1.2 I	50.582	-191.482	-128.302	-289.970	12.780	-144.707
	10:KOM.4 (1.2	51.263	-72.379	-34.540	-70.831	12.409	-146.566
	11:KOM.5 (0.9	36.394	-148.423	-115.974	-266.050	9.423	-104.161
	12:KOM.6 (0.9	37.075	-29.319	-22.212	-46.912	9.053	-106.020
	13:KOM.7 (1.2	52.308	-71.408	-34.531	-70.812	12.714	-149.536
	14:KOM.8 (1.2	52.308	-71.408	-34.531	-70.812	12.714	-149.536
	15:KOM.9 (1.2	52.308	-71.408	-34.531	-70.812	12.714	-149.536
	16:KOM.10 (1.:	52.308	-71.408	-34.531	-70.812	12.714	-149.536
	17:KOM.11 (0.!	38.120	-28.349	-22.203	-46.893	9.357	-108.990
	18:KOM.12 (0.!	38.120	-28.349	-22.203	-46.893	9.357	-108.990
	19:KOM.13 (0.!	38.120	-28.349	-22.203	-46.893	9.357	-108.990
	20:KOM.14 (0.!	38.120	-28.349	-22.203	-46.893	9.357	-108.990
111	1:BEAN MAT	-247.851	40E 3	-447.868	-904.773	-35.308	1.502
	2:BEAN HIDL	-1.426	10.6E 3	41.880	51.426	-0.702	-10.909
	3:BEAN GEV	-773.127	5.93E 3	-2.04E 3	-4.63E 3	5.316	1.93E 3
	4:BEAN GEV	773.127	-5.93E 3	2.04E 3	4.63E 3	-5.316	-1.93E 3
	5:BEAN ANG	-8.714	1.15E 3	-241.306	-550.665	0.327	12.052
	6:BEAN ANG	-5.011	5.025	-0.132	-0.250	-0.001	13.176
	7:KOM.1 (1.4 I	-346.991	56E 3	-627.015	-1.27E 3	-49.431	2.102
	8:KOM.2 (1.2 I	-299.702	65E 3	-470.465	-1E 3	-43.492	-15.653
	9:KOM.3 (1.2 I	-312.789	60.5E 3	-881.671	-1.92E 3	-42.549	10.176
	10:KOM.4 (1.2	-306.864	58.7E 3	-495.793	-1.03E 3	-43.073	11.974
	11:KOM.5 (0.9	-237.008	37.8E 3	-789.171	-1.7E 3	-31.255	20.635
	12:KOM.6 (0.9	-231.083	36E 3	-403.292	-814.695	-31.779	22.433
	13:KOM.7 (1.2	-298.847	58.7E 3	-495.581	-1.03E 3	-43.071	-9.107
	14:KOM.8 (1.2	-298.847	58.7E 3	-495.581	-1.03E 3	-43.071	-9.107
	15:KOM.9 (1.2	-298.847	58.7E 3	-495.581	-1.03E 3	-43.071	-9.107
	16:KOM.10 (1.:	-298.847	58.7E 3	-495.581	-1.03E 3	-43.071	-9.107
	17:KOM.11 (0.!	-223.066	36E 3	-403.081	-814.295	-31.777	1.352
	18:KOM.12 (0.!	-223.066	36E 3	-403.081	-814.295	-31.777	1.352
	19:KOM.13 (0.!	-223.066	36E 3	-403.081	-814.295	-31.777	1.352
	20:KOM.14 (0.!	-223.066	36E 3	-403.081	-814.295	-31.777	1.352
113	1:BEAN MAT	-407.374	34.6E 3	-174.185	-493.770	-45.174	379.012
	2:BEAN HIDL	-11.621	10.6E 3	-62.307	-98.752	-1.729	9.084
	3:BEAN GEV	-659.887	-5.66E 3	-2.03E 3	-4.81E 3	1.017	1.8E 3
	4:BEAN GEV	659.887	5.66E 3	2.03E 3	4.81E 3	-1.017	-1.8E 3
	5:BEAN ANG	6.607	-882.389	-241.333	-550.271	-0.095	-6.488
	6:BEAN ANG	-5.009	4.839	-0.003	-0.057	-0.010	13.162
	7:KOM.1 (1.4 I	-570.323	48.5E 3	-243.859	-891.278	-63.243	530.617



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Part		
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### Reactions Cont...

Node	L/C	Horizontal			Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg m)	MY (kg m)	MZ (kg m)
	8:KOM.2 (1.2 [	-507.442	58.5E 3	-308.713	-750.528	-56.974	489.349
	9:KOM.3 (1.2 [	-489.898	50.7E 3	-657.462	-1.57E 3	-56.088	453.517
	10:KOM.4 (1.2	-508.483	52.2E 3	-271.334	-691.389	-55.952	484.959
	11:KOM.5 (0.9	-356.065	29.8E 3	-542.900	-1.32E 3	-40.808	330.730
	12:KOM.6 (0.9	-374.650	31.2E 3	-156.771	-444.485	-40.672	362.171
	13:KOM.7 (1.2	-500.470	52.2E 3	-271.329	-691.277	-55.937	463.899
	14:KOM.8 (1.2	-500.470	52.2E 3	-271.329	-691.277	-55.937	463.899
	15:KOM.9 (1.2	-500.470	52.2E 3	-271.329	-691.277	-55.937	463.899
	16:KOM.10 (1.:	-500.470	52.2E 3	-271.329	-691.277	-55.937	463.899
	17:KOM.11 (0.!	-366.636	31.2E 3	-156.766	-444.393	-40.656	341.111
	18:KOM.12 (0.!	-366.636	31.2E 3	-156.766	-444.393	-40.656	341.111
	19:KOM.13 (0.!	-366.636	31.2E 3	-156.766	-444.393	-40.656	341.111
	20:KOM.14 (0.!	-366.636	31.2E 3	-156.766	-444.393	-40.656	341.111
115	1:BEBAN MAT	208.324	22.9E 3	-394.294	-745.820	-34.445	-540.702
	2:BEBAN HIDL	14.315	13.1E 3	-50.404	-78.355	-1.302	-29.290
	3:BEBAN GEM	-742.490	5.05E 3	-1.77E 3	-4.23E 3	0.380	1.92E 3
	4:BEBAN GEM	742.490	-5.05E 3	1.77E 3	4.23E 3	-0.380	-1.92E 3
	5:BEBAN ANG	0.090	786.839	-209.549	-503.426	-0.264	3.471
	6:BEBAN ANG	-5.203	3.455	0.066	0.057	-0.007	13.440
	7:KOM.1 (1.4 [	291.653	32.1E 3	-552.012	-1.04E 3	-48.223	-756.983
	8:KOM.2 (1.2 [	272.892	48.5E 3	-553.799	-1.02E 3	-43.418	-695.707
	9:KOM.3 (1.2 [	264.447	41.9E 3	-858.835	-1.78E 3	-43.058	-672.579
	10:KOM.4 (1.2	255.979	40.6E 3	-523.451	-973.247	-42.647	-656.628
	11:KOM.5 (0.9	187.635	21.9E 3	-690.143	-1.48E 3	-31.423	-481.078
	12:KOM.6 (0.9	179.167	20.8E 3	-354.759	-671.146	-31.011	-465.128
	13:KOM.7 (1.2	264.303	40.6E 3	-523.557	-973.338	-42.636	-678.133
	14:KOM.8 (1.2	264.303	40.6E 3	-523.557	-973.338	-42.636	-678.133
	15:KOM.9 (1.2	264.303	40.6E 3	-523.557	-973.338	-42.636	-678.133
	16:KOM.10 (1.:	264.303	40.6E 3	-523.557	-973.338	-42.636	-678.133
	17:KOM.11 (0.!	187.491	20.6E 3	-354.865	-671.238	-31.000	-486.632
	18:KOM.12 (0.!	187.491	20.6E 3	-354.865	-671.238	-31.000	-486.632
	19:KOM.13 (0.!	187.491	20.6E 3	-354.865	-671.238	-31.000	-486.632
	20:KOM.14 (0.!	187.491	20.6E 3	-354.865	-671.238	-31.000	-486.632
117	1:BEBAN MAT	187.991	26.4E 3	-217.653	-437.647	-24.490	-539.008
	2:BEBAN HIDL	4.455	13.5E 3	-32.551	-50.433	-0.733	-15.920
	3:BEBAN GEM	-761.175	6.14E 3	-1.73E 3	-4.17E 3	-0.472	1.95E 3
	4:BEBAN GEM	761.175	-6.14E 3	1.73E 3	4.17E 3	0.472	-1.95E 3
	5:BEBAN ANG	-3.793	1.08E 3	-201.939	-491.646	-0.340	9.478
	6:BEBAN ANG	-5.134	0.308	-0.035	-0.081	-0.005	13.336
	7:KOM.1 (1.4 [	263.187	37E 3	-304.714	-612.706	-34.286	-754.611
	8:KOM.2 (1.2 [	232.717	53.4E 3	-313.265	-605.870	-30.561	-672.282
	9:KOM.3 (1.2 [	223.975	47E 3	-616.837	-1.36E 3	-30.666	-647.564
	10:KOM.4 (1.2	221.830	45.3E 3	-293.790	-575.740	-30.128	-641.392
	11:KOM.5 (0.9	163.123	25.5E 3	-518.990	-1.18E 3	-22.586	-469.842
	12:KOM.6 (0.9	160.978	23.8E 3	-195.943	-394.013	-22.049	-463.769
	13:KOM.7 (1.2	230.044	45.3E 3	-293.734	-575.610	-30.121	-662.730
	14:KOM.8 (1.2	230.044	45.3E 3	-293.734	-575.610	-30.121	-662.730
	15:KOM.9 (1.2	230.044	45.3E 3	-293.734	-575.610	-30.121	-662.730
	16:KOM.10 (1.:	230.044	45.3E 3	-293.734	-575.610	-30.121	-662.730



Software licensed to Snow Panther [LZO]

Job No	Sheet No <b>25</b>	Rev
Part		
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By	Date 06-Oct-10	Chd
Client	File endhik-2 slp 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	17:KOM.11 (0.1	169.192	23.8E 3	-195.887	-393.883	-22.041	-485.107
	18:KOM.12 (0.1	169.192	23.8E 3	-195.887	-393.883	-22.041	-485.107
	19:KOM.13 (0.1	169.192	23.8E 3	-195.887	-393.883	-22.041	-485.107
	20:KOM.14 (0.1	169.192	23.8E 3	-195.887	-393.883	-22.041	-485.107
119	1:BEAN MAT	202.189	25.1E 3	-119.365	-260.777	-15.291	-585.064
	2:BEAN HIDL	15.664	10.5E 3	15.944	20.941	-0.119	-33.000
	3:BEAN GEN	-754.226	5.77E 3	-1.72E 3	-4.16E 3	-2.191	1.95E 3
	4:BEAN GEN	754.226	-5.77E 3	1.72E 3	4.16E 3	2.191	-1.95E 3
	5:BEAN ANG	-4.153	1.09E 3	-200.042	-488.118	-0.347	10.275
	6:BEAN ANG	-5.063	0.710	-0.032	-0.072	-0.003	13.230
	7:KOM.1 (1.4 E	283.065	35.1E 3	-167.111	-365.088	-21.407	-819.090
	8:KOM.2 (1.2 E	267.689	47E 3	-117.728	-279.427	-18.539	-754.877
	9:KOM.3 (1.2 E	251.646	42.4E 3	-447.362	-1.07E 3	-19.023	-718.637
	10:KOM.4 (1.2	250.190	40.7E 3	-127.346	-292.106	-18.473	-713.906
	11:KOM.5 (0.9	175.325	24.3E 3	-427.496	-1.02E 3	-14.318	-510.118
	12:KOM.6 (0.9	173.869	22.6E 3	-107.480	-234.815	-13.767	-505.389
	13:KOM.7 (1.2	258.291	40.7E 3	-127.294	-291.991	-18.468	-735.077
	14:KOM.8 (1.2	258.291	40.7E 3	-127.294	-291.991	-18.468	-735.077
	15:KOM.9 (1.2	258.291	40.7E 3	-127.294	-291.991	-18.468	-735.077
	16:KOM.10 (1.:	258.291	40.7E 3	-127.294	-291.991	-18.468	-735.077
	17:KOM.11 (0.1	181.970	22.8E 3	-107.429	-234.700	-13.762	-526.558
	18:KOM.12 (0.1	181.970	22.6E 3	-107.429	-234.700	-13.762	-526.558
	19:KOM.13 (0.1	181.970	22.6E 3	-107.429	-234.700	-13.762	-526.558
	20:KOM.14 (0.1	181.970	22.6E 3	-107.429	-234.700	-13.762	-526.558
121	1:BEAN MAT	312.304	32.2E 3	42.065	-8.197	-8.135	-767.226
	2:BEAN HIDL	16.826	13.1E 3	-13.277	-22.115	0.294	-35.233
	3:BEAN GEN	-776.635	4.91E 3	-1.75E 3	-4.2E 3	-3.296	1.98E 3
	4:BEAN GEN	776.635	-4.91E 3	1.75E 3	4.2E 3	3.296	-1.98E 3
	5:BEAN ANG	-7.102	899.093	-203.123	-491.904	-0.291	14.718
	6:BEAN ANG	-5.094	-2.021	-0.096	-0.160	-0.003	13.272
	7:KOM.1 (1.4 E	437.226	45E 3	58.890	-8.676	-11.389	-1.07E 3
	8:KOM.2 (1.2 E	401.686	59.5E 3	29.235	-42.820	-9.292	-977.045
	9:KOM.3 (1.2 E	380.227	53.1E 3	-287.795	-816.599	-9.934	-932.355
	10:KOM.4 (1.2	383.440	51.7E 3	37.047	-28.808	-9.473	-934.669
	11:KOM.5 (0.9	269.710	30.4E 3	-287.138	-792.625	-7.787	-666.954
	12:KOM.6 (0.9	272.923	28.9E 3	37.705	-5.834	-7.326	-669.268
	13:KOM.7 (1.2	391.591	51.7E 3	37.201	-29.551	-9.468	-955.905
	14:KOM.8 (1.2	391.591	51.7E 3	37.201	-29.551	-9.468	-955.905
	15:KOM.9 (1.2	391.591	51.7E 3	37.201	-29.551	-9.468	-955.905
	16:KOM.10 (1.:	391.591	51.7E 3	37.201	-29.551	-9.468	-955.905
	17:KOM.11 (0.1	281.074	28.9E 3	37.858	-5.578	-7.321	-690.504
	18:KOM.12 (0.1	281.074	28.9E 3	37.858	-5.578	-7.321	-690.504
	19:KOM.13 (0.1	281.074	28.9E 3	37.858	-5.578	-7.321	-690.504
	20:KOM.14 (0.1	281.074	28.9E 3	37.858	-5.578	-7.321	-690.504
123	1:BEAN MAT	663.712	95E 3	1.24E 3	1.76E 3	-3.996	-1.29E 3
	2:BEAN HIDL	13.628	13.3E 3	-1.227	-5.688	0.307	-30.731
	3:BEAN GEN	-860.780	-5.46E 3	-2.04E 3	-4.62E 3	-2.944	2.1E 3
	4:BEAN GEN	860.780	5.46E 3	2.04E 3	4.62E 3	2.944	-2.1E 3
	5:BEAN ANG	-17.562	-689.917	-235.218	-538.205	-0.187	29.745



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Job No	Sheet No <b>26</b>	Rev
Part		
Ref		
By	Date: 06-Oct-10	Chd
Client	File: endhik-2 sip 4.std	Date/Time: 21-Mar-2011 15:52

## Reactions Cont...

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	6: BEBAN ANG	-5.116	-17.729	-0.476	-0.710	-0.003	13.299
	7: KOM.1 (1.4 [	929.196	133E 3	1.74E 3	2.46E 3	-5.594	-1.81E 3
	8: KOM.2 (1.2 [	818.259	135E 3	1.49E 3	2.1E 3	-4.304	-1.6E 3
	9: KOM.3 (1.2 [	781.982	126E 3	1.12E 3	1.24E 3	-4.788	-1.54E 3
	10: KOM.4 (1.2	801.897	127E 3	1.49E 3	2.1E 3	-4.493	-1.56E 3
	11: KOM.5 (0.9	569.241	84.4E 3	744.132	719.080	-3.896	-1.12E 3
	12: KOM.6 (0.9	589.155	85.5E 3	1.12E 3	1.58E 3	-3.601	-1.14E 3
	13: KOM.7 (1.2	810.082	127E 3	1.49E 3	2.1E 3	-4.488	-1.58E 3
	14: KOM.8 (1.2	810.082	127E 3	1.49E 3	2.1E 3	-4.488	-1.58E 3
	15: KOM.9 (1.2	810.082	127E 3	1.49E 3	2.1E 3	-4.488	-1.58E 3
	16: KOM.10 (1.:	810.082	127E 3	1.49E 3	2.1E 3	-4.488	-1.58E 3
	17: KOM.11 (0.:	597.341	85.5E 3	1.12E 3	1.58E 3	-3.596	-1.17E 3
	18: KOM.12 (0.:	597.341	85.5E 3	1.12E 3	1.58E 3	-3.596	-1.17E 3
	19: KOM.13 (0.:	597.341	85.5E 3	1.12E 3	1.58E 3	-3.596	-1.17E 3
	20: KOM.14 (0.:	597.341	85.5E 3	1.12E 3	1.58E 3	-3.596	-1.17E 3
125	1: BEBAN MAT	-120.421	80.7E 3	396.714	511.360	-2.257	-168.908
	2: BEBAN HIDL	-4.520	11.3E 3	-56.355	-86.933	-0.005	-4.161
	3: BEBAN GEV	-698.858	-11.4E 3	-2.03E 3	-4.59E 3	3.171	1.86E 3
	4: BEBAN GEV	698.858	11.4E 3	2.03E 3	4.59E 3	-3.171	-1.86E 3
	5: BEBAN ANG	0.385	-1.79E 3	-239.604	-543.652	0.470	3.245
	6: BEBAN ANG	-4.903	1.060	0.000	-0.013	-0.003	12.985
	7: KOM.1 (1.4 [	-168.590	113E 3	555.400	715.905	-3.160	-236.471
	8: KOM.2 (1.2 [	-151.738	115E 3	385.889	474.539	-2.716	-209.347
	9: KOM.3 (1.2 [	-148.410	105E 3	36.336	-343.144	-1.961	-201.659
	10: KOM.4 (1.2	-156.870	108E 3	419.702	526.679	-2.717	-186.074
	11: KOM.5 (0.9	-107.763	69.8E 3	-25.323	-409.619	-1.280	-146.626
	12: KOM.6 (0.9	-116.224	72.7E 3	357.043	460.204	-2.035	-131.240
	13: KOM.7 (1.2	-149.026	108E 3	419.702	526.699	-2.713	-206.850
	14: KOM.8 (1.2	-149.026	108E 3	419.702	526.699	-2.713	-206.850
	15: KOM.9 (1.2	-149.026	108E 3	419.702	526.699	-2.713	-206.850
	16: KOM.10 (1.:	-149.026	108E 3	419.702	526.699	-2.713	-206.850
	17: KOM.11 (0.:	-108.379	72.7E 3	357.043	460.224	-2.031	-152.017
	18: KOM.12 (0.:	-108.379	72.7E 3	357.043	460.224	-2.031	-152.017
	19: KOM.13 (0.:	-108.379	72.7E 3	357.043	460.224	-2.031	-152.017
	20: KOM.14 (0.:	-108.379	72.7E 3	357.043	460.224	-2.031	-152.017
127	1: BEBAN MAT	220.195	33.5E 3	-90.801	-186.746	-5.694	-676.800
	2: BEBAN HIDL	9.248	10.9E 3	-82.414	-94.928	-0.634	-23.993
	3: BEBAN GEV	-696.842	-5.14E 3	-1.98E 3	-4.54E 3	9.127	1.85E 3
	4: BEBAN GEV	696.842	5.14E 3	1.98E 3	4.54E 3	-9.127	-1.85E 3
	5: BEBAN ANG	0.457	-835.104	-232.875	-536.402	1.123	2.158
	6: BEBAN ANG	-4.897	2.191	0.019	0.018	-0.003	12.976
	7: KOM.1 (1.4 [	308.273	46.9E 3	-127.261	-261.444	-7.972	-947.520
	8: KOM.2 (1.2 [	279.030	57.7E 3	-208.944	-375.979	-7.848	-850.548
	9: KOM.3 (1.2 [	274.213	49.8E 3	-544.095	-1.18E 3	-5.671	-932.700
	10: KOM.4 (1.2	265.647	51.1E 3	-171.465	-318.994	-7.472	-815.392
	11: KOM.5 (0.9	198.906	28.8E 3	-454.410	-1.03E 3	-3.328	-605.667
	12: KOM.6 (0.9	190.341	30.2E 3	-81.781	-168.042	-5.129	-588.359
	13: KOM.7 (1.2	273.482	51.1E 3	-171.495	-319.023	-7.467	-836.153
	14: KOM.8 (1.2	273.482	51.1E 3	-171.495	-319.023	-7.467	-836.153



