SKRIPSI

STUDI PERENCANAAN PORTAL STRKTUR GEDUNG DENGAN DASAR SNI 03-1726-2012 DAN SNI 03-2847-2013 PADA HOTEL ARIA CENTRA SURABAYA

Disusun Oleh :

EKO BUDI SUSANTO

10.21.072

PROGRAM STUDI TEKNIK SIPIL S-1
FAKULTAS TEKNIK SIPIL DAN PERENCANAAN
INSTITUT TEKNOLOGI NASIONAL
MALANG

2016

LEMBAR PERSETUJUAN
SKRIPSI

STUDI PERENCANAAN PORTAL STRUKTUR GEDUNG DENGAN DASAR SNI 03-1726-2012 DAN SNI 03-2847-2013 PADA HOTEL ARIA CENTRA SURABAYA

Diajukan Sebagai Salah Satu Syarat Memperoleh Gelar Sarjana Teknik Sipil S–1
Institut Teknologi Nasional Malang

Disusun Oleh :

Eko Budi Susanto
10.21.072

Disetujui Oleh :

Dosen Pembimbing I

(Ir. A. Agus Santosa, MT)

Mengetahui,

Ketua Program Studi Teknik Sipil S-1

Institut Teknologi Nasional Malang

(Ir. A. Agus Santosa, MT)

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LEMBAR PENGESAHAN

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S K R I P S I

Dipertahankan dihadapan dewan penguji skripsi Strata Satu (S-1)
Pada Hari : Rabu
Tanggal : 21 Februari 2016
Dan diterima untuk memenuhi persyaratan memperoleh gelar
Sarjana Teknik Sipil

Disusun Oleh :

Eko Budi Susanto

NIM : 1021072

Disahkan Oleh :

Ketua Program Studi
Teknik Sipil S-1

(Ir. A. Agus Santosa, MT)

Sekretaris Program Studi
Teknik Sipil S-1

(Ir. Munasih, MT)

Anggota Penguji :

Dosen Penguji I

(Ir. Bambang Wedyantadji, MT)

Dosen Penguji II

(Ir. Eding Iskak Imananto, MT)

PROGRAM STUDI TEKNIK SIPIL S-1

FAKULTAS TEKNIK SIPIL DAN PERENCANAAN

INSTITUT TEKNOLOGI NASIONAL MALANG

2016

PERNYATAAN KEASLIAN SKRIPSI

Saya yang bertanda tangan di bawah ini :

Nama	**: EKO BUDI SSANTO**
NIM	**: 10.21.072**
Program Studi	**: Teknik Sipil S-1**
Fakultas	**: Teknik Sipil dan Perencanaan**

Menyatakan dengan sesungguhnya bahwa skripsi saya yang berjudul :

"STUDI PERENCANAAN PORTAL STRUKTUR GEDUNG DENGAN DASAR SNI 03-1726-2012 DAN SNI 03-2847-2013 PADA HOTEL ARIA CENTRA SURABAYA"

Adalah hasil karya saya sendiri dan bukan merupakan duplikat serta mengutip atau menyadur seluruhnya dari hasil karya orang lain, kecuali yang disebut dari sumber aslinya dan tercantum dalam daftar pustaka.

Malang, Oktober 2018

...at pernyataan

METERAI TEMPEL 3F386AFF345614163 6000 ENAM RIBU RUPIAH

(Eko Budi Susanto)

"Studi Perencanaan Portal Struktur Gedung Dengan Dasar SNI 03-1726-2012 Pada Hotel Aria Centra Surabaya". Oleh : Eko Budi Susanto, (1021072). Skripsi, Program Studi Tenik Sipil S-1, Institut Teknologi Nasional Malang.Pembimbing: Ir. A. Agus Santosa, MT,

ABSTRAK

Merupakan kewajiban bagi estimator untuk merencanakan sebuah bangunan yang aman sekaligus efisien. Hal ini di karenakan seorang estimator sebuah bangunan memiliki tanggung jawab yang besar terhadap keamanaan struktur bangunan yang akan dibangun.

SNI 03-2847-2013 dan SNI 03-1726-2012 merupakan salah satu standar yang harus di mengerti dan dipahami oleh seorang estimator untuk merencanakan bangunan. Namun dikarenakan perkembangan zaman standar tersebut mengalami perubahan dari segi persyaratan maupun tata cara perencanaan, meskipun tidak banyak perubahan yang di hasilkan di setiap amandemen standar yang terbaru. Masih banyak estimator merencanakan sebuah bangunan dengan standard dan metode yang sebelumnya.

Sehubungan dengan hal diatas direncanakan kembali struktur portal Gedung hotel Aria Centra Surabaya yang meliputi : balok, kolom, joint rangka. dengan menggunkan SNI 03-2847-2013 dan SNI 03-1726-2012. Dengan analisa statikanya menggunakan program bantu STAADpro.

Dengan demikian struktur ini diaharapkan mampu mempunyai ketahanan terhadap gempa sesuai SNI 03-1726-2012 tentang Tata Cara Perencanaan Ketahanan Gempa Untuk Bangunan Gedung, serta memenuhi persyaratan yang di isyaratkan SNI 3-2847-2013 tentang Cara Perencanaan Struktur Beton untuk Bangunan Gedung.

Untuk mencapai kondisi seperti diatas diperlukan sistematika tahab dalam pembelajaran paduan pedoman. Untuk desain detail tulangan yang benar dan harus disesuaikan dengan panduan yang ada.

Kata Kunci : Struktur Portal, Tahan Gempa, SNI 3-2847-2013, 03-1726-2013, Kota Malang

KATA PENGANTAR

Dengan mengucapkan syukur kehadirat Allah SWT yang telah memberikan rahmat, taufik serta hidayahnya sehingga penulis dapat menyelesaikan Skripsi dengan Judul **"Studi Perencanaan Portal Struktur Gedung Dengan Dasar SNI 03-1726-2012 Pada Hotel Aria Centra Surabaya"** dengan baik dan tepat waktu.

Tak lepas dari berbagai hambatan, rintangan, dan kesulitan yang muncul, namun berkat petunjuk dan bimbingan dari semua pihak yang telah membantu penulis sehingga dapat menyelesaikan tugas besar ini. Sehubungan dengan hal tersebut dalam kesempatan ini penulis menyampaikan rasa hormat dan terima kasih yang sebesar- besarnya kepada:

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Malang, Januari 2016

Penulis

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BAB I

PENDAHULUAN

1.1 Latar Belakang

Indonesia merupakan negara yang berada di daerah pertemuan tiga pelat/lempeng tektonik bumi, yaitu lempeng pasifik, samudra hindia, eurasia dan filipina. Dengan itu akan terdapat banyak daerah rawan gempa di indonesia dengan bermacam-macam kekuatan gempa. Pada daerah yang berada di titik rawan gempa, sangat memerlukan perhatian khusus dari segi keamanan penduduk, pembangunan agar terciptanya kenyamanan pada penduduk yang tinggal di daerah tersebut. Pada peristiwa gempa bumi tidak dapat di cegah melainkan diantisipasi, maka perlu adanya pembangunan yang akan tahan terhadap gempa bumi.

Untuk perkembangan yang ada di indonesia sangatlah pesat dan terus membangun seperti halnya negara lain yang maju. Terutama Pembangunan gedung-gedung bertingkat yang selalu ada di seluruh daerah indonesia. Banyaknya kecelakaan terjadi ketika gempa bumi yang berbuah makan banyak nyawa manusia, salah satunya dari pembangunan gedung bertingkat yang tidak kuat menahan beban gempa bumi. Dengan ini pada perencanaan pada gedung bertingkat memperhatikan beberapa kriteria, antara lain kriteria kekuatan, perilaku struktur yang baik pada taraf gempa rencana serta aspek ekonomis. Perhatian yang matang perlu dilakukan dalam perencanaan pembangunan gedung bertingkat dasi segala aspek teknis maupun nonteknis.

Standart Nasional Indonesia (SNI) sangat berperan penting dalam melakukan perencanaan pembangunan terutama pada pembangunan gedung bertingkat. Banyaknya oknum yang tidak bertanggung jawab sebagai salah satu bencana atas ketidaknyamanan penduduk dari segala pembangunan yang tidak memenuhi Standart Nasional Indonesia (SNI). Maka dengan itu penulis akan membahas dalam bentuk studi yang berjudulkan "STUDI PERNCANAAN PORTAL STRUKTUR GEDUNG DENGAN DASAR SNI-

03-1726-2012 DAN SNI-03-2847-3013 PADA HOTEL ARIA CENTRA-SURABAYA".