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Part		
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By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	LC	Horizontal		Vertical	Moment		
		FX (kg)	FY (kg)		FZ (kg)	MX (kg·m)	MY (kg·m)
	15:KOM.9 (1.2	273.482	51.1E 3	-171.495	-319.023	-7.467	-836.153
	16:KOM.10 (1.	273.482	51.1E 3	-171.495	-319.023	-7.467	-836.153
	17:KOM.11 (0.	198.176	30.1E 3	-81.811	-168.071	-5.125	-609.120
	18:KOM.12 (0.	198.176	30.1E 3	-81.811	-168.071	-5.125	-609.120
	19:KOM.13 (0.	198.176	30.1E 3	-81.811	-168.071	-5.125	-609.120
	20:KOM.14 (0.	198.176	30.1E 3	-81.811	-168.071	-5.125	-609.120
129	1:BEBAN MAT	285.915	26.2E 3	-102.853	-195.071	-1.465	-791.776
	2:BEBAN HIDL	15.192	13.1E 3	-34.275	-51.973	-0.738	-32.662
	3:BEBAN GEV	-703.524	4.85E 3	-1.77E 3	-4.26E 3	10.092	1.85E 3
	4:BEBAN GEV	703.524	-4.85E 3	1.77E 3	4.26E 3	-10.092	-1.85E 3
	5:BEBAN ANG	2.650	738.403	-209.944	-505.033	1.203	-1.385
	6:BEBAN ANG	-5.094	4.296	0.114	0.160	-0.003	13.263
	7:KOM.1 (1.4 L	400.280	36.7E 3	-143.994	-273.100	-2.051	-1.11E 3
	8:KOM.2 (1.2 L	367.405	52.5E 3	-178.263	-317.243	-2.940	-1E 3
	9:KOM.3 (1.2 L	362.529	45.8E 3	-493.608	-1.09E 3	-0.571	-985.010
	10:KOM.4 (1.2	350.140	44.6E 3	-157.515	-285.803	-2.501	-961.573
	11:KOM.5 (0.9	261.563	24.8E 3	-428.477	-983.616	0.607	-714.815
	12:KOM.6 (0.9	249.173	23.6E 3	-92.385	-175.308	-1.323	-691.378
	13:KOM.7 (1.2	358.290	44.6E 3	-157.698	-286.059	-2.497	-982.794
	14:KOM.8 (1.2	358.290	44.6E 3	-157.698	-286.059	-2.497	-982.794
	15:KOM.9 (1.2	358.290	44.6E 3	-157.698	-286.059	-2.497	-982.794
	16:KOM.10 (1.	358.290	44.6E 3	-157.698	-286.059	-2.497	-982.794
	17:KOM.11 (0.	257.323	23.6E 3	-92.568	-175.564	-1.319	-712.599
	18:KOM.12 (0.	257.323	23.6E 3	-92.568	-175.564	-1.319	-712.599
	19:KOM.13 (0.	257.323	23.6E 3	-92.568	-175.564	-1.319	-712.599
	20:KOM.14 (0.	257.323	23.6E 3	-92.568	-175.564	-1.319	-712.599
131	1:BEBAN MAT	322.903	26.6E 3	-82.367	-166.768	4.625	-870.647
	2:BEBAN HIDL	-0.542	13.3E 3	-13.620	-20.462	-0.423	-10.402
	3:BEBAN GEV	-731.432	6.14E 3	-1.75E 3	-4.25E 3	10.062	1.89E 3
	4:BEBAN GEV	731.432	-6.14E 3	1.75E 3	4.25E 3	-10.062	-1.89E 3
	5:BEBAN ANG	-1.872	1.09E 3	-205.380	-500.649	1.261	5.177
	6:BEBAN ANG	-5.050	-0.067	0.005	0.004	-0.003	13.202
	7:KOM.1 (1.4 L	452.065	37.3E 3	-115.313	-233.475	6.476	-1.22E 3
	8:KOM.2 (1.2 L	386.616	53.3E 3	-120.631	-232.861	4.873	-1.06E 3
	9:KOM.3 (1.2 L	383.946	47E 3	-441.067	-1.02E 3	7.146	-1.05E 3
	10:KOM.4 (1.2	378.862	45.3E 3	-112.451	-220.576	5.123	-1.03E 3
	11:KOM.5 (0.9	287.617	25.7E 3	-402.738	-951.129	6.181	-775.300
	12:KOM.6 (0.9	282.534	24E 3	-74.121	-150.084	4.159	-762.459
	13:KOM.7 (1.2	386.942	45.3E 3	-112.460	-220.583	5.127	-1.06E 3
	14:KOM.8 (1.2	386.942	45.3E 3	-112.460	-220.583	5.127	-1.06E 3
	15:KOM.9 (1.2	386.942	45.3E 3	-112.460	-220.583	5.127	-1.06E 3
	16:KOM.10 (1.	386.942	45.3E 3	-112.460	-220.583	5.127	-1.06E 3
	17:KOM.11 (0.	290.613	24E 3	-74.130	-150.091	4.163	-783.582
	18:KOM.12 (0.	290.613	24E 3	-74.130	-150.091	4.163	-783.582
	19:KOM.13 (0.	290.613	24E 3	-74.130	-150.091	4.163	-783.582
	20:KOM.14 (0.	290.613	24E 3	-74.130	-150.091	4.163	-783.582
133	1:BEBAN MAT	355.071	26.3E 3	-85.600	-185.317	12.356	-944.284
	2:BEBAN HIDL	11.729	10.8E 3	10.867	15.676	0.150	-29.045
	3:BEBAN GEV	-732.967	5.73E 3	-1.78E 3	-4.3E 3	8.406	1.9E 3



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Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 slp 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal			Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	4:BEAN GEV	732.967	-5.73E 3	1.78E 3	4.3E 3	-8.406	-1.9E 3
	5:BEAN ANG	-2.132	1.11E 3	-207.959	-606.790	1.305	5.642
	6:BEAN ANG	-5.049	-0.397	0.001	0.003	-0.004	13.204
	7:KOM.1 (1.4 E	497.099	36.8E 3	-119.840	-259.444	17.298	-1.32E 3
	8:KOM.2 (1.2 E	444.852	48.8E 3	-85.332	-197.299	15.068	-1.18E 3
	9:KOM.3 (1.2 E	434.403	44.1E 3	-424.686	-1.02E 3	17.066	-1.15E 3
	10:KOM.4 (1.2	429.736	42.3E 3	-91.851	-206.700	14.971	-1.14E 3
	11:KOM.5 (0.9	316.162	25.4E 3	-409.773	-877.650	13.209	-840.829
	12:KOM.6 (0.9	311.486	23.7E 3	-77.038	-166.781	11.114	-828.729
	13:KOM.7 (1.2	437.815	42.3E 3	-91.853	-206.705	14.977	-1.16E 3
	14:KOM.8 (1.2	437.815	42.3E 3	-91.853	-206.705	14.977	-1.16E 3
	15:KOM.9 (1.2	437.815	42.3E 3	-91.853	-206.705	14.977	-1.16E 3
	16:KOM.10 (1.:	437.815	42.3E 3	-91.853	-206.705	14.977	-1.16E 3
	17:KOM.11 (0.:	319.564	23.7E 3	-77.040	-166.785	11.120	-849.856
	18:KOM.12 (0.:	319.564	23.7E 3	-77.040	-166.785	11.120	-849.856
	19:KOM.13 (0.:	319.564	23.7E 3	-77.040	-166.785	11.120	-849.856
	20:KOM.14 (0.:	319.564	23.7E 3	-77.040	-166.785	11.120	-849.856
135	1:BEAN MAT	376.136	26.4E 3	-142.048	-296.147	21.014	-1.01E 3
	2:BEAN HIDL	22.701	13.4E 3	-13.202	-20.231	0.799	-45.921
	3:BEAN GEV	-736.316	6.21E 3	-1.79E 3	-4.34E 3	6.626	1.9E 3
	4:BEAN GEV	736.316	-6.21E 3	1.79E 3	4.34E 3	-6.626	-1.9E 3
	5:BEAN ANG	-2.526	1.1E 3	-211.149	-513.859	1.307	6.364
	6:BEAN ANG	-5.057	0.022	-0.001	0.008	-0.005	13.220
	7:KOM.1 (1.4 E	529.391	36.9E 3	-198.867	-414.606	29.420	-1.41E 3
	8:KOM.2 (1.2 E	490.084	53.1E 3	-191.581	-387.747	26.496	-1.28E 3
	9:KOM.3 (1.2 E	472.423	46.8E 3	-521.499	-1.2E 3	28.108	-1.24E 3
	10:KOM.4 (1.2	468.372	45.1E 3	-183.661	-375.596	26.008	-1.23E 3
	11:KOM.5 (0.9	336.281	25.5E 3	-465.682	-1.09E 3	21.005	-895.201
	12:KOM.6 (0.9	332.231	23.7E 3	-127.844	-266.520	18.904	-884.231
	13:KOM.7 (1.2	476.464	45.1E 3	-183.660	-375.608	26.016	-1.25E 3
	14:KOM.8 (1.2	476.464	45.1E 3	-183.660	-375.608	26.016	-1.25E 3
	15:KOM.9 (1.2	476.464	45.1E 3	-183.660	-375.608	26.016	-1.25E 3
	16:KOM.10 (1.:	476.464	45.1E 3	-183.660	-375.608	26.016	-1.28E 3
	17:KOM.11 (0.:	340.322	23.7E 3	-127.843	-266.533	18.913	-905.383
	18:KOM.12 (0.:	340.322	23.7E 3	-127.843	-266.533	18.913	-905.383
	19:KOM.13 (0.:	340.322	23.7E 3	-127.843	-266.533	18.913	-905.383
	20:KOM.14 (0.:	340.322	23.7E 3	-127.843	-266.533	18.913	-905.383
137	1:BEAN MAT	334.117	23.5E 3	-305.143	-578.940	30.492	-971.283
	2:BEAN HIDL	-5.385	13E 3	-29.275	-45.743	1.363	-6.140
	3:BEAN GEV	-772.937	4.38E 3	-1.87E 3	-4.46E 3	5.729	1.96E 3
	4:BEAN GEV	772.937	-4.38E 3	1.87E 3	4.46E 3	-5.729	-1.96E 3
	5:BEAN ANG	-6.701	805.666	-220.715	-530.194	1.276	12.632
	6:BEAN ANG	-5.124	-3.893	-0.080	-0.098	-0.007	13.320
	7:KOM.1 (1.4 E	467.764	32.9E 3	-427.201	-810.516	42.689	-1.36E 3
	8:KOM.2 (1.2 E	392.325	49E 3	-413.012	-767.917	38.771	-1.18E 3
	9:KOM.3 (1.2 E	384.835	42.5E 3	-748.591	-1.59E 3	39.985	-1.15E 3
	10:KOM.4 (1.2	387.358	41.2E 3	-395.575	-740.628	37.942	-1.15E 3
	11:KOM.5 (0.9	289.984	22.4E 3	-627.773	-1.37E 3	29.484	-853.944
	12:KOM.6 (0.9	292.507	21.1E 3	-274.757	-521.203	27.431	-852.843



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Job No	Sheet No <b>29</b>	Rev
Part	Ref	
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

## Reactions Cont...

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	13:KOM.7 (1.2	395.556	41.2E 3	-395.447	-740.471	37.953	-1.17E 3
	14:KOM.8 (1.2	395.556	41.2E 3	-395.447	-740.471	37.953	-1.17E 3
	15:KOM.9 (1.2	395.556	41.2E 3	-395.447	-740.471	37.953	-1.17E 3
	16:KOM.10 (1.2	395.556	41.2E 3	-395.447	-740.471	37.953	-1.17E 3
	17:KOM.11 (0.1	300.705	21.1E 3	-274.629	-521.046	27.443	-874.154
	18:KOM.12 (0.1	300.705	21.1E 3	-274.629	-521.046	27.443	-874.154
	19:KOM.13 (0.1	300.705	21.1E 3	-274.629	-521.046	27.443	-874.154
	20:KOM.14 (0.1	300.705	21.1E 3	-274.629	-521.046	27.443	-874.154
139	1:BEAN MAT	928.005	35.5E 3	-129.579	-385.284	40.232	-1.86E 3
	2:BEAN HIDL	18.614	10.9E 3	-66.682	-103.474	1.686	-41.648
	3:BEAN GEM	-806.613	-5.84E 3	-2.14E 3	-4.86E 3	5.697	2.01E 3
	4:BEAN GEM	806.613	5.84E 3	2.14E 3	4.86E 3	-5.697	-2.01E 3
	5:BEAN ANG	-13.343	-934.196	-256.501	-584.681	1.210	22.620
	6:BEAN ANG	-4.982	-3.321	0.009	0.045	-0.010	13.117
	7:KOM.1 (1.4 [	1.3E 3	49.7E 3	-181.411	-539.370	56.325	-2.6E 3
	8:KOM.2 (1.2 [	1.14E 3	60E 3	-262.186	-627.876	50.977	-2.3E 3
	9:KOM.3 (1.2 [	1.11E 3	52E 3	-632.578	-1.5E 3	51.901	-2.24E 3
	10:KOM.4 (1.2	1.12E 3	53.4E 3	-222.163	-565.717	49.949	-2.25E 3
	11:KOM.5 (0.9	813.856	30.4E 3	-527.022	-1.28E 3	38.145	-1.64E 3
	12:KOM.6 (0.9	827.234	31.9E 3	-116.607	-346.664	36.193	-1.65E 3
	13:KOM.7 (1.2	1.13E 3	53.4E 3	-222.177	-565.791	49.965	-2.27E 3
	14:KOM.8 (1.2	1.13E 3	53.4E 3	-222.177	-565.791	49.965	-2.27E 3
	15:KOM.9 (1.2	1.13E 3	53.4E 3	-222.177	-565.791	49.965	-2.27E 3
	16:KOM.10 (1.2	1.13E 3	53.4E 3	-222.177	-565.791	49.965	-2.27E 3
	17:KOM.11 (0.1	835.204	31.9E 3	-116.621	-346.738	36.209	-1.67E 3
	18:KOM.12 (0.1	835.204	31.9E 3	-116.621	-346.738	36.209	-1.67E 3
	19:KOM.13 (0.1	835.204	31.9E 3	-116.621	-346.738	36.209	-1.67E 3
	20:KOM.14 (0.1	835.204	31.9E 3	-116.621	-346.738	36.209	-1.67E 3
141	1:BEAN MAT	650.317	40.3E 3	-387.265	-776.094	25.978	-1.21E 3
	2:BEAN HIDL	2.838	10.9E 3	49.962	64.546	0.197	-9.235
	3:BEAN GEM	-577.783	6.38E 3	-2.14E 3	-4.86E 3	4.471	1.61E 3
	4:BEAN GEM	577.783	-6.38E 3	2.14E 3	4.86E 3	-4.471	-1.61E 3
	5:BEAN ANG	15.150	1.2E 3	-256.333	-584.764	0.944	-26.545
	6:BEAN ANG	-4.985	-3.415	0.080	0.155	-0.001	13.135
	7:KOM.1 (1.4 [	910.444	56.4E 3	-542.171	-1.09E 3	36.369	-1.69E 3
	8:KOM.2 (1.2 [	784.921	65.9E 3	-384.778	-828.039	31.488	-1.46E 3
	9:KOM.3 (1.2 [	807.459	61.2E 3	-824.888	-1.8E 3	32.880	-1.5E 3
	10:KOM.4 (1.2	775.242	55.3E 3	-414.628	-866.519	31.369	-1.44E 3
	11:KOM.5 (0.9	609.526	38.2E 3	-758.671	-1.63E 3	24.890	-1.13E 3
	12:KOM.6 (0.9	577.309	36.3E 3	-348.411	-698.237	23.379	-1.06E 3
	13:KOM.7 (1.2	783.219	59.3E 3	-414.756	-866.766	31.370	-1.46E 3
	14:KOM.8 (1.2	783.219	59.3E 3	-414.756	-866.766	31.370	-1.46E 3
	15:KOM.9 (1.2	783.219	59.3E 3	-414.756	-866.766	31.370	-1.46E 3
	16:KOM.10 (1.2	783.219	59.3E 3	-414.756	-866.766	31.370	-1.46E 3
	17:KOM.11 (0.1	585.286	36.3E 3	-348.539	-698.484	23.380	-1.09E 3
	18:KOM.12 (0.1	585.286	36.3E 3	-348.539	-698.484	23.380	-1.09E 3
	19:KOM.13 (0.1	585.286	36.3E 3	-348.539	-698.484	23.380	-1.09E 3
	20:KOM.14 (0.1	585.286	36.3E 3	-348.539	-698.484	23.380	-1.09E 3
143	1:BEAN MAT	126.207	23.3E 3	-50.138	-242.588	21.934	-454.958





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Part		
Ref		
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## Reactions Cont...

Node	L/C	Horizontal		FZ (kg)	Moment		
		FX (kg)	FY (kg)		MX (kg·m)	MY (kg·m)	MZ (kg·m)
	2:BEAN HIDL	-14.023	13E 3	15.551	13.835	-0.058	14.452
	3:BEAN GEV	-658.322	-5.24E 3	-1.85E 3	-4.43E 3	6.488	1.73E 3
	4:BEAN GEV	658.322	5.24E 3	1.85E 3	4.43E 3	-6.488	-1.73E 3
	5:BEAN ANG	8.716	-546.015	-221.009	-530.892	1.321	-16.681
	6:BEAN ANG	-5.148	-3.891	0.148	0.249	-0.003	13.368
	7:KOM.1 (1.4 I	176.690	32.6E 3	-70.194	-339.623	30.708	-636.941
	8:KOM.2 (1.2 I	129.013	48.7E 3	-35.284	-268.970	26.228	-522.827
	9:KOM.3 (1.2 I	151.371	40E 3	-398.230	-1.13E 3	28.377	-558.188
	10:KOM.4 (1.2	129.189	40.9E 3	-44.378	-276.872	26.258	-510.109
	11:KOM.5 (0.9	127.532	20.1E 3	-398.739	-1.07E 3	21.855	-436.152
	12:KOM.6 (0.9	105.350	20.9E 3	-44.887	-217.930	19.736	-388.073
	13:KOM.7 (1.2	137.426	40.9E 3	-44.615	-277.271	26.263	-531.498
	14:KOM.8 (1.2	137.426	40.9E 3	-44.615	-277.271	26.263	-531.498
	15:KOM.9 (1.2	137.426	40.9E 3	-44.615	-277.271	26.263	-531.498
	16:KOM.10 (1.	137.426	40.9E 3	-44.615	-277.271	26.263	-531.498
	17:KOM.11 (0.1	113.586	20.9E 3	-45.124	-218.329	19.741	-409.462
	18:KOM.12 (0.1	113.586	20.9E 3	-45.124	-218.329	19.741	-409.462
	19:KOM.13 (0.1	113.586	20.9E 3	-45.124	-218.329	19.741	-409.462
	20:KOM.14 (0.1	113.586	20.9E 3	-45.124	-218.329	19.741	-409.462
145	1:BEAN MAT	188.403	25.7E 3	-104.385	-287.693	17.002	-548.855
	2:BEAN HIDL	14.066	13.4E 3	2.676	-5.285	-0.005	-26.852
	3:BEAN GEV	-681.994	-6.26E 3	-1.79E 3	-4.33E 3	7.739	1.77E 3
	4:BEAN GEV	681.994	6.26E 3	1.79E 3	4.33E 3	-7.739	-1.77E 3
	5:BEAN ANG	4.193	-842.840	-211.137	-513.806	1.452	-9.610
	6:BEAN ANG	-5.077	0.057	0.043	0.088	-0.004	13.264
	7:KOM.1 (1.4 I	263.764	35.9E 3	-146.139	-402.770	23.803	-768.396
	8:KOM.2 (1.2 I	248.590	52.2E 3	-120.980	-353.687	20.394	-701.269
	9:KOM.3 (1.2 I	246.858	42.8E 3	-460.405	-1.17E 3	22.720	-700.653
	10:KOM.4 (1.2	232.026	44.2E 3	-122.517	-350.375	20.391	-684.056
	11:KOM.5 (0.9	176.271	21.7E 3	-431.766	-1.08E 3	17.625	-509.346
	12:KOM.6 (0.9	161.439	23.1E 3	-93.878	-258.782	15.295	-472.747
	13:KOM.7 (1.2	240.150	44.2E 3	-122.585	-350.516	20.397	-685.278
	14:KOM.8 (1.2	240.150	44.2E 3	-122.585	-350.516	20.397	-685.278
	15:KOM.9 (1.2	240.150	44.2E 3	-122.585	-350.516	20.397	-685.278
	16:KOM.10 (1.	240.150	44.2E 3	-122.585	-350.516	20.397	-685.278
	17:KOM.11 (0.1	169.563	23.1E 3	-93.946	-258.924	15.302	-493.969
	18:KOM.12 (0.1	169.563	23.1E 3	-93.946	-258.924	15.302	-493.969
	19:KOM.13 (0.1	169.563	23.1E 3	-93.946	-258.924	15.302	-493.969
	20:KOM.14 (0.1	169.563	23.1E 3	-93.946	-258.924	15.302	-493.969
147	1:BEAN MAT	188.101	25.5E 3	-98.080	-252.848	12.487	-550.863
	2:BEAN HIDL	1.527	10.7E 3	-20.100	-38.391	0.236	-8.852
	3:BEAN GEV	-684.771	-5.8E 3	-1.77E 3	-4.29E 3	9.437	1.78E 3
	4:BEAN GEV	684.771	5.8E 3	1.77E 3	4.29E 3	-9.437	-1.78E 3
	5:BEAN ANG	3.762	-840.651	-207.530	-505.791	1.429	-8.696
	6:BEAN ANG	-5.072	-0.298	0.025	0.055	-0.004	13.255
	7:KOM.1 (1.4 I	263.341	35.7E 3	-137.312	-353.988	17.482	-771.208
	8:KOM.2 (1.2 I	228.165	47.8E 3	-149.856	-364.843	15.363	-674.879
	9:KOM.3 (1.2 I	233.268	40E 3	-469.843	-1.15E 3	17.507	-683.601
	10:KOM.4 (1.2	219.133	41.3E 3	-137.755	-341.721	15.214	-648.480