1.2 Rumusan Masalah

1. Masuk Kategori Desain Seismik apa gedung Hotel Aria Centra-Surabaya dengan panduan SNI-03-1726-2012 ?
2. Berapa dimensi penampang balok dan kolom struktur gedung Hotel Aria Centra-Surabaya dengan panduan SNI 03-1726-2012 ?
3. Berapa jumlah tulangan yang dibutuhkan pada struktur gedung Hotel Aria Centra-Surabaya ?

1.3 Tujuan Penelitian

1. Menganalisa Kategori Desain Seismik pada perencanaan struktur penahan gempa dengan panduan SNI-031726 2012
2. Menganalisa dimensi penampang kolom dan balok
3. Menganalisa jumlah tulangan yang dibutuhkan pada struktur gedung Hotel Aria Centra-Surabaya.

1.4 Manfaat Penelitian

Dari studi perencanaan portal struktur gedung dengan ini diharapkan menghasilkan suatu perencanaan struktur yang tepat dengan mengkodisikan tempat pembangunan yang direncanakan dengan sesuai standart yang ada sehingga dapat menghasilkan :

1. Didapatkan konstruksi gedung yang sesuai dengan kondisi wilayah setempat dan peraturan yang berlaku di indonesia.
2. Sebagai bahan pertimbangan bagi pembaca dan masyarakat umum dalam perencanaan pembangunan.
3. Dengan perencanaan struktur gedung yang sesuai dengan prosedur perhitungan dan memiliki kemampuan untuk menahan berbagai jenis gaya yang bekerja, bisa menciptakan keamanan dan kenyamanan.
4. Menambah wawasan penulis tentang perencanaan struktur tahan gempa.

1.5 Batasan Masalah

Batasan masalah dalam Studi Perencanaan portal Struktur Gedung menggunakan panduan SNI-03-1726-2012 dan SNI-03-2847-2013 Pàda perencanaan struktur Hotel Aria Centra Surabaya adalah :

1. Penulangan dan sambungan struktur yang dihitung hanya balok dan kolom.
2. Portal yang dianalisa line 4.
3. Perencanaan portal pada struktur gedung berdasarkan dengan SNI 03-1726-2012 dan SNI 03-2847-2013.
4. Untuk analisis struktur portal gedung ini menggunakan program bantu komputer yaitu STAADPRO

BAB II

TINJAUAN PUSTAKA

2.1 Uraian Umum

Obyek atau proyek yang digunakan untuk perhitungan alternatif perencanaan struktur ini adalah hotel aria centra-surabaya. Proyek yang ditinjau ini memiliki enam belas lantai yang merupakan gedung bertingkat. Sehingga, dalam melakukan perencanaan gedung ini harus memperhatikan beberapa aspek, dan kriteria-kriteria yang harus diperhatikan seperti :

a. Kekakuan

Suatu struktur harus memiliki kekakuan yang cukup sehingga pergerakkannya dapat dibatasi. Kekakuan struktur dapat diukur dari besarnya simpangan antar lantai (drift) bangunan, semakin kecil simpangan struktur maka bangunan tersebut akan semakin kaku (Smith dan Coull, 1991).

b. Kekuatan

Syarat kekuatan ini mencakup seluruh elemen struktur, baik pelat, kolom, balok, dan dinding geser. Cara mengontrol sesuai dengan perilaku elemen-elemen tersebut. Misalnya kolom, dicari terlebih dahulu diagram interaksi dan tentukan dimana titik Pu,Mu maksimum pada diagram interaksi tersebut, jika titik tersebut berada di luar dan di bawah keadaan *balance* (seimbang), maka terjadi kegagalan tarik. Jika berada di luar sebelah atas keadaan *balance* (seimbang) maka terjadi kegagalan tekan.

c. Kestabilan

Kolom atau dinding geser dapat mengalami tekuk atau *buckling*, keadaannya pun berbeda-beda, namun jika kolom atau dinding geser tersebut dapat kembali pada keadaan semula maka kolom atau dinding geser tersebut dapat dikatakan stabil.