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Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

## Reactions Cont...

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	11:KOM.5 (0.9	175.310	21.6E 3	-420.319	-1.04E 3	13.524	-509.690
	12:KOM.6 (0.9	161.175	23E 3	-88.231	-227.476	11.231	-474.569
	13:KOM.7 (1.2	227.248	41.3E 3	-137.796	-341.809	15.221	-669.688
	14:KOM.8 (1.2	227.248	41.3E 3	-137.796	-341.809	15.221	-669.688
	15:KOM.9 (1.2	227.248	41.3E 3	-137.796	-341.809	15.221	-669.688
	16:KOM.10 (1.	227.248	41.3E 3	-137.796	-341.809	15.221	-669.688
	17:KOM.11 (0.9	169.291	23E 3	-88.272	-227.564	11.238	-495.777
	18:KOM.12 (0.9	169.291	23E 3	-88.272	-227.564	11.238	-495.777
	19:KOM.13 (0.9	169.291	23E 3	-88.272	-227.564	11.238	-495.777
	20:KOM.14 (0.9	169.291	23E 3	-88.272	-227.564	11.238	-495.777
149	1:BEBAN MAT	193.276	25.8E 3	-64.829	-185.625	8.823	-559.478
	2:BEBAN HIDL	-6.582	13.3E 3	4.474	-1.841	0.499	2.983
	3:BEBAN GEV	-687.482	-6.16E 3	-1.75E 3	-4.23E 3	9.906	1.78E 3
	4:BEBAN GEV	687.482	6.16E 3	1.75E 3	4.23E 3	-9.906	-1.78E 3
	5:BEBAN ANG	3.425	-828.294	-204.662	-499.008	1.274	-8.133
	6:BEBAN ANG	-5.077	-0.152	0.002	0.012	-0.004	13.261
	7:KOM.1 (1.4 I	270.587	36.1E 3	-90.760	-259.875	12.353	-783.269
	8:KOM.2 (1.2 I	221.400	52.2E 3	-70.637	-225.696	11.386	-666.601
	9:KOM.3 (1.2 I	230.829	42.9E 3	-400.780	-1.02E 3	13.124	-681.403
	10:KOM.4 (1.2	217.227	44.2E 3	-73.317	-224.573	11.080	-647.173
	11:KOM.5 (0.9	179.428	21.9E 3	-385.805	-965.475	9.979	-516.543
	12:KOM.6 (0.9	165.826	23.2E 3	-58.342	-167.044	7.934	-482.312
	13:KOM.7 (1.2	225.349	44.2E 3	-73.321	-224.591	11.087	-668.391
	14:KOM.8 (1.2	225.349	44.2E 3	-73.321	-224.591	11.087	-668.391
	15:KOM.9 (1.2	225.349	44.2E 3	-73.321	-224.591	11.087	-668.391
	16:KOM.10 (1.	225.349	44.2E 3	-73.321	-224.591	11.087	-668.391
	17:KOM.11 (0.9	173.948	23.2E 3	-58.346	-167.063	7.941	-503.530
	18:KOM.12 (0.9	173.948	23.2E 3	-58.346	-167.063	7.941	-503.530
	19:KOM.13 (0.9	173.948	23.2E 3	-58.346	-167.063	7.941	-503.530
	20:KOM.14 (0.9	173.948	23.2E 3	-58.346	-167.063	7.941	-503.530
151	1:BEBAN MAT	181.118	25.6E 3	-33.321	-126.652	5.869	-541.960
	2:BEBAN HIDL	10.391	13.1E 3	24.232	28.211	0.621	-21.884
	3:BEBAN GEV	-725.808	-3.98E 3	-1.8E 3	-4.29E 3	8.388	1.84E 3
	4:BEBAN GEV	725.808	3.98E 3	1.8E 3	4.29E 3	-8.388	-1.84E 3
	5:BEBAN ANG	-0.898	-505.694	-209.144	-503.315	0.985	-2.036
	6:BEBAN ANG	-5.120	4.175	-0.119	-0.174	-0.003	13.324
	7:KOM.1 (1.4 I	253.566	35.9E 3	-46.650	-177.313	8.217	-758.744
	8:KOM.2 (1.2 I	233.968	51.7E 3	-1.214	-106.845	8.036	-685.366
	9:KOM.3 (1.2 I	226.297	43E 3	-350.383	-929.076	9.239	-675.494
	10:KOM.4 (1.2	219.541	43.8E 3	-15.944	-124.050	7.659	-650.918
	11:KOM.5 (0.9	161.571	22.2E 3	-364.619	-919.291	6.857	-491.022
	12:KOM.6 (0.9	154.814	23.1E 3	-30.180	-114.265	5.277	-466.446
	13:KOM.7 (1.2	227.733	43.8E 3	-15.753	-123.772	7.664	-672.236
	14:KOM.8 (1.2	227.733	43.8E 3	-15.753	-123.772	7.664	-672.236
	15:KOM.9 (1.2	227.733	43.8E 3	-15.753	-123.772	7.664	-672.236
	16:KOM.10 (1.	227.733	43.8E 3	-15.753	-123.772	7.664	-672.236
	17:KOM.11 (0.9	163.007	23.1E 3	-29.989	-113.987	5.282	-487.764
	18:KOM.12 (0.9	163.007	23.1E 3	-29.989	-113.987	5.282	-487.764
	19:KOM.13 (0.9	163.007	23.1E 3	-29.989	-113.987	5.282	-487.764



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Part		
Ref		
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Client	File: endhik-2 sip 4.std	Date/Time: 21-Mar-2011 15:52

### Reactions Cont...

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg*m)	MY (kg*m)	MZ (kg*m)
	20:KOM.14 (0.1	163.007	23.1E 3	-29.989	-113.987	5.282	-487.764
153	1:BEAN MAT	140.265	33E 3	-53.913	-147.712	3.360	-481.111
	2:BEAN HIDL	5.335	10.9E 3	50.957	68.698	0.462	-14.978
	3:BEAN GEV	-677.591	5.34E 3	-1.98E 3	-4.54E 3	4.533	1.77E 3
	4:BEAN GEV	677.591	-5.34E 3	1.98E 3	4.54E 3	-4.533	-1.77E 3
	5:BEAN ANG	1.714	1.02E 3	-232.269	-535.416	0.516	-8.041
	6:BEAN ANG	-4.936	1.867	-0.038	-0.063	-0.002	13.057
	7:KOM.1 (1.4 I	196.371	46.2E 3	-75.478	-206.797	4.704	-673.555
	8:KOM.2 (1.2 I	176.854	57.1E 3	16.835	-67.338	4.771	-601.298
	9:KOM.3 (1.2 I	176.396	52.2E 3	-385.369	-965.222	5.319	-601.977
	10:KOM.4 (1.2	165.756	50.6E 3	-13.800	-108.657	4.490	-571.420
	11:KOM.5 (0.9	128.981	31.4E 3	-420.152	-989.606	3.850	-442.665
	12:KOM.6 (0.9	118.341	29.7E 3	-48.583	-133.042	3.021	-412.109
	13:KOM.7 (1.2	173.653	50.6E 3	-13.739	-108.557	4.493	-592.311
	14:KOM.8 (1.2	173.653	50.6E 3	-13.739	-108.557	4.493	-592.311
	15:KOM.9 (1.2	173.653	50.6E 3	-13.739	-108.557	4.493	-592.311
	16:KOM.10 (1.:	173.653	50.6E 3	-13.739	-108.557	4.493	-592.311
	17:KOM.11 (0.1	126.238	29.7E 3	-48.521	-132.941	3.024	-433.000
	18:KOM.12 (0.1	126.238	29.7E 3	-48.521	-132.941	3.024	-433.000
	19:KOM.13 (0.1	126.238	29.7E 3	-48.521	-132.941	3.024	-433.000
	20:KOM.14 (0.1	126.238	29.7E 3	-48.521	-132.941	3.024	-433.000
155	1:BEAN MAT	-105.016	72.7E 3	-499.750	-803.139	1.127	-131.074
	2:BEAN HIDL	-5.555	11.2E 3	45.962	61.468	0.105	0.101
	3:BEAN GEV	-697.979	9.28E 3	-2.03E 3	-4.6E 3	0.973	1.8E 3
	4:BEAN GEV	697.979	-9.28E 3	2.03E 3	4.6E 3	-0.973	-1.8E 3
	5:BEAN ANG	-1.498	1.78E 3	-239.710	-545.806	0.159	-0.946
	6:BEAN ANG	-4.881	-2.015	0.002	-0.010	-0.001	12.979
	7:KOM.1 (1.4 I	-147.023	102E 3	-699.650	-1.12E 3	1.578	-183.504
	8:KOM.2 (1.2 I	-134.908	105E 3	-526.161	-865.417	1.520	-157.128
	9:KOM.3 (1.2 I	-133.972	101E 3	-937.274	-1.78E 3	1.711	-158.701
	10:KOM.4 (1.2	-139.385	98.4E 3	-553.735	-902.314	1.455	-136.422
	11:KOM.5 (0.9	-96.912	68.3E 3	-833.311	-1.6E 3	1.269	-119.480
	12:KOM.6 (0.9	-102.325	65.4E 3	-449.771	-722.841	1.013	-97.201
	13:KOM.7 (1.2	-131.575	98.4E 3	-553.738	-902.298	1.457	-157.188
	14:KOM.8 (1.2	-131.575	98.4E 3	-553.738	-902.298	1.457	-157.188
	15:KOM.9 (1.2	-131.575	98.4E 3	-553.738	-902.298	1.457	-157.188
	16:KOM.10 (1.:	-131.575	98.4E 3	-553.738	-902.298	1.457	-157.188
	17:KOM.11 (0.1	-94.515	65.4E 3	-449.775	-722.825	1.014	-117.967
	18:KOM.12 (0.1	-94.515	65.4E 3	-449.775	-722.825	1.014	-117.967
	19:KOM.13 (0.1	-94.515	65.4E 3	-449.775	-722.825	1.014	-117.967
	20:KOM.14 (0.1	-94.515	65.4E 3	-449.775	-722.825	1.014	-117.967
157	1:BEAN MAT	497.033	86.9E 3	-1.38E 3	-2.09E 3	-5.162	-1.02E 3
	2:BEAN HIDL	6.751	13.1E 3	-10.816	-21.442	-0.331	-18.772
	3:BEAN GEV	-602.592	3.34E 3	-1.98E 3	-4.54E 3	-1.399	1.66E 3
	4:BEAN GEV	602.592	-3.34E 3	1.98E 3	4.54E 3	1.399	-1.66E 3
	5:BEAN ANG	16.319	1.11E 3	-240.990	-547.538	-0.015	-26.597
	6:BEAN ANG	-5.179	-14.481	0.368	0.521	-0.003	13.411
	7:KOM.1 (1.4 I	695.846	122E 3	-1.93E 3	-2.92E 3	-7.227	-1.42E 3
	8:KOM.2 (1.2 I	607.241	125E 3	-1.67E 3	-2.54E 3	-6.724	-1.25E 3



Software licensed to Snow Parther [L20]

Job No	Sheet No <b>33</b>	Rev
Part		
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**Reactions Cont...**

Node	L/C	Horizontal		Vertical		Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg'm)	MY (kg'm)	MZ (kg'm)	
	9:KOM.3 (1.2 I	629.300	119E 3	-2.05E 3	-3.4E 3	-6.550	-1.28E 3	
	10:KOM.4 (1.2	594.904	117E 3	-1.66E 3	-2.52E 3	-6.530	-1.22E 3	
	11:KOM.5 (0.9	473.440	80E 3	-1.63E 3	-2.75E 3	-4.670	-956.468	
	12:KOM.6 (0.9	439.044	78.2E 3	-1.24E 3	-1.88E 3	-4.650	-892.455	
	13:KOM.7 (1.2	603.190	117E 3	-1.67E 3	-2.52E 3	-6.526	-1.24E 3	
	14:KOM.8 (1.2	603.190	117E 3	-1.67E 3	-2.52E 3	-6.526	-1.24E 3	
	15:KOM.9 (1.2	603.190	117E 3	-1.67E 3	-2.52E 3	-6.526	-1.24E 3	
	16:KOM.10 (1.:	603.190	117E 3	-1.67E 3	-2.52E 3	-6.526	-1.24E 3	
	17:KOM.11 (0.!	447.330	78.2E 3	-1.24E 3	-1.88E 3	-4.646	-913.913	
	18:KOM.12 (0.!	447.330	78.2E 3	-1.24E 3	-1.88E 3	-4.646	-913.913	
	19:KOM.13 (0.!	447.330	78.2E 3	-1.24E 3	-1.88E 3	-4.646	-913.913	
	20:KOM.14 (0.!	447.330	78.2E 3	-1.24E 3	-1.88E 3	-4.646	-913.913	
159	1:BEAN MAT	306.846	31.6E 3	-228.543	-424.079	-10.962	-753.254	
	2:BEAN HIDL	15.598	13.1E 3	2.143	-3.838	-0.554	-32.573	
	3:BEAN GEN	-670.600	-5.07E 3	-1.75E 3	-4.21E 3	-3.182	1.76E 3	
	4:BEAN GEN	670.600	5.07E 3	1.75E 3	4.21E 3	3.182	-1.76E 3	
	5:BEAN ANG	6.304	-603.276	-205.409	-496.172	-0.253	-11.938	
	6:BEAN ANG	-5.103	-1.554	0.025	0.013	-0.004	13.302	
	7:KOM.1 (1.4 I	429.584	44.3E 3	-319.960	-593.710	-15.346	-1.05E 3	
	8:KOM.2 (1.2 I	393.173	58.8E 3	-270.823	-515.036	-14.041	-956.022	
	9:KOM.3 (1.2 I	393.901	50E 3	-600.763	-1.31E 3	-14.113	-955.580	
	10:KOM.4 (1.2	375.649	51E 3	-272.068	-512.712	-13.715	-915.195	
	11:KOM.5 (0.9	286.249	27.5E 3	-534.343	-1.18E 3	-10.270	-697.030	
	12:KOM.6 (0.9	267.997	28.4E 3	-205.648	-381.650	-9.872	-656.646	
	13:KOM.7 (1.2	383.814	51E 3	-272.109	-512.733	-13.708	-936.478	
	14:KOM.8 (1.2	383.814	51E 3	-272.109	-512.733	-13.708	-936.478	
	15:KOM.9 (1.2	383.814	51E 3	-272.109	-512.733	-13.708	-936.478	
	16:KOM.10 (1.:	383.814	51E 3	-272.109	-512.733	-13.708	-936.478	
	17:KOM.11 (0.!	276.161	28.4E 3	-205.689	-381.671	-9.866	-677.929	
	18:KOM.12 (0.!	276.161	28.4E 3	-205.689	-381.671	-9.866	-677.929	
	19:KOM.13 (0.!	276.161	28.4E 3	-205.689	-381.671	-9.866	-677.929	
	20:KOM.14 (0.!	276.161	28.4E 3	-205.689	-381.671	-9.866	-677.929	
161	1:BEAN MAT	217.843	24.1E 3	-97.504	-257.360	-16.752	-636.257	
	2:BEAN HIDL	12.665	10.5E 3	-26.313	-46.420	-0.502	-29.061	
	3:BEAN GEN	-693.929	-5.64E 3	-1.73E 3	-4.18E 3	-2.001	1.8E 3	
	4:BEAN GEN	693.929	5.64E 3	1.73E 3	4.18E 3	2.001	-1.8E 3	
	5:BEAN ANG	2.718	-827.771	-201.234	-490.792	-0.325	-6.415	
	6:BEAN ANG	-5.085	0.528	-0.031	-0.078	-0.005	13.278	
	7:KOM.1 (1.4 I	304.980	33.8E 3	-136.506	-360.304	-23.453	-890.759	
	8:KOM.2 (1.2 I	281.675	45.8E 3	-159.107	-383.103	-20.905	-810.006	
	9:KOM.3 (1.2 I	278.425	38.2E 3	-465.293	-1.14E 3	-21.124	-802.833	
	10:KOM.4 (1.2	265.940	39.5E 3	-143.367	-355.376	-20.612	-771.325	
	11:KOM.5 (0.9	200.408	20.4E 3	-409.728	-1.02E 3	-15.596	-582.895	
	12:KOM.6 (0.9	187.922	21.7E 3	-87.803	-231.749	-15.084	-551.387	
	13:KOM.7 (1.2	274.076	39.5E 3	-143.319	-355.252	-20.604	-792.569	
	14:KOM.8 (1.2	274.076	39.5E 3	-143.319	-355.252	-20.604	-792.569	
	15:KOM.9 (1.2	274.076	39.5E 3	-143.319	-355.252	-20.604	-792.569	
	16:KOM.10 (1.:	274.076	39.5E 3	-143.319	-355.252	-20.604	-792.569	
	17:KOM.11 (0.!	196.058	21.7E 3	-87.754	-231.624	-15.077	-572.631	



Software licensed to Snow Parther (LZO)

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Rev

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Part

Ref

By

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Chd

Client

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**Reactions Cont...**

Node	L/C	Horizontal			Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	18:KOM.12 (0.!	196.058	21.7E 3	-87.754	-231.624	-15.077	-572.631
	19:KOM.13 (0.!	196.058	21.7E 3	-87.754	-231.624	-15.077	-572.631
	20:KOM.14 (0.!	196.058	21.7E 3	-87.754	-231.624	-15.077	-572.631
163	1:BEBAN MAT	250.296	25.4E 3	-74.102	-257.656	-22.867	-694.730
	2:BEBAN HIDL	6.089	13.5E 3	20.763	21.508	-0.394	-20.137
	3:BEBAN GEV	-709.346	-6.03E 3	-1.74E 3	-4.2E 3	0.033	1.82E 3
	4:BEBAN GEV	709.346	6.03E 3	1.74E 3	4.2E 3	-0.033	-1.82E 3
	5:BEBAN ANG	1.874	-815.143	-203.084	-494.222	-0.269	-4.675
	6:BEBAN ANG	-5.156	-0.018	-0.050	-0.115	-0.004	13.381
	7:KOM.1 (1.4 E	350.415	35.5E 3	-103.743	-360.718	-32.014	-972.621
	8:KOM.2 (1.2 E	310.098	52E 3	-55.702	-274.774	-28.071	-865.894
	9:KOM.3 (1.2 E	309.442	42.6E 3	-393.094	-1.08E 3	-28.266	-861.293
	10:KOM.4 (1.2	298.195	43.9E 3	-68.239	-287.863	-27.842	-832.403
	11:KOM.5 (0.9	228.265	21.5E 3	-391.626	-1.02E 3	-21.011	-532.737
	12:KOM.6 (0.9	217.018	22.9E 3	-66.772	-232.074	-20.588	-603.847
	13:KOM.7 (1.2	306.445	43.9E 3	-68.160	-287.679	-27.835	-853.812
	14:KOM.8 (1.2	306.445	43.9E 3	-68.160	-287.679	-27.835	-853.812
	15:KOM.9 (1.2	306.445	43.9E 3	-68.160	-287.679	-27.835	-853.812
	16:KOM.10 (1.:	306.445	43.9E 3	-68.160	-287.679	-27.835	-853.812
	17:KOM.11 (0.!	225.267	22.9E 3	-66.692	-231.890	-20.580	-625.257
	18:KOM.12 (0.!	225.267	22.9E 3	-66.692	-231.890	-20.580	-625.257
	19:KOM.13 (0.!	225.267	22.9E 3	-66.692	-231.890	-20.580	-625.257
	20:KOM.14 (0.!	225.267	22.9E 3	-66.692	-231.890	-20.580	-625.257
165	1:BEBAN MAT	299.022	22.5E 3	-28.831	-237.357	-29.327	-776.684
	2:BEBAN HIDL	17.424	13.1E 3	34.373	40.900	-0.421	-37.267
	3:BEBAN GEV	-749.103	-4.26E 3	-1.8E 3	-4.3E 3	2.132	1.89E 3
	4:BEBAN GEV	749.103	4.26E 3	1.8E 3	4.3E 3	-2.132	-1.89E 3
	5:BEBAN ANG	-2.487	-524.189	-210.525	-505.634	-0.092	2.347
	6:BEBAN ANG	-5.226	3.331	-0.177	-0.310	-0.003	13.485
	7:KOM.1 (1.4 E	418.631	31.5E 3	-40.363	-332.299	-41.058	-1.09E 3
	8:KOM.2 (1.2 E	386.704	48E 3	20.399	-219.388	-35.866	-991.649
	9:KOM.3 (1.2 E	372.270	39.3E 3	-337.064	-1.05E 3	-35.761	-965.534
	10:KOM.4 (1.2	367.888	40.1E 3	-0.508	-244.424	-35.619	-947.712
	11:KOM.5 (0.9	265.140	19.4E 3	-362.788	-1.02E 3	-26.542	-695.261
	12:KOM.6 (0.9	260.758	20.3E 3	-26.232	-214.117	-26.400	-677.440
	13:KOM.7 (1.2	376.250	40.1E 3	-0.224	-243.928	-35.614	-969.289
	14:KOM.8 (1.2	376.250	40.1E 3	-0.224	-243.928	-35.614	-969.289
	15:KOM.9 (1.2	376.250	40.1E 3	-0.224	-243.928	-35.614	-969.289
	16:KOM.10 (1.:	376.250	40.1E 3	-0.224	-243.928	-35.614	-969.289
	17:KOM.11 (0.!	269.120	20.3E 3	-25.948	-213.621	-26.395	-699.016
	18:KOM.12 (0.!	269.120	20.3E 3	-25.948	-213.621	-26.395	-699.016
	19:KOM.13 (0.!	269.120	20.3E 3	-25.948	-213.621	-26.395	-699.016
	20:KOM.14 (0.!	269.120	20.3E 3	-25.948	-213.621	-26.395	-699.016
333	1:BEBAN MAT	427.617	84E 3	-2.49E 3	-3.78E 3	39.807	-889.677
	2:BEBAN HIDL	-47.881	8.84E 3	-105.626	-149.463	1.069	64.388
	3:BEBAN GEV	-664.690	4.47E 3	-2.25E 3	-5.03E 3	5.285	1.74E 3
	4:BEBAN GEV	664.690	-4.47E 3	2.25E 3	5.03E 3	-5.285	-1.74E 3
	5:BEBAN ANG	-2.951	463.734	-262.329	-594.486	0.764	-0.825
	6:BEBAN ANG	-4.422	2.575	-0.564	-0.779	0.013	12.321



Software licensed to Snow Panther [LZO]

Job No

Sheet No

35

Rev

Part

Job Title

Ref

By

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Chd

Client

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Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg'm)	MY (kg'm)	MZ (kg'm)
	7:KOM.1 (1.4 [	598.664	118E 3	-3.49E 3	-5.3E 3	55.730	-1.25E 3
	8:KOM.2 (1.2 [	436.531	115E 3	-3.16E 3	-4.78E 3	49.479	-984.592
	9:KOM.3 (1.2 [	460.538	110E 3	-3.51E 3	-5.64E 3	50.060	-1E 3
	10:KOM.4 (1.2	458.184	110E 3	-3.09E 3	-4.69E 3	48.858	-983.511
	11:KOM.5 (0.9	380.134	76.3E 3	-2.66E 3	-4.36E 3	37.049	-802.029
	12:KOM.6 (0.9	377.780	75.6E 3	-2.24E 3	-3.41E 3	35.847	-780.955
	13:KOM.7 (1.2	465.259	110E 3	-3.09E 3	-4.69E 3	48.837	-1E 3
	14:KOM.8 (1.2	465.259	110E 3	-3.09E 3	-4.69E 3	48.837	-1E 3
	15:KOM.9 (1.2	465.259	110E 3	-3.09E 3	-4.69E 3	48.837	-1E 3
	16:KOM.10 (1.;	465.259	110E 3	-3.09E 3	-4.69E 3	48.837	-1E 3
	17:KOM.11 (0.!	384.855	75.6E 3	-2.24E 3	-3.4E 3	35.827	-800.709
	18:KOM.12 (0.!	384.855	75.6E 3	-2.24E 3	-3.4E 3	35.827	-800.709
	19:KOM.13 (0.!	384.855	75.6E 3	-2.24E 3	-3.4E 3	35.827	-800.709
	20:KOM.14 (0.!	384.855	75.6E 3	-2.24E 3	-3.4E 3	35.827	-800.709
1040	1:BEBA MAT	55.933	39.2E 3	3.538	-4.974	-2.987	-372.498
	2:BEBA HIDL	-1.435	1.92E 3	4.695	20.778	0.514	3.910
	3:BEBA GEM	-49.575	-560.496	-141.769	-1E 3	-4.253	375.147
	4:BEBA GEM	49.575	560.496	141.769	1E 3	4.253	-375.147
	5:BEBA ANG	1.613	-113.222	-21.666	-153.346	-0.182	-8.022
	6:BEBA ANG	-0.522	-1.924	-0.006	-0.033	-0.002	3.772
	7:KOM.1 (1.4 [	78.307	54.9E 3	4.953	-6.964	-4.182	-521.498
	8:KOM.2 (1.2 [	64.824	50.1E 3	11.757	27.275	-2.761	-440.742
	9:KOM.3 (1.2 [	68.265	48.8E 3	-25.725	-230.545	-3.361	-455.923
	10:KOM.4 (1.2	64.850	48.9E 3	8.931	14.756	-3.073	-437.053
	11:KOM.5 (0.9	52.921	35.1E 3	-31.481	-249.831	-2.979	-348.084
	12:KOM.6 (0.9	49.505	35.3E 3	3.174	-4.529	-2.691	-329.214
	13:KOM.7 (1.2	65.685	48.9E 3	8.940	14.809	-3.070	-443.088
	14:KOM.8 (1.2	65.685	48.9E 3	8.940	14.809	-3.070	-443.088
	15:KOM.9 (1.2	65.685	48.9E 3	8.940	14.809	-3.070	-443.088
	16:KOM.10 (1.;	65.685	48.9E 3	8.940	14.809	-3.070	-443.088
	17:KOM.11 (0.!	50.340	35.3E 3	3.184	-4.477	-2.688	-335.248
	18:KOM.12 (0.!	50.340	35.3E 3	3.184	-4.477	-2.688	-335.248
	19:KOM.13 (0.!	50.340	35.3E 3	3.184	-4.477	-2.688	-335.248
	20:KOM.14 (0.!	50.340	35.3E 3	3.184	-4.477	-2.688	-335.248
1042	1:BEBA MAT	79.017	31.9E 3	-3.635	-20.218	-0.535	-477.794
	2:BEBA HIDL	-1.551	1.63E 3	-0.908	-4.439	-0.134	4.305
	3:BEBA GEM	-64.900	101.783	-22.560	-158.227	-1.070	452.058
	4:BEBA GEM	64.900	-101.783	22.560	158.227	1.070	-452.058
	5:BEBA ANG	-1.780	89.154	-3.500	-24.504	-0.060	9.299
	6:BEBA ANG	-0.504	-1.362	0.000	0.001	-0.000	3.649
	7:KOM.1 (1.4 [	110.624	44.6E 3	-5.089	-28.306	-0.749	-668.912
	8:KOM.2 (1.2 [	92.340	40.8E 3	-5.814	-31.365	-0.857	-566.466
	9:KOM.3 (1.2 [	90.422	40E 3	-10.870	-67.907	-0.873	-554.170
	10:KOM.4 (1.2	92.464	39.8E 3	-5.269	-28.699	-0.777	-563.210
	11:KOM.5 (0.9	68.268	28.8E 3	-8.872	-57.403	-0.578	-415.137
	12:KOM.6 (0.9	70.310	28.7E 3	-3.271	-18.194	-0.482	-424.176
	13:KOM.7 (1.2	93.270	39.8E 3	-5.270	-28.701	-0.777	-569.048
	14:KOM.8 (1.2	93.270	39.8E 3	-5.270	-28.701	-0.777	-569.048
	15:KOM.9 (1.2	93.270	39.8E 3	-5.270	-28.701	-0.777	-569.048



Software licensed to Snow Panther [L20]

Job No	Sheet No <b>36</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

### Reactions Cont...

Node	L/C	Horizontal		FZ (kg)	Moment		
		FX (kg)	FY (kg)		MX (kg·m)	MY (kg·m)	MZ (kg·m)
	16:KOM.10 (1.:	93.270	39.8E 3	-5.270	-28.701	-0.777	-569.048
	17:KOM.11 (0.:	71.116	28.7E 3	-3.271	-18.197	-0.482	-430.015
	18:KOM.12 (0.:	71.116	28.7E 3	-3.271	-18.197	-0.482	-430.015
	19:KOM.13 (0.:	71.116	28.7E 3	-3.271	-18.197	-0.482	-430.015
	20:KOM.14 (0.:	71.116	28.7E 3	-3.271	-18.197	-0.482	-430.015
1075	1:BEBA MAT	-16.9E 3	29.7E 3	11.7E 3	0.000	0.000	0.000
	2:BEBA HIDI	-617.495	1.06E 3	431.728	0.000	0.000	0.000
	3:BEBA GEM	7.1E 3	-8.25E 3	-4.02E 3	0.000	0.000	0.000
	4:BEBA GEM	-7.1E 3	8.25E 3	4.02E 3	0.000	0.000	0.000
	5:BEBA ANG	895.969	-1.03E 3	-497.596	0.000	0.000	0.000
	6:BEBA ANG	-2.284	1.907	0.913	0.000	0.000	0.000
	7:KOM.1 (1.4 [	-23.7E 3	41.6E 3	16.4E 3	0.000	0.000	0.000
	8:KOM.2 (1.2 [	-21.3E 3	37.3E 3	14.7E 3	0.000	0.000	0.000
	9:KOM.3 (1.2 [	-19.5E 3	35E 3	13.7E 3	0.000	0.000	0.000
	10:KOM.4 (1.2	-20.9E 3	36.7E 3	14.5E 3	0.000	0.000	0.000
	11:KOM.5 (0.9	-13.8E 3	25.1E 3	9.72E 3	0.000	0.000	0.000
	12:KOM.6 (0.9	-15.2E 3	26.7E 3	10.5E 3	0.000	0.000	0.000
	13:KOM.7 (1.2	-20.9E 3	36.7E 3	14.5E 3	0.000	0.000	0.000
	14:KOM.8 (1.2	-20.9E 3	36.7E 3	14.5E 3	0.000	0.000	0.000
	15:KOM.9 (1.2	-20.9E 3	36.7E 3	14.5E 3	0.000	0.000	0.000
	16:KOM.10 (1.:	-20.9E 3	36.7E 3	14.5E 3	0.000	0.000	0.000
	17:KOM.11 (0.:	-15.2E 3	26.7E 3	10.5E 3	0.000	0.000	0.000
	18:KOM.12 (0.:	-15.2E 3	26.7E 3	10.5E 3	0.000	0.000	0.000
	19:KOM.13 (0.:	-15.2E 3	26.7E 3	10.5E 3	0.000	0.000	0.000
	20:KOM.14 (0.:	-15.2E 3	26.7E 3	10.5E 3	0.000	0.000	0.000
1077	1:BEBA MAT	-38.5E 3	43E 3	-10.5E 3	0.000	0.000	0.000
	2:BEBA HIDI	-1.39E 3	1.51E 3	-347.047	0.000	0.000	0.000
	3:BEBA GEM	-6.3E 3	7.3E 3	-3.78E 3	0.000	0.000	0.000
	4:BEBA GEM	6.3E 3	-7.3E 3	3.78E 3	0.000	0.000	0.000
	5:BEBA ANG	-706.312	845.920	-451.234	0.000	0.000	0.000
	6:BEBA ANG	-3.401	2.183	-0.169	0.000	0.000	0.000
	7:KOM.1 (1.4 [	-53.9E 3	60.2E 3	-14.7E 3	0.000	0.000	0.000
	8:KOM.2 (1.2 [	-48.4E 3	54E 3	-13.2E 3	0.000	0.000	0.000
	9:KOM.3 (1.2 [	-48.7E 3	54.5E 3	-13.7E 3	0.000	0.000	0.000
	10:KOM.4 (1.2	-47.6E 3	53.1E 3	-13E 3	0.000	0.000	0.000
	11:KOM.5 (0.9	-35.8E 3	40.1E 3	-10.2E 3	0.000	0.000	0.000
	12:KOM.6 (0.9	-34.6E 3	38.7E 3	-9.48E 3	0.000	0.000	0.000
	13:KOM.7 (1.2	-47.6E 3	53.1E 3	-13E 3	0.000	0.000	0.000
	14:KOM.8 (1.2	-47.6E 3	53.1E 3	-13E 3	0.000	0.000	0.000
	15:KOM.9 (1.2	-47.6E 3	53.1E 3	-13E 3	0.000	0.000	0.000
	16:KOM.10 (1.:	-47.6E 3	53.1E 3	-13E 3	0.000	0.000	0.000
	17:KOM.11 (0.:	-34.6E 3	38.7E 3	-9.48E 3	0.000	0.000	0.000
	18:KOM.12 (0.:	-34.6E 3	38.7E 3	-9.48E 3	0.000	0.000	0.000
	19:KOM.13 (0.:	-34.6E 3	38.7E 3	-9.48E 3	0.000	0.000	0.000
	20:KOM.14 (0.:	-34.6E 3	38.7E 3	-9.48E 3	0.000	0.000	0.000
1078	1:BEBA MAT	-19.5E 3	32.1E 3	9.83E 3	0.000	0.000	0.000
	2:BEBA HIDI	-637.334	1.05E 3	318.824	0.000	0.000	0.000
	3:BEBA GEM	4.01E 3	-8.96E 3	-11.6E 3	0.000	0.000	0.000
	4:BEBA GEM	-4.01E 3	8.96E 3	11.6E 3	0.000	0.000	0.000



Software licensed to Snow Panther (L20)

Job No	Sheet No <b>37</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg'm)	MY (kg'm)	MZ (kg'm)
	5:BEAN ANG	614.650	-1.15E 3	-1.38E 3	0.000	0.000	0.000
	6:BEAN ANG	-8.151	5.071	0.039	0.000	0.000	0.000
	7:KOM.1 (1.4 Γ	-27.3E 3	44.9E 3	13.8E 3	0.000	0.000	0.000
	8:KOM.2 (1.2 Γ	-24.4E 3	40.2E 3	12.3E 3	0.000	0.000	0.000
	9:KOM.3 (1.2 Γ	-23E 3	37.7E 3	9.91E 3	0.000	0.000	0.000
	10:KOM.4 (1.2	-24E 3	39.6E 3	12.1E 3	0.000	0.000	0.000
	11:KOM.5 (0.9	-16.5E 3	27E 3	6.64E 3	0.000	0.000	0.000
	12:KOM.6 (0.9	-17.5E 3	28.9E 3	8.85E 3	0.000	0.000	0.000
	13:KOM.7 (1.2	-24E 3	39.6E 3	12.1E 3	0.000	0.000	0.000
	14:KOM.8 (1.2	-24E 3	39.6E 3	12.1E 3	0.000	0.000	0.000
	15:KOM.9 (1.2	-24E 3	39.6E 3	12.1E 3	0.000	0.000	0.000
	16:KOM.10 (1.;	-24E 3	39.6E 3	12.1E 3	0.000	0.000	0.000
	17:KOM.11 (0.!	-17.5E 3	28.9E 3	8.85E 3	0.000	0.000	0.000
	18:KOM.12 (0.!	-17.5E 3	28.9E 3	8.85E 3	0.000	0.000	0.000
	19:KOM.13 (0.!	-17.5E 3	28.9E 3	8.85E 3	0.000	0.000	0.000
	20:KOM.14 (0.!	-17.5E 3	28.9E 3	8.85E 3	0.000	0.000	0.000
1080	1:BEAN MAT	-1.32E 3	14.9E 3	-8.77E 3	0.000	0.000	0.000
	2:BEAN HIDL	0.221	449.694	-281.415	0.000	0.000	0.000
	3:BEAN GEN	-5.37E 3	10.2E 3	-11.9E 3	0.000	0.000	0.000
	4:BEAN GEN	5.37E 3	-10.2E 3	11.9E 3	0.000	0.000	0.000
	5:BEAN ANG	-572.481	1.13E 3	-1.37E 3	0.000	0.000	0.000
	6:BEAN ANG	-4.294	5.608	-2.690	0.000	0.000	0.000
	7:KOM.1 (1.4 Γ	-1.85E 3	20.9E 3	-12.3E 3	0.000	0.000	0.000
	8:KOM.2 (1.2 Γ	-1.59E 3	18.6E 3	-11E 3	0.000	0.000	0.000
	9:KOM.3 (1.2 Γ	-2.51E 3	20.1E 3	-13E 3	0.000	0.000	0.000
	10:KOM.4 (1.2	-1.6E 3	18.3E 3	-10.8E 3	0.000	0.000	0.000
	11:KOM.5 (0.9	-2.11E 3	15.2E 3	-10.1E 3	0.000	0.000	0.000
	12:KOM.6 (0.9	-1.2E 3	13.4E 3	-7.9E 3	0.000	0.000	0.000
	13:KOM.7 (1.2	-1.59E 3	18.3E 3	-10.8E 3	0.000	0.000	0.000
	14:KOM.8 (1.2	-1.59E 3	18.3E 3	-10.8E 3	0.000	0.000	0.000
	15:KOM.9 (1.2	-1.59E 3	18.3E 3	-10.8E 3	0.000	0.000	0.000
	16:KOM.10 (1.;	-1.59E 3	18.3E 3	-10.8E 3	0.000	0.000	0.000
	17:KOM.11 (0.!	-1.19E 3	13.4E 3	-7.89E 3	0.000	0.000	0.000
	18:KOM.12 (0.!	-1.19E 3	13.4E 3	-7.89E 3	0.000	0.000	0.000
	19:KOM.13 (0.!	-1.19E 3	13.4E 3	-7.89E 3	0.000	0.000	0.000
	20:KOM.14 (0.!	-1.19E 3	13.4E 3	-7.89E 3	0.000	0.000	0.000
1082	1:BEAN MAT	17.3E 3	30.1E 3	11.9E 3	0.000	0.000	0.000
	2:BEAN HIDL	634.234	1.08E 3	439.390	0.000	0.000	0.000
	3:BEAN GEN	-8.35E 3	-9.45E 3	-4.6E 3	0.000	0.000	0.000
	4:BEAN GEN	8.35E 3	9.45E 3	4.6E 3	0.000	0.000	0.000
	5:BEAN ANG	-968.479	-1.1E 3	-535.228	0.000	0.000	0.000
	6:BEAN ANG	-2.120	-1.787	-0.865	0.000	0.000	0.000
	7:KOM.1 (1.4 Γ	24.2E 3	42.1E 3	16.6E 3	0.000	0.000	0.000
	8:KOM.2 (1.2 Γ	21.7E 3	37.8E 3	14.9E 3	0.000	0.000	0.000
	9:KOM.3 (1.2 Γ	19.8E 3	35.4E 3	13.8E 3	0.000	0.000	0.000
	10:KOM.4 (1.2	21.4E 3	37.2E 3	14.7E 3	0.000	0.000	0.000
	11:KOM.5 (0.9	14E 3	25.3E 3	9.82E 3	0.000	0.000	0.000
	12:KOM.6 (0.9	15.5E 3	27.1E 3	10.7E 3	0.000	0.000	0.000
	13:KOM.7 (1.2	21.4E 3	37.2E 3	14.7E 3	0.000	0.000	0.000





Software licensed to Snow Panther (L20)

Job No	Sheet No <b>38</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

## Reactions Cont...

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg*m)	MY (kg*m)	MZ (kg*m)
	14:KOM.8 (1.2	21.4E 3	37.2E 3	14.7E 3	0.000	0.000	0.000
	15:KOM.9 (1.2	21.4E 3	37.2E 3	14.7E 3	0.000	0.000	0.000
	16:KOM.10 (1.	21.4E 3	37.2E 3	14.7E 3	0.000	0.000	0.000
	17:KOM.11 (0.9	15.5E 3	27.1E 3	10.7E 3	0.000	0.000	0.000
	18:KOM.12 (0.9	15.5E 3	27.1E 3	10.7E 3	0.000	0.000	0.000
	19:KOM.13 (0.9	15.5E 3	27.1E 3	10.7E 3	0.000	0.000	0.000
	20:KOM.14 (0.9	15.5E 3	27.1E 3	10.7E 3	0.000	0.000	0.000
1084	1:BEAN MAT	38.3E 3	42.8E 3	-10.5E 3	0.000	0.000	0.000
	2:BEAN HIDL	1.39E 3	1.51E 3	-344.248	0.000	0.000	0.000
	3:BEAN GEV	5.83E 3	7.24E 3	-4.03E 3	0.000	0.000	0.000
	4:BEAN GEV	-5.83E 3	-7.24E 3	4.03E 3	0.000	0.000	0.000
	5:BEAN ANG	762.118	909.617	-485.296	0.000	0.000	0.000
	6:BEAN ANG	-2.967	-1.848	0.096	0.000	0.000	0.000
	7:KOM.1 (1.4 [	53.6E 3	60E 3	-14.6E 3	0.000	0.000	0.000
	8:KOM.2 (1.2 [	48.2E 3	53.8E 3	-13.1E 3	0.000	0.000	0.000
	9:KOM.3 (1.2 [	48.6E 3	54.4E 3	-13.7E 3	0.000	0.000	0.000
	10:KOM.4 (1.2	47.4E 3	52.9E 3	-12.9E 3	0.000	0.000	0.000
	11:KOM.5 (0.9	35.7E 3	40E 3	-10.2E 3	0.000	0.000	0.000
	12:KOM.6 (0.9	34.5E 3	38.5E 3	-9.4E 3	0.000	0.000	0.000
	13:KOM.7 (1.2	47.4E 3	52.9E 3	-12.9E 3	0.000	0.000	0.000
	14:KOM.8 (1.2	47.4E 3	52.9E 3	-12.9E 3	0.000	0.000	0.000
	15:KOM.9 (1.2	47.4E 3	52.9E 3	-12.9E 3	0.000	0.000	0.000
	16:KOM.10 (1.	47.4E 3	52.9E 3	-12.9E 3	0.000	0.000	0.000
	17:KOM.11 (0.9	34.5E 3	38.5E 3	-9.41E 3	0.000	0.000	0.000
	18:KOM.12 (0.9	34.5E 3	38.5E 3	-9.41E 3	0.000	0.000	0.000
	19:KOM.13 (0.9	34.5E 3	38.5E 3	-9.41E 3	0.000	0.000	0.000
	20:KOM.14 (0.9	34.5E 3	38.5E 3	-9.41E 3	0.000	0.000	0.000
2018	1:BEAN MAT	3.76E 3	32.9E 3	101.415	3.34E 3	-171.425	-6.57E 3
	2:BEAN HIDL	137.940	1.14E 3	4.081	117.896	-5.782	-245.253
	3:BEAN GEV	-705.729	19.3E 3	-2.78E 3	-4.3E 3	-244.245	1.41E 3
	4:BEAN GEV	705.729	-19.3E 3	2.78E 3	4.3E 3	244.245	-1.41E 3
	5:BEAN ANG	-52.075	2.49E 3	-402.851	-596.002	-50.639	98.451
	6:BEAN ANG	-2.029	-11.501	4.317	5.016	1.307	4.333
	7:KOM.1 (1.4 [	5.26E 3	46.1E 3	141.981	4.67E 3	-239.995	-9.19E 3
	8:KOM.2 (1.2 [	4.73E 3	41.3E 3	126.228	4.19E 3	-214.961	-8.27E 3
	9:KOM.3 (1.2 [	4.56E 3	44.6E 3	-518.782	3.17E 3	-292.515	-7.97E 3
	10:KOM.4 (1.2	4.64E 3	40.6E 3	132.686	4.13E 3	-209.401	-8.12E 3
	11:KOM.5 (0.9	3.3E 3	33.6E 3	-553.288	2.05E 3	-235.306	-5.75E 3
	12:KOM.6 (0.9	3.38E 3	29.6E 3	98.181	3.01E 3	-152.192	-5.9E 3
	13:KOM.7 (1.2	4.65E 3	40.6E 3	125.779	4.12E 3	-211.492	-8.13E 3
	14:KOM.8 (1.2	4.65E 3	40.6E 3	125.779	4.12E 3	-211.492	-8.13E 3
	15:KOM.9 (1.2	4.65E 3	40.6E 3	125.779	4.12E 3	-211.492	-8.13E 3
	16:KOM.10 (1.	4.65E 3	40.6E 3	125.779	4.12E 3	-211.492	-8.13E 3
	17:KOM.11 (0.9	3.38E 3	29.6E 3	91.274	3E 3	-154.283	-5.91E 3
	18:KOM.12 (0.9	3.38E 3	29.6E 3	91.274	3E 3	-154.283	-5.91E 3
	19:KOM.13 (0.9	3.38E 3	29.6E 3	91.274	3E 3	-154.283	-5.91E 3
	20:KOM.14 (0.9	3.38E 3	29.6E 3	91.274	3E 3	-154.283	-5.91E 3
2019	1:BEAN MAT	3.4E 3	39.9E 3	9.15E 3	11.3E 3	172.201	-5.83E 3
	2:BEAN HIDL	118.949	1.31E 3	319.618	394.491	7.665	-210.424



Software licensed to Snow Panther [LZO]

Job No	Sheet No <b>39</b>	Rev
Part		
Ref		
By	Date 06-Oct-10	Chd
Client	File endhik-2 sip 4.std	Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg'm)	MY (kg'm)	MZ (kg'm)
	3:BEAN GEV	-968.330	-22.7E 3	-3.63E 3	-5.33E 3	379.719	2.09E 3
	4:BEAN GEV	968.330	22.7E 3	3.63E 3	5.33E 3	-379.719	-2.09E 3
	5:BEAN ANG	24.190	-2.45E 3	-421.319	-645.371	22.426	-44.827
	6:BEAN ANG	-8.632	-16.029	-0.838	0.458	1.386	18.058
	7:KOM.1 (1.4 I	4.77E 3	55.8E 3	12.8E 3	15.6E 3	241.082	-8.16E 3
	8:KOM.2 (1.2 I	4.27E 3	50E 3	11.5E 3	14.2E 3	218.906	-7.33E 3
	9:KOM.3 (1.2 I	4.24E 3	45.2E 3	10.6E 3	12.9E 3	250.189	-7.28E 3
	10:KOM.4 (1.2	4.19E 3	49.1E 3	11.3E 3	13.9E 3	216.524	-7.18E 3
	11:KOM.5 (0.9	3.1E 3	32E 3	7.56E 3	9.12E 3	190.863	-5.32E 3
	12:KOM.6 (0.9	3.05E 3	35.9E 3	8.24E 3	10.2E 3	157.199	-5.22E 3
	13:KOM.7 (1.2	4.2E 3	49.2E 3	11.3E 3	13.9E 3	214.307	-7.21E 3
	14:KOM.8 (1.2	4.2E 3	49.2E 3	11.3E 3	13.9E 3	214.307	-7.21E 3
	15:KOM.9 (1.2	4.2E 3	49.2E 3	11.3E 3	13.9E 3	214.307	-7.21E 3
	16:KOM.10 (1.	4.2E 3	49.2E 3	11.3E 3	13.9E 3	214.307	-7.21E 3
	17:KOM.11 (0.!	3.06E 3	35.9E 3	8.24E 3	10.2E 3	154.981	-5.25E 3
	18:KOM.12 (0.!	3.06E 3	35.9E 3	8.24E 3	10.2E 3	154.981	-5.25E 3
	19:KOM.13 (0.!	3.06E 3	35.9E 3	8.24E 3	10.2E 3	154.981	-5.25E 3
	20:KOM.14 (0.!	3.06E 3	35.9E 3	8.24E 3	10.2E 3	154.981	-5.25E 3
3749	1:BEAN MAT	-70.787	40.6E 3	-157.404	-287.757	3.583	-86.985
	2:BEAN HIDL	-15.540	6.04E 3	-8.772	-17.399	0.210	17.992
	3:BEAN GEV	-585.900	-4.43E 3	-1.8E 3	-4.09E 3	-2.241	1.4E 3
	4:BEAN GEV	585.900	4.43E 3	1.8E 3	4.09E 3	2.241	-1.4E 3
	5:BEAN ANG	-6.004	-804.176	-210.778	-482.165	-0.262	6.477
	6:BEAN ANG	-3.841	9.768	-0.052	-0.083	-0.005	9.724
	7:KOM.1 (1.4 I	-99.102	56.8E 3	-220.366	-402.880	5.017	-121.779
	8:KOM.2 (1.2 I	-109.808	58.4E 3	-202.920	-373.146	4.636	-75.595
	9:KOM.3 (1.2 I	-110.090	53.5E 3	-534.902	-1.13E 3	4.090	-76.027
	10:KOM.4 (1.2	-106.630	54.8E 3	-197.741	-362.839	4.502	-70.832
	11:KOM.5 (0.9	-73.315	35.2E 3	-478.908	-1.03E 3	2.805	-67.923
	12:KOM.6 (0.9	-69.855	36.5E 3	-141.748	-259.113	3.218	-62.729
	13:KOM.7 (1.2	-100.484	54.8E 3	-197.657	-362.707	4.510	-86.390
	14:KOM.8 (1.2	-100.484	54.8E 3	-197.657	-362.707	4.510	-86.390
	15:KOM.9 (1.2	-100.484	54.8E 3	-197.657	-362.707	4.510	-86.390
	16:KOM.10 (1.	-100.484	54.8E 3	-197.657	-362.707	4.510	-86.390
	17:KOM.11 (0.!	-63.708	36.5E 3	-141.664	-258.981	3.225	-78.286
	18:KOM.12 (0.!	-63.708	36.5E 3	-141.664	-258.981	3.225	-78.286
	19:KOM.13 (0.!	-63.708	36.5E 3	-141.664	-258.981	3.225	-78.286
	20:KOM.14 (0.!	-63.708	36.5E 3	-141.664	-258.981	3.225	-78.286
3750	1:BEAN MAT	-29.279	42.4E 3	23.597	-14.646	-18.580	-193.363
	2:BEAN HIDL	-13.611	6.12E 3	-1.629	-6.132	-0.710	13.317
	3:BEAN GEV	-475.223	6.67E 3	-1.78E 3	-4.07E 3	12.648	1.26E 3
	4:BEAN GEV	475.223	-6.67E 3	1.78E 3	4.07E 3	-12.648	-1.26E 3
	5:BEAN ANG	8.443	884.245	-211.215	-483.072	1.547	-12.058
	6:BEAN ANG	-3.810	10.258	0.035	0.042	0.002	9.667
	7:KOM.1 (1.4 I	-40.990	59.4E 3	33.036	-20.505	-26.012	-270.708
	8:KOM.2 (1.2 I	-56.912	60.7E 3	25.711	-27.386	-23.432	-210.729
	9:KOM.3 (1.2 I	-35.236	58.4E 3	-311.256	-796.622	-20.531	-238.012
	10:KOM.4 (1.2	-54.841	57E 3	26.743	-23.639	-23.003	-203.252
	11:KOM.5 (0.9	-12.842	39.6E 3	-316.707	-786.096	-14.247	-193.320



Software licensed to Snow Panther [LZO]

Job No

Sheet No

40

Rev

Part

Job Title

Ref

By

Date 06-Oct-10

Chd

Client

File endhik-2 sip 4.std

Date/Time 21-Mar-2011 15:52

**Reactions Cont...**

Node	L/C	Horizontal	Vertical	Horizontal	Moment		
		FX (kg)	FY (kg)	FZ (kg)	MX (kg·m)	MY (kg·m)	MZ (kg·m)
	12:KOM.6 (0.9	-32.447	38.2E 3	21.293	-13.114	-16.719	-158.560
	13:KOM.7 (1.2	-48.745	57E 3	26.688	-23.707	-23.006	-218.719
	14:KOM.8 (1.2	-48.745	57E 3	26.688	-23.707	-23.006	-218.719
	15:KOM.9 (1.2	-48.745	57E 3	26.688	-23.707	-23.006	-218.719
	16:KOM.10 (1.	-48.745	57E 3	26.688	-23.707	-23.006	-218.719
	17:KOM.11 (0.!	-26.351	38.2E 3	21.238	-13.182	-16.722	-174.027
	18:KOM.12 (0.!	-26.351	38.2E 3	21.238	-13.182	-16.722	-174.027
	19:KOM.13 (0.!	-26.351	38.2E 3	21.238	-13.182	-16.722	-174.027
	20:KOM.14 (0.!	-26.351	38.2E 3	21.238	-13.182	-16.722	-174.027
3751	1:BEBAN MAT	-1.27E 3	229E 3	-1.4E 3	-2.11E 3	62.590	1.66E 3
	2:BEBAN HIDU	-28.972	8.8E 3	-38.755	-61.465	3.162	37.300
	3:BEBAN GEM	-653.853	6.95E 3	-1.76E 3	-4.03E 3	3.411	1.51E 3
	4:BEBAN GEM	653.853	-6.95E 3	1.76E 3	4.03E 3	-3.411	-1.51E 3
	5:BEBAN ANG	-19.461	999.856	-203.331	-471.085	-0.004	27.141
	6:BEBAN ANG	-3.607	4.592	-0.171	-0.262	0.013	9.387
	7:KOM.1 (1.4 [	-1.78E 3	320E 3	-1.96E 3	-2.95E 3	87.627	2.33E 3
	8:KOM.2 (1.2 [	-1.57E 3	289E 3	-1.74E 3	-2.63E 3	80.168	2.05E 3
	9:KOM.3 (1.2 [	-1.59E 3	285E 3	-2.04E 3	-3.34E 3	78.263	2.07E 3
	10:KOM.4 (1.2	-1.56E 3	283E 3	-1.72E 3	-2.59E 3	78.291	2.05E 3
	11:KOM.5 (0.9	-1.18E 3	208E 3	-1.58E 3	-2.65E 3	56.324	1.54E 3
	12:KOM.6 (0.9	-1.15E 3	206E 3	-1.26E 3	-1.9E 3	56.352	1.51E 3
	13:KOM.7 (1.2	-1.56E 3	283E 3	-1.72E 3	-2.59E 3	78.270	2.03E 3
	14:KOM.8 (1.2	-1.56E 3	283E 3	-1.72E 3	-2.59E 3	78.270	2.03E 3
	15:KOM.9 (1.2	-1.56E 3	283E 3	-1.72E 3	-2.59E 3	78.270	2.03E 3
	16:KOM.10 (1.	-1.56E 3	283E 3	-1.72E 3	-2.59E 3	78.270	2.03E 3
	17:KOM.11 (0.!	-1.14E 3	206E 3	-1.26E 3	-1.9E 3	56.331	1.5E 3
	18:KOM.12 (0.!	-1.14E 3	206E 3	-1.26E 3	-1.9E 3	56.331	1.5E 3
	19:KOM.13 (0.!	-1.14E 3	206E 3	-1.26E 3	-1.9E 3	56.331	1.5E 3
	20:KOM.14 (0.!	-1.14E 3	206E 3	-1.26E 3	-1.9E 3	56.331	1.5E 3
3752	1:BEBAN MAT	-1.29E 3	247E 3	1.2E 3	1.67E 3	35.862	1.62E 3
	2:BEBAN HIDU	-24.934	9.32E 3	24.856	31.543	2.233	28.290
	3:BEBAN GEM	-442.286	-3.16E 3	-1.56E 3	-3.72E 3	42.078	1.24E 3
	4:BEBAN GEM	442.286	3.16E 3	1.56E 3	3.72E 3	-42.078	-1.24E 3
	5:BEBAN ANG	15.384	-844.826	-187.589	-445.324	4.385	-19.168
	6:BEBAN ANG	-3.932	18.194	0.211	0.296	0.032	9.836
	7:KOM.1 (1.4 [	-1.8E 3	346E 3	1.67E 3	2.34E 3	50.207	2.27E 3
	8:KOM.2 (1.2 [	-1.59E 3	311E 3	1.47E 3	2.06E 3	46.608	1.99E 3
	9:KOM.3 (1.2 [	-1.55E 3	305E 3	1.16E 3	1.33E 3	52.284	1.94E 3
	10:KOM.4 (1.2	-1.58E 3	306E 3	1.46E 3	2.04E 3	45.319	1.99E 3
	11:KOM.5 (0.9	-1.14E 3	221E 3	775.648	794.690	39.292	1.43E 3
	12:KOM.6 (0.9	-1.17E 3	222E 3	1.08E 3	1.51E 3	32.327	1.47E 3
	13:KOM.7 (1.2	-1.57E 3	306E 3	1.46E 3	2.04E 3	45.268	1.97E 3
	14:KOM.8 (1.2	-1.57E 3	306E 3	1.46E 3	2.04E 3	45.268	1.97E 3
	15:KOM.9 (1.2	-1.57E 3	306E 3	1.46E 3	2.04E 3	45.268	1.97E 3
	16:KOM.10 (1.	-1.57E 3	306E 3	1.46E 3	2.04E 3	45.268	1.97E 3
	17:KOM.11 (0.!	-1.16E 3	222E 3	1.08E 3	1.51E 3	32.276	1.46E 3
	18:KOM.12 (0.!	-1.16E 3	222E 3	1.08E 3	1.51E 3	32.276	1.46E 3
	19:KOM.13 (0.!	-1.16E 3	222E 3	1.08E 3	1.51E 3	32.276	1.46E 3
	20:KOM.14 (0.!	-1.16E 3	222E 3	1.08E 3	1.51E 3	32.276	1.46E 3

Nomor : 06 / JHS-PCI / R-UNM/III/10  
Lampiran :  
Perihal : Permohonan Data & Informasi

Kepada Yth. Dekan Fakultas Teknik Sipil dan Perencanaan  
Institut Teknologi Nasional Malang

Menanggapi surat Saudara Nomor : ITN-117/III.TA/1/2009 perihal  
Permohonan Data dan Informasi

Setelah melalui berbagai pertimbangan, maka dengan ini kami  
memutuskan bahwa proyek yang sedang kami kerjakan, yakni :

PROYEK : RUSUNAWA UNIVERSITAS NEGERI MALANG  
LOKASI : JALAN SURABAYA NO.6 MALANG  
LINGKUP PEK. :  STRUKTURAL  ARSITEKTURAL  INSTALASI  M.E.  
PEMILIK : KEMENTERIAN NEGARA PERUMAHAN RAKYAT  
PERENCANA : PT. PERENTJANA DJAJA  
PENGAWAS : PT. SAKA ADHI PRADA  
PELAKSANA : PT. JHS PRECAST CONCRETE INDONESIA, JAKARTA TIMUR

adalah .....**LAYAK / ~~TIDAK LAYAK~~**.....coret yang tidak sesuai.

untuk dijadikan object melakukan Pengambilan Data & Informasi guna  
penyusunan Tugas Akhir bagi mahasiswa yang namanya tercantum dalam  
surat Saudara.

Demikian tanggapan dari kami. Atas perhatian ,kepercayaan dan kerjasama  
yang baik ini, disampaikan terima kasih.

Malang, 12 Maret 2010  
Project Manajer

  
PRECAST  
CONCRETE  
INDONESIA

**Dian Agung P. ST**

Tembusan Kepada:

1. Arsip

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## PT. JHS PRECAST CONCRETE INDUSTRI

**Office** : Jl. Tanah Abang II No. 23 Jakarta, Indonesia Phone : (021) 3844045 (6 Lines) Fax : (021) 372889 Telex : 44049 SAETI IA  
Jl. Raya Cakung Cilincing Kav. 48 - 50 Jakarta 13910 Phone : (021) 46827171 (Hunting) Fax : (021) 46827173  
**Branch Office** : Regency Park Blok 2 Number 22, Pelita - Batam 29432 Phone : (0778) 452061, Fax. : (0778) 451923  
**Factory** : Zona Industri Klari - Sumur Kondang, Karawang Phone./Fax. : (0267) - 433117

Tabel 3.1 : Perhitungan Pusat Massa Untuk Kolom Pada Lantai 2

No	Uraian	Berat Sendiri	Jarak		Momen		
			X	Y	MX	MY	
1	<b>KOLOM</b>						
	Berat sendiri = Luas x (1/2 tinggi kolom bawah + 1/2 tinggi kolom atas) x 2400						
	Titik pusat atau titik 0 (0,0) dapat dilihat pada gambar denah kolom pada masing-masing lantai						
	1	K2 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	6,70	3,50	7163,64	3742,20
	2	K3 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	10,60	3,50	11333,52	3742,20
	3	K4 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	14,50	3,50	15503,40	3742,20
	4	K5 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	18,40	3,50	19673,28	3742,20
	5	K6 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	22,30	3,50	23843,16	3742,20
	6	K7 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	26,20	3,50	28013,04	3742,20
	7	K8 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	30,10	3,50	32182,92	3742,20
	8	K18 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	6,70	8,30	7163,64	8874,36
	9	K19 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	10,60	8,30	11333,52	8874,36
	10	K20 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	14,50	8,30	15503,40	8874,36
	11	K21 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	18,40	8,30	19673,28	8874,36
	12	K22 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	22,30	8,30	23843,16	8874,36
13	K23 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	26,20	8,30	28013,04	8874,36	
14	K24 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	30,10	8,30	32182,92	8874,36	
		14968,80			275425,92	88315,92	
2	1	K9 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	34,00	3,50	36352,80	3742,20
	2	K10 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	37,90	3,50	40522,68	3742,20
	3	K11 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	41,80	3,50	44692,56	3742,20
	4	K12 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	45,70	3,50	48862,44	3742,20
	5	K13 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	49,50	3,50	52925,40	3742,20
	6	K14 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	53,50	3,50	57202,20	3742,20
	7	K15 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	57,40	3,50	61372,08	3742,20
	8	K24 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	30,10	8,30	32182,92	8874,36
	9	K26 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	37,90	8,30	40522,68	8874,36
	10	K27 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	41,80	8,30	44692,56	8874,36
	11	K28 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	45,70	8,30	48862,44	8874,36
	12	K29 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	49,50	8,30	52925,40	8874,36
	13	K30 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	53,50	8,30	57202,20	8874,36
	14	K31 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	57,40	8,30	61372,08	8874,36
		14968,80			679690,44	88315,92	
3	1	K41 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	34,00	15,50	36352,80	16572,60
	2	K42 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	37,90	15,50	40522,68	16572,60
	3	K43 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	41,80	15,50	44692,56	16572,60
	4	K44 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	45,70	15,50	48862,44	16572,60
	5	K45 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	49,50	15,50	52925,40	16572,60
	6	K46 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	53,50	15,50	57202,20	16572,60
	7	K47 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	57,40	15,50	61372,08	16572,60
	8	K57 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	34,00	20,30	36352,80	21704,76
	9	K58 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	37,90	20,30	40522,68	21704,76
	10	K59 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	41,80	20,30	44692,56	21704,76
	11	K60 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	45,70	20,30	48862,44	21704,76
	12	K61 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	49,50	20,30	52925,40	21704,76
	13	K62 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	53,50	20,30	57202,20	21704,76
	14	K63 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	57,40	20,30	61372,08	21704,76
		14968,80			683860,32	267941,52	
4	1	K34 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	6,70	15,50	7163,64	16572,60
	2	K35 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	10,60	15,50	11333,52	16572,60
	3	K36 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	14,50	15,50	15503,40	16572,60
	4	K37 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	18,40	15,50	19673,28	16572,60
	5	K38 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	22,30	15,50	23843,16	16572,60
	6	K39 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	26,20	15,50	28013,04	16572,60
	7	K40 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	30,10	15,50	32182,92	16572,60
	8	K50 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	6,70	20,30	7163,64	21704,76
	9	K51 = $0.3 \times 0.45 \times 1/2 \times (3.4+3.2) \times 2400$	1069,20	10,60	20,30	11333,52	21704,76



Tabel 3.2 : Perhitungan Pusat Massa Untuk Balok Pada Lantai 2

No	Uraian	Berat Sendiri	Jarak		Momen	
			X	Y	MX	MY
2	BALOK					
	Berat sendiri = Luas x Panjang x 2400					
	Titik pusat atau titik 0 (0,0) dapat dilihat pada gambar denah balok pada masing-masing lantai					
1	Line A (2-8) = 0.2 x 0.4 x 23.40 x 2400	4492,8	18,40	3,50	82667,52	15724,80
	Line B' (2-8) = 0.2 x 0.4 x 23.40 x 2400	4492,8	18,40	10,10	82667,52	45377,28
	Titik 2 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	6,70	6,80	13266,00	13464,00
	Titik 3 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	10,60	6,80	20988,00	13464,00
	Titik 4 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	14,50	6,80	28710,00	13464,00
	Titik 5 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	18,40	6,80	36432,00	13464,00
	Titik 6 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	22,30	6,80	44154,00	13464,00
	Titik 7 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	26,20	6,80	51876,00	13464,00
	Titik 8 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	30,10	6,80	59598,00	13464,00
		22845,60			420359,04	155350,08
2	Line A (9-15) = 0.2 x 0.4 x 23.40 x 2400	4492,8	45,69	3,50	205276,03	15724,80
	Line B' (9-15) = 0.2 x 0.4 x 23.40 x 2400	4492,8	45,69	10,10	205276,03	45377,28
	Titik 9 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	34,00	6,80	67320,00	13464,00
	Titik 10 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	37,90	6,80	75042,00	13464,00
	Titik 11 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	41,80	6,80	82764,00	13464,00
	Titik 12 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	45,70	6,80	90486,00	13464,00
	Titik 13 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	49,50	6,80	98010,00	13464,00
	Titik 14 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	53,50	6,80	105930,00	13464,00
Titik 15 (A-B') = 0.25 x 0.5 x 6.60 x 2400	1980,00	57,40	6,80	113652,00	13464,00	
		22845,60			1043756,06	155350,08
3	Line D (9-15) = 0.2 x 0.4 x 23.40 x 2400	4492,8	45,69	20,30	205276,03	91203,84
	Line B' (9-15) = 0.2 x 0.4 x 23.40 x 2400	4492,8	45,69	13,70	205276,03	61551,36
	Titik 9 (B'-D) = 0.25 x 0.5 x 6.60 x 2400	1980,00	34,00	17,00	67320,00	33660,00
	Titik 10 (B'-D) = 0.25 x 0.5 x 6.60 x 2400	1980,00	37,90	17,00	75042,00	33660,00
	Titik 11 (B'-D) = 0.25 x 0.5 x 6.60 x 2400	1980,00	41,80	17,00	82764,00	33660,00
	Titik 12 (B'-D) = 0.25 x 0.5 x 6.60 x 2400	1980,00	45,70	17,00	90486,00	33660,00
	Titik 13 (B'-D) = 0.25 x 0.5 x 6.60 x 2400	1980,00	49,50	17,00	98010,00	33660,00
	Titik 14 (B'-D) = 0.25 x 0.5 x 6.60 x 2400	1980,00	53,50	17,00	105930,00	33660,00
Titik 15 (B'-D) = 0.25 x 0.5 x 6.60 x 2400	1980,00	57,40	17,00	113652,00	33660,00	
		22845,60			1043756,06	388375,20
4	Line D (2-8) = 0.2 x 0.4 x 23.40 x 2400	4492,8	18,40	20,30	82667,52	91203,84
	Line B' (2-8) = 0.2 x 0.4 x 23.40 x 2400	4492,8	18,40	13,70	82667,52	61551,36
	Titik 2 (A-B) = 0.25 x 0.5 x 6.60 x 2400	1980,00	6,70	17,00	13266,00	33660,00
	Titik 3 (A-B) = 0.25 x 0.5 x 6.60 x 2400	1980,00	10,60	17,00	20988,00	33660,00
	Titik 4 (A-B) = 0.25 x 0.5 x 6.60 x 2400	1980,00	14,50	17,00	28710,00	33660,00
	Titik 5 (A-B) = 0.25 x 0.5 x 6.60 x 2400	1980,00	18,40	17,00	36432,00	33660,00
	Titik 6 (A-B) = 0.25 x 0.5 x 6.60 x 2400	1980,00	22,30	17,00	44154,00	33660,00
	Titik 7 (A-B) = 0.25 x 0.5 x 6.60 x 2400	1980,00	26,20	17,00	51876,00	33660,00
Titik 8 (A-B) = 0.25 x 0.5 x 6.60 x 2400	1980,00	30,10	17,00	59598,00	33660,00	
		22845,60			420359,04	388375,20
5	Line A (1-2) = 0.2 x 0.4 x 4.50 x 2400	864	4,45	3,50	3844,80	3024,00
	Line E' (1-2) = 0.2 x 0.4 x 4.50 x 2400	864	4,45	10,38	3844,80	8968,32
	Titik 1 (A-E') = 0.25 x 0.5 x 6.88 x 2400	2064	2,20	6,94	4540,80	14324,16
	Titik 2 (A-E') = 0.25 x 0.5 x 6.88 x 2400	2064	6,70	6,94	13828,80	14324,16
		5856,00			26059,20	40640,64
6	Line A (15-16) = 0.2 x 0.4 x 4.50 x 2400	864	59,65	3,50	51537,60	3024,00
	Line E' (15-16) = 0.2 x 0.4 x 4.50 x 2400	864	59,65	10,38	51537,60	8968,32
	Titik 15 (A-E') = 0.25 x 0.5 x 6.88 x 2400	2064	57,40	6,94	118473,60	14324,16
	Titik 16 (A-E') = 0.25 x 0.5 x 6.88 x 2400	2064	61,90	6,94	127761,60	14324,16
		5856,00			349310,40	40640,64
7	Line D (1-2) = 0.2 x 0.4 x 4.50 x 2400	864	4,45	20,30	3844,80	17539,20
	Line B' (1-2) = 0.2 x 0.4 x 4.50 x 2400	864	4,45	13,43	3844,80	11603,52
	Titik 1 (A-E') = 0.25 x 0.5 x 6.88 x 2400	2064	2,20	16,86	4540,80	34799,04
	Titik 2 (A-E') = 0.25 x 0.5 x 6.88 x 2400	2064	6,70	16,86	13828,80	34799,04
		5856,00			26059,20	98740,80

8	Line D (15-16)	= 0.2 x 0.4 x 4.50 x 2400	864	59,65	20,30	51537,60	17539,20
	Line B* (15-16)	= 0.2 x 0.4 x 4.50 x 2400	864	59,65	13,43	51537,60	11603,52
	Titik 15 (A-E)	= 0.25 x 0.5 x 6.88 x 2400	2064	57,40	16,86	118473,60	34799,04
	Titik 16 (A-E)	= 0.25 x 0.5 x 6.88 x 2400	2064	61,90	16,86	127761,60	34799,04
			5856,00			349310,40	98740,80
9	Line C' (16-16')	= 0.2 x 0.4 x 1.90 x 2400	364,8	62,85	17,50	22927,68	6384,00
	Line E' (16-16')	= 0.2 x 0.4 x 1.90 x 2400	364,8	62,85	13,43	22927,68	4899,26
	Titik 16 (A-E')	= 0.2 x 0.3 x 4.08 x 2400	587,52	61,90	15,46	36367,49	9083,06
	Titik 16' (A-E')	= 0.2 x 0.3 x 4.08 x 2400	587,52	63,80	15,46	37483,78	9083,06
			1904,64			119706,62	29449,38
10	Line A' (16-16')	= 0.2 x 0.4 x 1.90 x 2400	364,8	62,85	6,30	22927,68	2298,24
	Line E' (16-16')	= 0.2 x 0.4 x 1.90 x 2400	364,8	62,85	10,68	22927,68	3896,06
	Titik 16 (A-E')	= 0.2 x 0.3 x 4.08 x 2400	587,52	61,90	8,34	36367,49	4899,92
	Titik 16' (A-E')	= 0.2 x 0.3 x 4.08 x 2400	587,52	63,80	8,34	37483,78	4899,92
			1904,64			119706,62	15994,14
11	Line A' (16-16')	= 0.2 x 0.4 x 1.90 x 2400	364,8	1,25	6,30	456,00	2298,24
	Line E' (16-16')	= 0.2 x 0.4 x 1.90 x 2400	364,8	1,25	10,68	456,00	3896,06
	Titik 1' (A-E')	= 0.2 x 0.3 x 4.08 x 2400	587,52	0,30	8,34	176,26	4899,92
	Titik 1 (A-E')	= 0.2 x 0.3 x 4.08 x 2400	587,52	2,20	8,34	1292,54	4899,92
			1904,64			2380,80	15994,14
12	Line A' (16-16')	= 0.2 x 0.4 x 1.90 x 2400	364,8	1,25	17,50	456,00	6384,00
	Line E' (16-16')	= 0.2 x 0.4 x 1.90 x 2400	364,8	1,25	13,43	456,00	4899,26
	Titik 1' (A-E')	= 0.2 x 0.3 x 4.08 x 2400	587,52	0,30	15,46	176,26	9083,06
	Titik 1 (A-E')	= 0.2 x 0.3 x 4.08 x 2400	587,52	2,20	15,46	1292,54	9083,06
			1904,64			2380,80	29449,38
13	Line A0 (8-9)	= 0.2 x 0.3 x 3.90 x 2400	561,6	32,05	0,00	17999,28	0,00
	Line A (8-9)	= 0.2 x 0.4 x 3.90 x 2400	748,8	32,05	3,50	23999,04	2620,80
	Line B (8-9)	= 0.2 x 0.4 x 3.90 x 2400	748,8	32,05	8,80	23999,04	6589,44
	Line B' (8-9)	= 0.2 x 0.4 x 3.90 x 2400	748,8	32,05	10,10	23999,04	7562,88
	Line B* (8-9)	= 0.2 x 0.4 x 3.90 x 2400	748,8	32,05	13,70	23999,04	10258,56
	Line C (8-9)	= 0.2 x 0.4 x 3.90 x 2400	748,8	32,05	15,50	23999,04	11606,40
	Line D (8-9)	= 0.2 x 0.4 x 3.90 x 2400	748,8	32,05	20,30	23999,04	15200,64
	Line D' (8-9)	= 0.2 x 0.3 x 3.90 x 2400	561,6	32,05	23,80	17999,28	13366,08
	Titik 8 (A0-A')	= 0.2 x 0.3 x 3.50 x 2400	504	30,10	1,50	15170,40	756,00
	Titik 8 (A-B')	= 0.25 x 0.5 x 6.60 x 2400	1980	30,10	6,80	59598,00	13464,00
	Titik 8 (B*-D)	= 0.25 x 0.5 x 6.60 x 2400	1980	30,10	17,00	59598,00	33660,00
	Titik 8 (D-D')	= 0.2 x 0.3 x 3.50 x 2400	504	30,10	22,05	15170,40	11113,20
	Titik 9 (A0-A)	= 0.2 x 0.3 x 3.50 x 2400	504	34,00	1,50	17136,00	756,00
	Titik 9 (A-D)	= 0.25 x 0.5 x 16.80 x 2400	1980	34,00	11,90	67320,00	23562,00
Titik 9 (D-D')	= 0.2 x 0.3 x 3.50 x 2400	504	34,00	22,05	17136,00	11113,20	
			13572,00			431121,60	161629,20



Tabel 3.3 : Perhitungan Pusat Massa Untuk Plat Pada Lantai 2

No	Uraian	Berat Sendiri	Jarak		Momen	
			X	Y	MX	MY
3	PLAT Berat Sendiri = Luas x Tebal x 2400 Titik pusat atau titik 0 (0,0) dapat dilihat pada gambar denah dinding pada masing-masing lantai					
1	Line A-B' (2-8) = 23.40 x 6.60 x 0.12 x 2400	44478,72	18,40	6,80	818408,45	302455,30
2	Line A-B' (9-15) = 23.40 x 6.60 x 0.12 x 2400	44478,72	45,69	6,80	2032232,72	302455,30
3	Line B*-D (9-15) = 23.40 x 6.60 x 0.12 x 2400	44478,72	45,69	17,00	2032232,72	756138,24
4	Line B*-D (9-15) = 23.40 x 6.60 x 0.12 x 2400	44478,72	23,40	17,00	1040802,05	756138,24
5	Line A-B' (1-2) = 4.50 x 6.88 x 0.12 x 2400	8916,48	4,45	6,94	39678,34	61880,37
6	Line A-B' (15-16) = 4.50 x 6.88 x 0.12 x 2400	8916,48	59,62	6,94	531600,54	61880,37
7	Line A-B' (1-2) = 4.50 x 6.88 x 0.12 x 2400	8916,48	4,45	16,86	39678,34	150331,85
8	Line A-B' (15-16) = 4.50 x 6.88 x 0.12 x 2400	8916,48	59,62	16,86	531600,54	150331,85
9	Line C-E* (16-16') = 1.90 x 4.08 x 0.12 x 2400	2232,576	62,85	15,46	140317,40	34515,62
10	Line A'-E' (16-16') = 1.90 x 4.08 x 0.12 x 2400	2232,576	62,85	8,34	140317,40	18619,68
11	Line A'-E' (1*-1) = 1.90 x 4.08 x 0.12 x 2400	2232,576	1,25	8,34	2790,72	18619,68
12	Line C-E* (1*-1) = 1.90 x 4.08 x 0.12 x 2400	2232,576	1,25	15,46	2790,72	34515,62
13	Line A0-D' (8-9) = 3.90 x 23.80 x 0.12 x 2400	26732,16	32,05	11,90	856765,73	318112,70
		249243,26			8209215,65	2965994,84

Tabel 3.4 : Perhitungan Pusat Massa Untuk Tangga Dan Bordes Pada Lantai 2

No	Uraian	Berat Sendiri	Jarak		Momen	
			X	Y	MX	MY
4	<b>TANGGA</b>					
	Berat sendiri tangga = (Panjang miring x lebar x tebal plat tangga x 2400) + (Jumlah anak tangga x 0,5 x panjang anak tangga x lebar anak tangga x tinggi anak tangga x 2400)					
	Bordes = (n jumlah balok bordes x lebar balok bordes x tinggi balok bordes x 2400) + (tebal bordes x lebar bordes x panjang bordes x 2400)					
	Titik pusat atau titik 0 (0,0) dapat dilihat pada gambar denah dinding pada masing-masing lantai					
	<b>Tangga Samping kiri</b>					
	Berat Sendiri = (2.754 x 1.375 x 0.21 x 2400) + (9 x 0.5 x 1.375 x 0.2 x 2400)	7578,52	3,63	11,90	27510,03	90184,41
	bordes = 2 x (0.2 x 0.3 x 1.825 x 2400) + (0.12 x 3.05 x 2400)	1404,00	1,40	11,90	1965,60	16707,60
	<b>Tangga Utama</b>					
	Berat sendiri = (2.754 x 1.5 x 0.21 x 2400) + (9 x 0.5 x 1.375 x 0.2 x 2400)	7752,02	28,69	11,90	222405,57	92249,09
	bordes = 2 x (0.2 x 0.3 x 1.825 x 2400) + (0.12 x 3.05 x 2400)	1404,00	26,37	11,90	37023,48	16707,60
	<b>Tangga Samping kanan</b>					
	Berat Sendiri = (2.754 x 1.375 x 0.21 x 2400) + (9 x 0.5 x 1.375 x 0.2 x 2400)	7578,52	60,48	11,90	458349,01	90184,41
	bordes = 2 x (0.2 x 0.3 x 1.825 x 2400) + (0.12 x 3.05 x 2400)	1404,00	62,70	11,90	88030,80	16707,60
	Σ	27121,07			835284,49	322740,71

Tabel 3.5 : Perhitungan Pusat Massa Untuk Dinding Pada Lantai 2

No	Uraian	Berat Sendiri	Jarak		Momen		
			X	Y	MX	MY	
5	DINDING						
	Berat sendiri = Panjang x Tinggi x 250						
	Titik pusat atau titik 0 (0,0) dapat dilihat pada gambar denah dinding pada masing-masing lantai						
	1 Line A (2-8)	= 23,40 x 2,9 x 250	16965,00	18,40	3,50	312156,00	59377,50
	2 Line B (2-8)	= 23,40 x 2,9 x 250	16965,00	18,40	8,30	312156,00	140809,50
	3 Titik 2 (A-B)	= 4,80 x 2,9 x 250	3480,00	6,70	5,90	23316,00	20532,00
	4 Titik 3 (A-B)	= 4,80 x 2,9 x 250	3480,00	10,60	5,90	36888,00	20532,00
	5 Titik 4 (A-B)	= 4,80 x 2,9 x 250	3480,00	14,50	5,90	50460,00	20532,00
	6 Titik 5 (A-B)	= 4,80 x 2,9 x 250	3480,00	18,40	5,90	64032,00	20532,00
	7 Titik 6 (A-B)	= 4,80 x 2,9 x 250	3480,00	22,30	5,90	77604,00	20532,00
8 Titik 7 (A-B)	= 4,80 x 2,9 x 250	3480,00	26,20	5,90	91176,00	20532,00	
9 Titik 8 (A-B)	= 4,80 x 2,9 x 250	3480,00	30,10	5,90	104748,00	20532,00	
		58290,00			1072536,00	343911,00	
2	1 Line A (9-15)	= 23,40 x 2,9 x 250	16965,00	45,69	3,50	775130,85	59377,50
	2 Line B (9-15)	= 23,40 x 2,9 x 250	16965,00	45,69	10,10	775130,85	171346,50
	3 Titik 9 (A-B)	= 4,80 x 2,9 x 250	3480,00	34,00	5,90	118320,00	20532,00
	4 Titik 10 (A-B)	= 4,80 x 2,9 x 250	3480,00	37,90	5,90	131892,00	20532,00
	5 Titik 11 (A-B)	= 4,80 x 2,9 x 250	3480,00	41,80	5,90	145464,00	20532,00
	6 Titik 12 (A-B)	= 4,80 x 2,9 x 250	3480,00	45,70	5,90	159036,00	20532,00
	7 Titik 13 (A-B)	= 4,80 x 2,9 x 250	3480,00	49,50	5,90	172608,00	20532,00
	8 Titik 14 (A-B)	= 4,80 x 2,9 x 250	3480,00	53,50	5,90	186180,00	20532,00
	9 Titik 15 (A-B)	= 4,80 x 2,9 x 250	3480,00	57,40	5,90	199752,00	20532,00
			58290,00			2663165,70	374448,00
3	1 Line D (9-15)	= 23,40 x 2,9 x 250	16965,00	45,69	20,30	775130,85	344389,50
	2 Line C (9-15)	= 23,40 x 2,9 x 250	16965,00	45,69	13,70	775130,85	232420,50
	3 Titik 9 (B*-D)	= 4,80 x 2,9 x 250	3480,00	34,00	17,90	118320,00	62292,00
	4 Titik 10 (B*-D)	= 4,80 x 2,9 x 250	3480,00	37,90	17,90	131892,00	62292,00
	5 Titik 11 (B*-D)	= 4,80 x 2,9 x 250	3480,00	41,80	17,90	145464,00	62292,00
	6 Titik 12 (B*-D)	= 4,80 x 2,9 x 250	3480,00	45,70	17,90	159036,00	62292,00
	7 Titik 13 (B*-D)	= 4,80 x 2,9 x 250	3480,00	49,50	17,90	172608,00	62292,00
	8 Titik 14 (B*-D)	= 4,80 x 2,9 x 250	3480,00	53,50	17,90	186180,00	62292,00
	9 Titik 15 (B*-D)	= 4,80 x 2,9 x 250	3480,00	57,40	17,90	199752,00	62292,00
			58290,00			2663165,70	1012854,00
4	1 Line D (2-8)	= 23,40 x 2,9 x 250	16965,00	18,40	20,30	312156,00	344389,50
	2 Line C (2-8)	= 23,40 x 2,9 x 250	16965,00	18,40	13,70	312156,00	232420,50
	3 Titik 2 (A-B)	= 4,80 x 2,9 x 250	3480,00	6,70	17,90	23316,00	62292,00
	4 Titik 3 (A-B)	= 4,80 x 2,9 x 250	3480,00	10,60	17,90	36888,00	62292,00
	5 Titik 4 (A-B)	= 4,80 x 2,9 x 250	3480,00	14,50	17,90	50460,00	62292,00
	6 Titik 5 (A-B)	= 4,80 x 2,9 x 250	3480,00	18,40	17,90	64032,00	62292,00
	7 Titik 6 (A-B)	= 4,80 x 2,9 x 250	3480,00	22,30	17,90	77604,00	62292,00
	8 Titik 7 (A-B)	= 4,80 x 2,9 x 250	3480,00	26,20	17,90	91176,00	62292,00
	9 Titik 8 (A-B)	= 4,80 x 2,9 x 250	3480,00	30,10	17,90	104748,00	62292,00
			58290,00			1072536,00	1012854,00
5	1 Line A (1-2)	= 4,50 x 2,9 x 250	3262,50	4,45	3,50	14518,13	11418,75
	2 Line B (1-2)	= 4,50 x 2,9 x 250	3262,50	4,45	10,38	14518,13	33864,75
	3 Titik 1 (A-B)	= 4,80 x 2,9 x 250	3480,00	2,20	6,94	7656,00	24151,20
	4 Titik 2 (A-B)	= 4,80 x 2,9 x 250	3480,00	6,70	6,94	23316,00	24151,20
		13485,00			60008,25	93585,90	
6	1 Line A (15-16)	= 4,50 x 2,9 x 250	3262,50	59,65	3,50	194608,13	11418,75
	2 Line B (15-16)	= 4,50 x 2,9 x 250	3262,50	59,65	10,38	194608,13	33864,75
	3 Titik 15 (A-B)	= 4,80 x 2,9 x 250	3480,00	57,40	6,94	199752,00	24151,20
	4 Titik 16 (A-B)	= 4,80 x 2,9 x 250	3480,00	61,90	6,94	215412,00	24151,20
		13485,00			804380,25	93585,90	

7	1	Line D (1-2)	= 4.50 x 2.9 x 250	3262,50	4,45	20,30	14518,13	66228,75
	2	Line C (1-2)	= 4.50 x 2.9 x 250	3262,50	4,45	13,43	14518,13	43815,38
	3	Tiik 1 (C-D)	= 4.80 x 2.9 x 250	3480,00	2,20	16,86	7656,00	58672,80
	4	Tiik 2 (C-D)	= 4.80 x 2.9 x 250	3480,00	6,70	16,86	23316,00	58672,80
				13485,00			60008,25	227389,73
8	1	Line D (15-16)	= 4.50 x 2.9 x 250	3262,50	59,65	20,30	194608,13	66228,75
	2	Line C (15-16)	= 4.50 x 2.9 x 250	3262,50	59,65	13,43	194608,13	43815,38
	3	Tiik 15 (C-D)	= 4.80 x 2.9 x 250	3480,00	57,40	16,86	199752,00	58672,80
	4	Tiik 16 (C-D)	= 4.80 x 2.9 x 250	3480,00	61,90	16,86	215412,00	58672,80
				13485,00			804380,25	227389,73
9	1	Line C' (16-16')	= 1.90 x 0.8 x 250	380,00	62,85	17,50	23883,00	6650,00
	2	Line B' (16-16')	= 1.90 x 0.8 x 250	380,00	62,85	13,43	23883,00	5103,40
	3	Tiik 16 (A-B')	= 4.08 x 0.8 x 250	816,00	61,90	15,46	50510,40	12615,36
	4	Tiik 16' (A-B')	= 4.08 x 0.8 x 250	816,00	63,80	15,46	52060,80	12615,36
				2392,00			150337,20	36984,12
10	1	Line A' (16-16')	= 1.90 x 0.8 x 250	380,00	62,85	6,30	23883,00	2394,00
	2	Line E' (16-16')	= 1.90 x 0.8 x 250	380,00	62,85	10,68	23883,00	4058,40
	3	Tiik 16 (A-E')	= 4.08 x 0.8 x 250	816,00	61,90	8,34	50510,40	6805,44
	4	Tiik 16' (A-E')	= 4.08 x 0.8 x 250	816,00	63,80	8,34	52060,80	6805,44
				2392,00			150337,20	20063,28
11	1	Line A' (16-16')	= 1.90 x 0.8 x 250	380,00	1,25	6,30	475,00	2394,00
	2	Line E' (16-16')	= 1.90 x 0.8 x 250	380,00	1,25	10,68	475,00	4058,40
	3	Tiik 1' (A-E')	= 4.08 x 0.8 x 250	816,00	0,30	8,34	244,80	6805,44
	4	Tiik 1 (A-E')	= 4.08 x 0.8 x 250	816,00	2,20	8,34	1795,20	6805,44
				2392,00			2990,00	20063,28
12	1	Line C' (1*-1)	= 1.90 x 0.8 x 250	380,00	1,25	17,50	475,00	6650,00
	2	Line B' (1*-1)	= 1.90 x 0.8 x 250	380,00	1,25	13,43	475,00	5103,40
	3	Tiik 1' (C-B')	= 4.08 x 0.8 x 250	816,00	0,30	15,46	244,80	12615,36
	4	Tiik 1 (C-B')	= 4.08 x 0.8 x 250	816,00	2,20	15,46	1795,20	12615,36
				2392,00			2990,00	36984,12
13	1	Line A (8-9)	= 3.90 x 2.9 x 250	2827,50	32,05	3,50	90621,38	9896,25
	2	Line B (8-9)	= 3.90 x 2.9 x 250	2827,50	32,05	8,30	90621,38	23468,25
	3	Line C (8-9)	= 3.90 x 2.9 x 250	2827,50	32,05	15,50	90621,38	43826,25
	4	Line D (8-9)	= 3.90 x 2.9 x 250	2827,50	32,05	20,30	90621,38	57398,25
	5	Tiik 8 (A-B)	= 4.80 x 2.9 x 250	33408,00	30,10	6,80	1005580,80	227174,40
	6	Tiik 8 (C-D)	= 4.80 x 2.9 x 250	33408,00	30,10	17,00	1005580,80	567936,00
	7	Tiik 8 (D-D')	= 4.80 x 2.9 x 250	33408,00	30,10	22,05	1005580,80	736646,40
	8	Tiik 9 (A-D)	= 4.80 x 2.9 x 250	33408,00	34,00	11,90	1135872,00	397555,20
				144942,00			4515099,90	2063901,00













### III. HASIL PENGUJIAN DAN ANALISIS HASIL UJI

#### A. Hasil Uji Sondir

Data dan grafik perolehan angka uji sondir di tiap titik uji dapat dilihat pada lembar lampiran. Dari denah lokasi dan data uji sondir di setiap titik uji dapat dikemukakan sebagai berikut:

**Tabel 01. Ringkasan Hasil Uji Sondir**

Titik Uji	Kedalaman Uji Sondir (m)	Perlawanan Konus (qc) (kg/cm <sup>2</sup> )	JHP (kg/cm)
S1	0.00	0	0.00
	0.40	25	20.00
	1.00	10	70.00
	1.40	20	110.00
	2.00	35	170.00
	2.40	45	220.00
	3.00	30	310.00
	3.40	20	360.00
	4.00	25	420.00
	4.40	20	450.00
	5.00	45	520.00
	5.40	50	570.00
	6.00	150	710.00
S2	0.00	0	0.00
	0.40	10	30.00
	1.00	25	130.00
	1.40	15	170.00
	2.00	25	230.00
	2.40	45	300.00
	3.00	65	390.00
	3.40	65	490.00
	4.00	40	590.00
	4.40	35	650.00
	5.00	30	730.00
	5.40	35	770.00
	6.00	90	890.00
6.40	150	1030.00	

Titik Uji	Kedalaman Uji Sondir (m)	Perlawanan Konus (qc) (kg/cm <sup>2</sup> )	JHP (kg/cm)
S3	0.00	0	0.00
	0.40	15	20.00
	1.00	30	30.00
	1.40	20	140.00
	2.00	20	170.00
	2.40	20	200.00
	3.00	30	270.00
	3.40	30	320.00
	4.00	25	400.00
	4.40	15	440.00
	5.00	15	510.00
	5.40	30	560.00
	6.00	45	670.00
	6.40	110	820.00
	6.60	155	880.00

Dari hasil pengujian diperoleh data tanah keras dengan nilai qc  $\geq$  150 kg/cm<sup>2</sup>, terdapat pada kedalaman yang berbeda-beda, seperti pada tabel berikut:

Titik Uji	Kedalaman Tanah Keras (m)	Perlawanan Konus (qc) (kg/cm <sup>2</sup> )	JHP (kg/cm)
S1	6.00	150	710.00
S2	5.40	150	1030.00
S3	6.60	155	880.00

#### IV. LAMPIRAN - LAMPIRAN

1. Denah Lokasi
2. Data dan Grafik Uji Sondir
3. Photo-photo



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Proyek : PEMBANGUNAN GEDUNG RUSUNAWA Tgl. Uji : 03.05.2009 TITIK UJI : S1  
 UNIVERSITAS NEGERI MALANG Dikerjakan : St dan Ww  
 Lokasi : JL. SURABAYA 6 MALANG Diperiksa : MS

**UJI PENETRASI KONUS (CONE PENETRATION TEST)**

**ASTM D 3441 - 86**

Kedalaman	Perlawanan Konus (PK)	Jumlah Perlawanan (JP)	Hambatan Pelekat (HP) =(JP-PK)/10	HP x 20	JHP =Σ (HP x 20)	Ratio Gesekan (FR) = (HP/PK)x100
(Meter)	(kg/cm <sup>2</sup> )	(kg/cm <sup>2</sup> )	(kg/cm <sup>2</sup> )	(kg/cm)	(kg/cm)	(%)
0,00	0	0	0,00	0,00	0,00	0,00
0,20	0	0	0,00	0,00	0,00	0,00
0,40	25	35	1,00	20,00	20,00	4,00
0,60	10	25	1,50	30,00	50,00	15,00
0,80	10	15	0,50	10,00	60,00	5,00
1,00	10	15	0,50	10,00	70,00	5,00
1,20	15	25	1,00	20,00	90,00	6,67
1,40	20	30	1,00	20,00	110,00	5,00
1,60	20	30	1,00	20,00	130,00	5,00
1,80	25	35	1,00	20,00	150,00	4,00
2,00	35	45	1,00	20,00	170,00	2,86
2,20	40	55	1,50	30,00	200,00	3,75
2,40	45	55	1,00	20,00	220,00	2,22
2,60	45	60	1,50	30,00	250,00	3,33
2,80	40	55	1,50	30,00	280,00	3,75
3,00	30	45	1,50	30,00	310,00	5,00
3,20	30	35	0,50	10,00	320,00	1,67
3,40	20	40	2,00	40,00	360,00	10,00
3,60	25	35	1,00	20,00	380,00	4,00
3,80	20	30	1,00	20,00	400,00	5,00
4,00	25	35	1,00	20,00	420,00	4,00
4,20	20	30	1,00	20,00	440,00	5,00
4,40	20	25	0,50	10,00	450,00	2,50
4,60	15	25	1,00	20,00	470,00	6,67
4,80	35	45	1,00	20,00	490,00	2,86
5,00	45	60	1,50	30,00	520,00	3,33
5,20	30	40	1,00	20,00	540,00	3,33
5,40	50	65	1,50	30,00	570,00	3,00
5,60	80	95	1,50	30,00	600,00	1,88
5,80	105	130	2,50	50,00	650,00	2,38
6,00	150	180	3,00	60,00	710,00	2,00

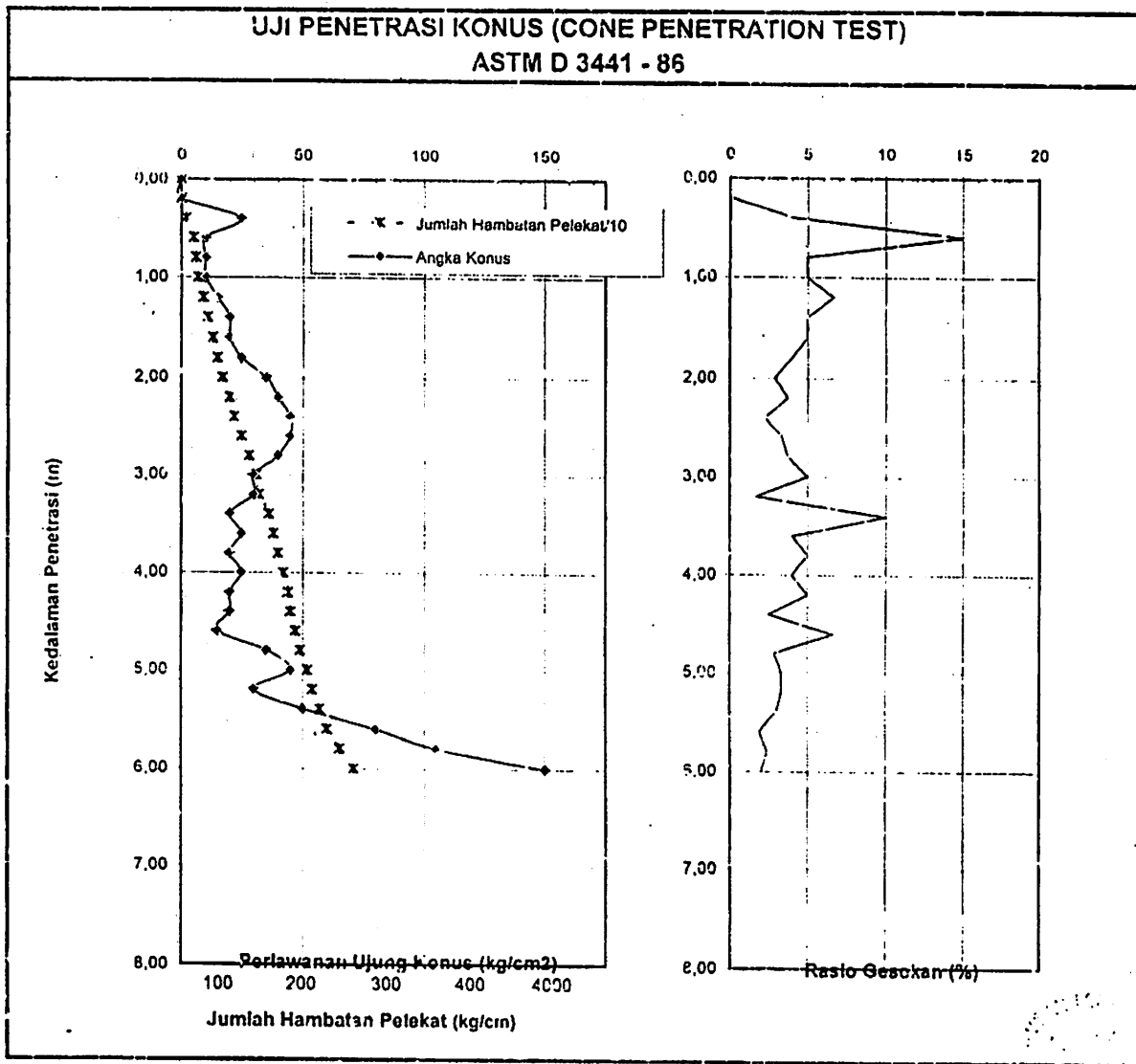


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**UNIVERSITAS NEGERI MALANG**

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Proyek : PEMBANGUNAN GEDUNG RUSUNAWA Tgl. Uji : 03.05.2009 TITIK UJI : S1  
Lokasi : JL. SURABAYA 6, MALANG Dikerjakan : St dan Ww  
Diperiksa : MS

**UJI PENETRASI KONUS (CONE PENETRATION TEST)**  
**ASTM D 3441 - 86**





**LABORATORIUM MEKANIKA TANAH**  
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**UNIVERSITAS NEGERI MALANG**

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Proyek : PEMBANGUNAN GEDUNG RUSUNAWA Tgl. Uji : 03.05.2009 TITIK Uji : S2  
 UNIVERSITAS NEGERI MALANG Dilaksanakan : St dan Ww  
 Lokasi : JL. SURABAYA 6 MALANG Diperiksa : MS

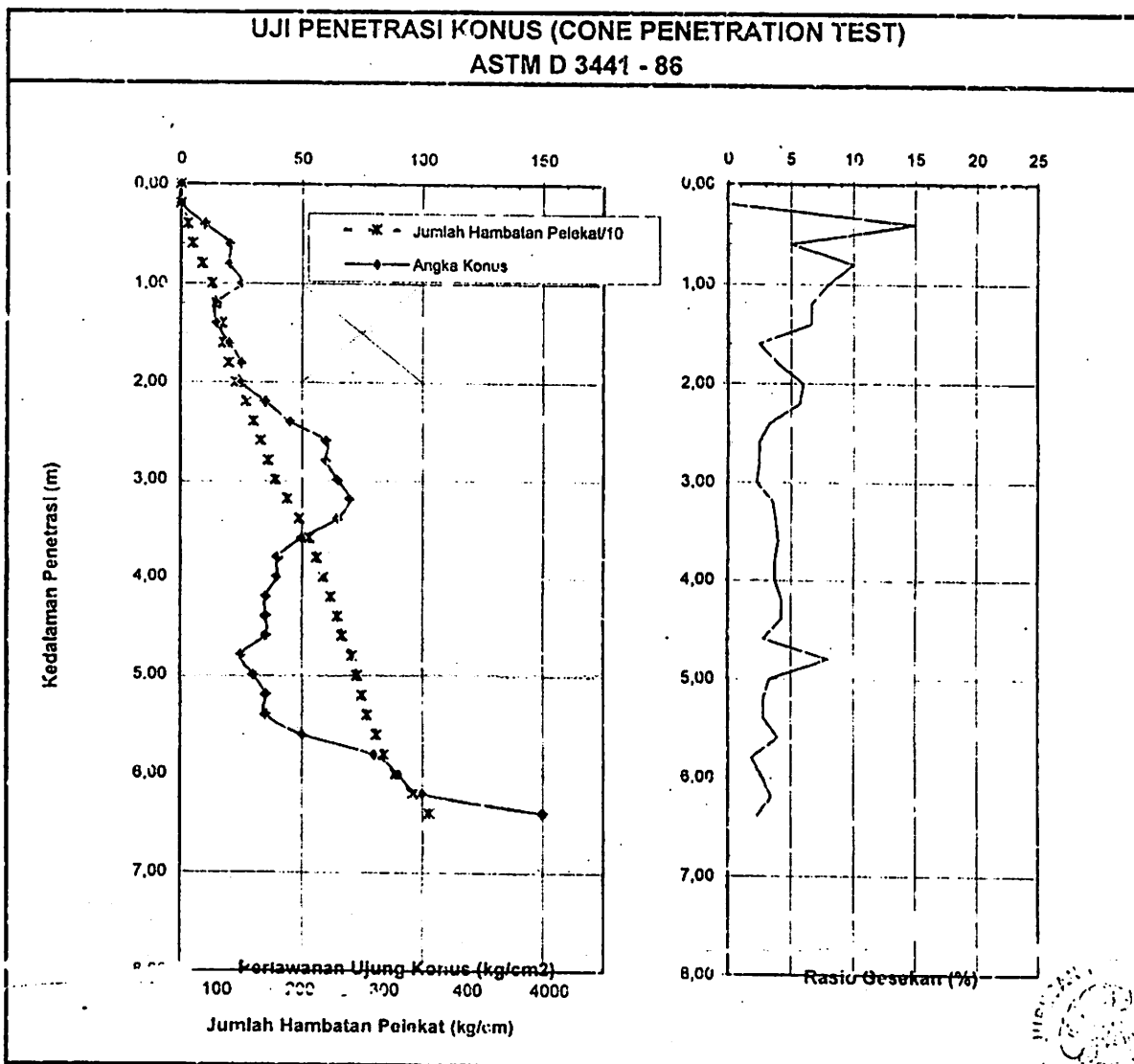
UJI PENETRASI KONUS (CONE PENETRATION TEST)						
ASTM D 3441 - 86						
Kedalaman	Perlawanan Konus (PK)	Jumlah Perlawanan (JP)	Hambatan Pelekat (HP) =(JP-PK)/10	HP x 20	JHP =Σ (HP x 20)	Ratio Gesekan (FR) = (HP/PK)x100
(Meter)	(kg/cm <sup>2</sup> )	(kg/cm <sup>2</sup> )	(kg/cm <sup>2</sup> )	(kg/cm)	(kg/cm)	(%)
0,00	0	0	0,00	0,00	0,00	0,00
0,20	0	0	0,00	0,00	0,00	0,00
0,40	10	25	1,50	30,00	30,00	15,00
0,60	20	30	1,00	20,00	50,00	5,00
0,80	20	40	2,00	40,00	90,00	10,00
1,00	25	45	2,00	40,00	130,00	8,00
1,20	15	25	1,00	20,00	150,00	6,67
1,40	15	25	1,00	20,00	170,00	6,67
1,60	20	25	0,50	10,00	180,00	2,50
1,80	25	35	1,00	20,00	200,00	4,00
2,00	25	40	1,50	30,00	230,00	6,00
2,20	35	55	2,00	40,00	270,00	5,71
2,40	45	60	1,50	30,00	300,00	3,33
2,60	60	75	1,50	30,00	330,00	2,50
2,80	60	75	1,50	30,00	360,00	2,50
3,00	65	80	1,50	30,00	390,00	2,31
3,20	70	95	2,50	50,00	440,00	3,57
3,40	65	90	2,50	50,00	490,00	3,85
3,60	50	70	2,00	40,00	530,00	4,00
3,80	40	55	1,50	30,00	560,00	3,75
4,00	40	55	1,50	30,00	590,00	3,75
4,20	35	50	1,50	30,00	620,00	4,29
4,40	35	50	1,50	30,00	650,00	4,29
4,60	35	45	1,00	20,00	670,00	2,86
4,80	25	45	2,00	40,00	710,00	8,00
5,00	30	40	1,00	20,00	730,00	3,33
5,20	35	45	1,00	20,00	750,00	2,86
5,40	35	45	1,00	20,00	770,00	2,86
5,60	50	70	2,00	40,00	810,00	4,00
5,80	80	95	1,50	30,00	840,00	1,88
6,00	90	115	2,50	50,00	890,00	2,78
6,20	100	135	3,50	70,00	960,00	3,50
6,40	150	185	3,50	70,00	1030,00	2,33



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Proyek : PEMBANGUNAN GEDUNG RUSUNAWA Tgl. Uji : 03.05.2009 TITIK UJI : 32  
Lokasi : JL. SURABAYA 6 MALANG Dikerjakan : St dan Ww  
Diperiksa : MS





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**UNIVERSITAS NEGERI MALANG**

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Proyek : PEMBANGUNAN GEDUNG RUSUNAWA Tgl. Uji : 03.05.2009 TITIK UJI : S3  
 UNIVERSITAS NEGERI MALANG Dikerjakan : St dan Ww  
 Lokasi : JL. SURABAYA 6 MALANG Diperiksa : MS

UJI PENETRASI KONUS (CONE PENETRATION TEST)						
ASTM D 3441 - 85						
Kedalaman	Perlawanan Konus (PK)	Jumlah Perlawanan (JP)	Hambatan Pelekat (HP) = (JP-PK)/10	HP x 20	JHP = Σ (HP x 20)	Ratio Gesekan (FR) = (JHP/PK)x100
(Meter)	[kg/cm <sup>2</sup> ]	[kg/cm <sup>2</sup> ]	[kg/cm <sup>2</sup> ]	[kg/cm]	[kg/cm]	(%)
0,00	0	0	0,00	0,00	0,00	0,00
0,20	0	0	0,00	0,00	0,00	0,00
0,40	15	25	1,00	20,00	20,00	6,67
0,60	15	25	1,00	20,00	40,00	6,67
0,80	30	40	1,00	20,00	60,00	3,33
1,00	30	40	1,00	20,00	80,00	3,33
1,20	30	50	2,00	40,00	120,00	6,67
1,40	20	30	1,00	20,00	140,00	5,00
1,60	20	25	0,50	10,00	150,00	2,50
1,80	20	25	0,50	10,00	160,00	2,50
2,00	20	25	0,50	10,00	170,00	2,50
2,20	20	25	0,50	10,00	180,00	2,50
2,40	20	30	1,00	20,00	200,00	5,00
2,50	25	35	1,00	20,00	220,00	4,00
2,80	25	40	1,50	30,00	250,00	6,00
3,00	30	40	1,00	20,00	270,00	3,33
3,20	35	45	1,00	20,00	290,00	2,86
3,40	30	45	1,50	30,00	320,00	5,00
3,60	25	40	1,50	30,00	350,00	6,00
3,80	20	35	1,50	30,00	380,00	7,50
4,00	25	35	1,00	20,00	400,00	4,00
4,20	20	25	0,50	10,00	410,00	2,50
4,40	15	30	1,50	30,00	440,00	10,00
4,60	35	45	1,00	20,00	460,00	2,86
4,80	20	35	1,50	30,00	490,00	7,50
5,00	15	25	1,00	20,00	510,00	6,67
5,20	15	25	1,00	20,00	530,00	6,67
5,40	30	45	1,50	30,00	560,00	5,00
5,60	30	50	2,00	40,00	600,00	3,67
5,80	35	45	1,00	20,00	620,00	2,86
6,00	45	70	2,50	50,00	670,00	8,89
6,20	75	110	3,50	70,00	740,00	4,67
6,40	110	150	4,00	80,00	820,00	3,64
6,50	155	185	3,00	60,00	880,00	1,94



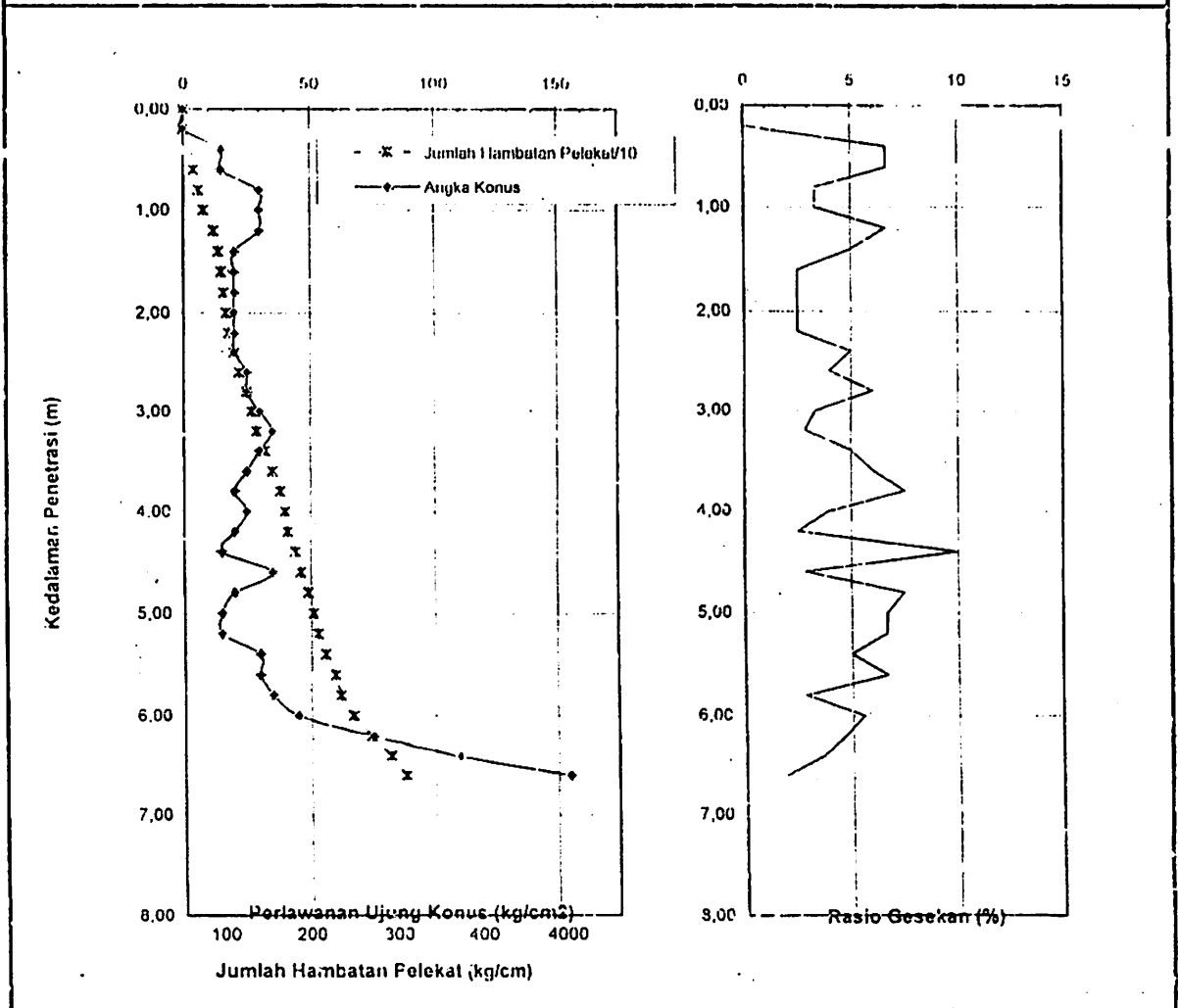


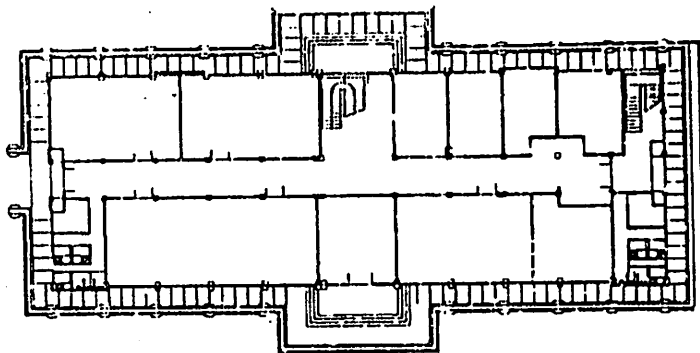
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**UNIVERSITAS NEGERI MALANG**

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Proyek : PEMBANGUNAN GEDUNG RUSUNAWA Tgl. Uji : 03.05.2009 TITIK UJI : S3  
Lokasi : JL. SURABAYA 6 MALANG Dikerjakan : St dan Ww  
Diperiksa : MS

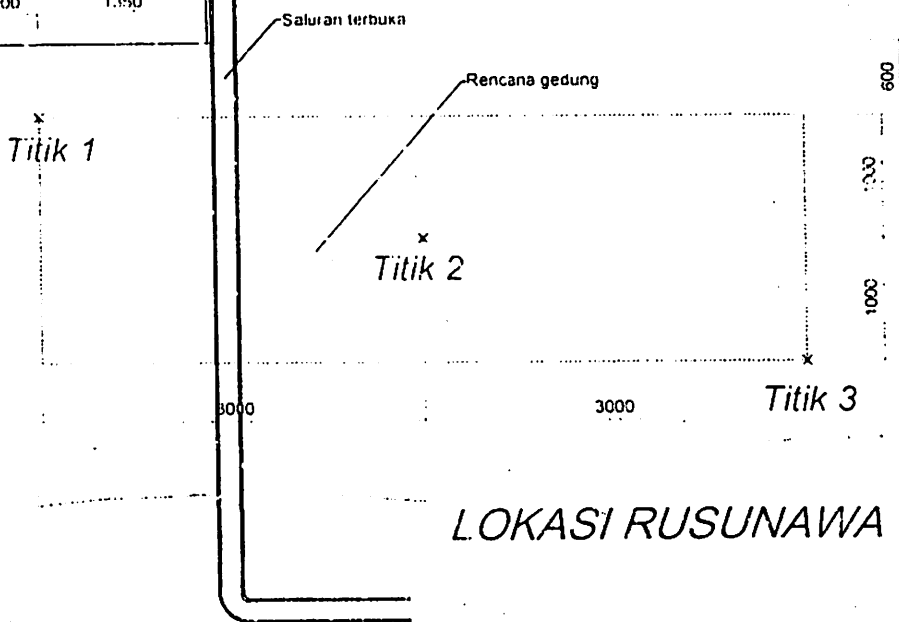
**UJI PENETRASI KONUS (CONE PENETRATION TEST)**  
**ASTM D 3441 - 86**





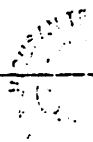
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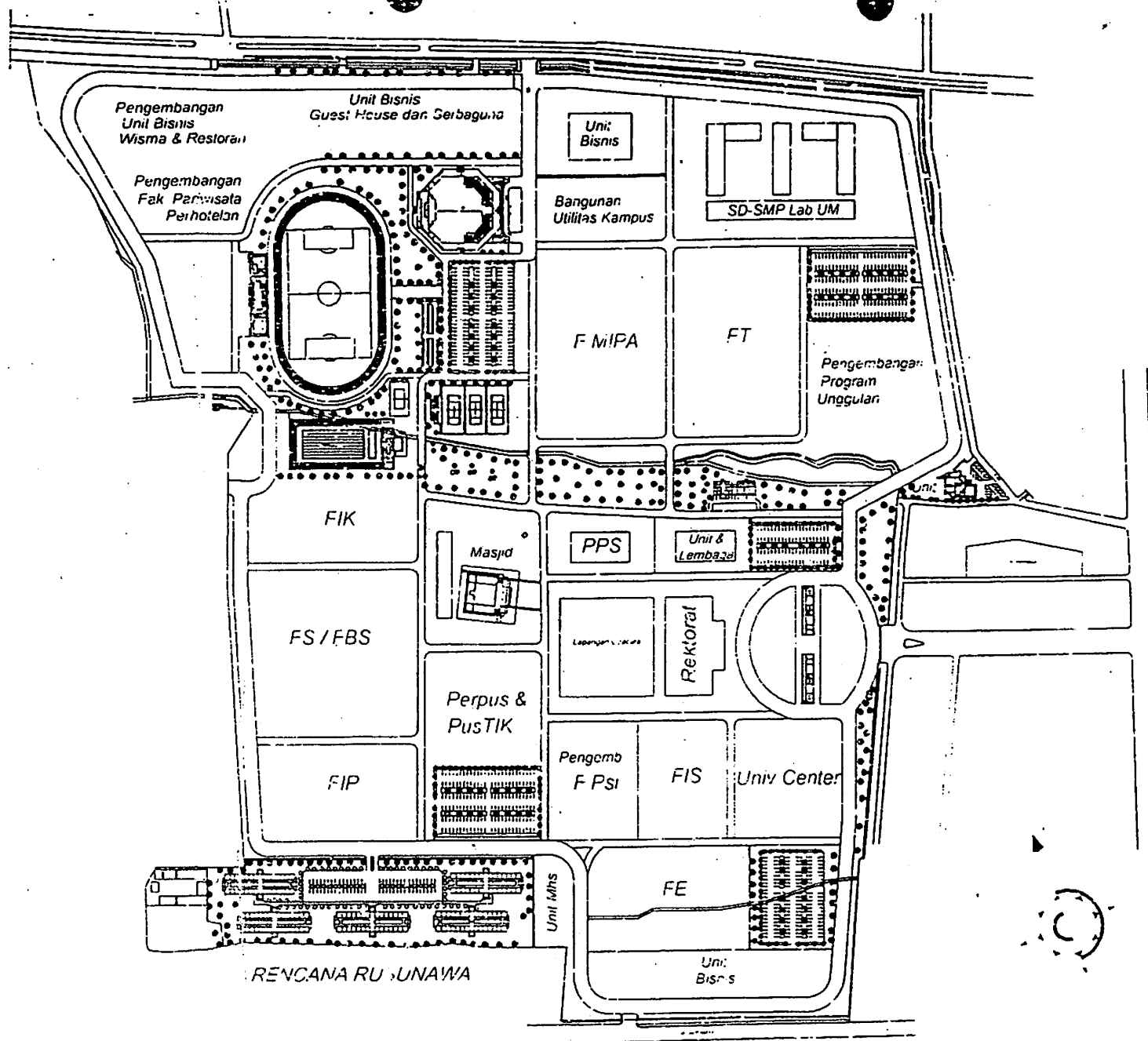
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LOKASI RUSUNAWA

LETAK TITIK SONDIR





RENCANA RU UNAWA

LAY OUT ZONA PENGEMBANGAN