2.2 Pembebanan Komponen Struktur

Dalam perencanaan suatu struktur bangunan harus memenuhi peraturan – peraturan yang berlaku untuk mendapatkan suatu struktur bangunan yang aman secara kontruksi. Struktur bangunan yang direncanakan harus mampu menahan beban mati, beban hidup dan beban gempa yang bekerja pada struktur bangunan tersebut.

2.2.1 BebanMati

Beban mati adalah berat dari semua bagian dari suatu gedung yang bersifat tetap, termasuk segala unsur tambahan, penyelesaian- penyelesaian (*finishing*), mesin-mesin, serta peralatan tetap yang merupakan bagian yang tak terpisahkan dari gedung.

Berat material bangunan tergantung dari bahan jenis bangunan yang dipakai. Beban mati tambahan adalah beban yang berasal dari finishing lantai (keramik, plester), beban dinding dan beban tambahan lainnya.

2.2.2 Beban Hidup

Beban hidup adalah semua beban yang terjadi akibat penghunian atau penggunaan suatu gedung, dan termasuk beban-beban pada lantai yang berasal dari barang-barang yang berpindah, mesin-mesin serta peralatan yang tidak merupakan bagian yang tak terpisahkan dari gedung dan dapat diganti selama masa hidup dari gedung itu, sehingga mengakibatkan perubahan dalam pembebanan atap dan lantai tersebut.

2.2.3 Beban Gempa

Beban gempa adalah semua beban yang ditimbulkan dari gerakan-gerakan lapisan bumi ke arah horisontal dan vertikal, dimana gerakan vertikalnya lebih kecil dari gerakan horisontalnya.

2.2.3.1 Metoda Analisa Gempa Dinamis

2.2.3.1.1 Ketentuan Untuk analisis respon dinamik

1. Untuk struktur gedung tidak beraturan, pengaruh gempa rencana terhadap struktur gedung tersebut harus ditentukan melaluai analisis respon dinamik 3 dimensi. Untuk mencegah terjadinya respons struktur gedung terhadap pembebanan gempa yang dominant dalam rotasi, dari hasil analisis vibrasi bebas 3 dimensi, paling tidak gerak ragam pertama (fundamental) harus dominant dalam translasi.
2. Daktilitas struktur tidak beraturan harus ditentukan yang representative mewakili daktilitas struktur 3D. Tingkat daktilitas tersebut dapat dinyatakan dalama factor reduksi gempa untuk2 arah sumbu koordinat orthogonal dengan gaya geser dassar yang dipikul olehstruktur gedung dalam masing-masing arah tersebut sebagai besaran pembobotnya menurut persaman :

$$R = \frac{V_x^2 + V_y^2}{V_x^0 / R_x + V_y^0 / R_y}$$

   Dimana R_x dan V_x^0 adalah factor reduksi gempa dan gaya geser dasar untuk pembebanan gempa dalam arah sumbu-x, sedangkan R_y dan V_y^0 adalah factor reduksi gempa dan gaya geser dasar untuk pembebanan gempa dalam aray sumbu –y, metoda ini hanya boleh dipakai, apabila rasio antara nilai-nilai factor reduksi gempa 2 arah pembebanan gempa terssebut tidak lebih dari 1,5.
3. Nilai akhir respon s dinamik struktur gedung terhadap pembebanan gempa niminal akibat pengaruh gempa rencana dalam suatu arah tertentu, tidak bolehdiambilkurang dari 80% nilai respons ragam yang pertama. Bila respons dinamik struktur gedung dinyatakan menurut persamaan berikut

$$V \geq 0,8\, V_1$$

   Dimana V_1 adalah gaya geser dassar nominal sebagai respons ragam yang pertama terhadap pengaruh Gempa Rencana menurut persamaan :

$V = CsxWt$

Dimana:

V = Beban geser dasar nominal

C_s = Koefisien respon seismic $(Sds/R.Ie)$

Wt = Berat Total Gedung

2.2.3.1.2 Analisa Ragam Spectrum Respons

1. Perhitungan respons dinamik struktur geedung tidak beraturan terhadap pembebanan gempa nominal akibat pengaruh gempa rencana,dapat dilakukan dengan metoda analisa ragam spectrum respons dengan memakai Spectrum Respons Rencana menurut gambar 2 yang nilai ordinatnya dikalikan factor koreksi I/R, dimana I adalah factor reduksi gempa representative dari struktur gedung yang besangkutan. Dalam hal ini, jumlah ragam vibrasi yang ditinjau dalam penjumlahan respons ragam menurut metoda ini harus sedemikian rupa, sehingga partisipasi massa dalam menghasilkan respons total haruss mencapai sekurang-kurangnya 90%.
2. Penjumlahan respons ragam yang disebut dalam nol untuk struktur gedung tidak beraturan yang memiliki watu-waktu getar alami, yang berdekatan harus dilakukan dengan metoda yang dikenal dengan kombinasi kuadralik lengkap (Complete Quadratic Combination atau CQC). Waktu getar alami ini harus dianggap berdekatan, apabila selisih nilainya kurang dari 15%. Untuk struktur gedung tidak beraturan yang memiliki waktu getar alami yang berjauhan, penjumlahan respons ragam tersebut dapat dilakukan dengan metoda yang dikenal dengan Akar Jumlah Kuadrat (Square Root of the Sum Square atau SRSS).
3. Untuk memenuhi persyaratan menurut no.1, maka gaya geser tingkat nominal akibat pengaruh gempa rencana sepanjang tinggi struktur gedung hasil analisis regam spectrum respons dalam suatu arah tertentu, harus dikalikan nilainya dengan suatu factor skala : $\frac{0{,}8.V_1}{V_1} \geq 1$

Dimana :

V_1 = Gaya geser dasar nominal sebagai respons dinamik ragam yang pertama saja

V_t = Gaya geser dasar nominal yang didapat dari hasil analisis ragam spectrum respons yang telah dilakukan.

4. Bila diinginkan , dari diagram atau kurva gaya geser tingkat nominal akibat pengaruh gempa rencanasepanjang tinggi struktur gedung yang telah disesuaikan nilainya menurut no 3 dapat ditentukan beban-beban gempa nominal static ekuivalen yang besangkutan (selisih gaya geser tingkat dari 2 tingkat berturut-turut), yang bila perlu diagram atau kurvanya dimodifikasi terlebih dulu secara konsevatif untuk mendapatkan pembagian beban-beban gempa nominal static ekuivalen ini kemudian dapat dipakai dalam analisis static ekuivalen 2 dimensi biasa.

2.2.3.1.3 Analisa Respons Dinamik Riwayat Waktu

1. Bila diinginkan, perhitungan respons dinamik struktur gedung tidak beraturan terhadap pengaruh gempa rencana, dapat dilakukan dengan metoda analisis dinamik 3 dimensi berupa analisis respons dinamik linier dan non-linier riwayat waktu dengan suatu akselerogram gempa yang diangkakan sebagai gerakan tanah masukan.
2. Untuk perencanaan strutur gedung melalui analisis dinamik linier riwayat waktu terhadap pengaruh gempa rencana pada taraf pembebanan gempa nominal, percepatan muka tanah asli dari gempa masukan harus diskalakan ketaraf pembebanan gempa nominal tersebut, sehingga nilai percepatan puncaknya A menjadi :

$$A = \frac{A_0 . I}{R}$$

Dimana:

A_0 =Percepatan puncak muka tanah

R =Faktor reduksi representative dari struktur gedung yang bersangkutan

I = Faktor keutamaan

Selanjutnya harus dipenuhi juga persyaratan pada 2.4.1.3 dan untuk itu factor skala yang dipakai adalah sama seperti yang ditentukan dalam 2.4.2.3, hanya V_t disini merupakan gaya geser dasar maksimum yang terjadi di tingkat dassar yang didapat dari hasil analisis respons dinamik riwayat waktu yang telah dilakukan. Dalam analisis ini redaman struktur yang harus diperhitungkan dapat dianggap 5%.dari reedaman kritis.

3. Untuk mengkaji perilaku pasca-elastik struktur gedung terhadap pengaruh gempa rencana, harus dilakukan analisis respons dinamik non-linier riwayat, dimana percepatan muka tanah asli dari gempa masukan harus diskalakan, sehingga nilai percepatan puncaknya menjadi ssama dengan A_0 I dimana A_0 adalah percepatan puncak muka tanah dan I adalah factor keutamaan.
4. Akselogram gempa masukan yang ditinjau dalam analisis respons dinamik linier dan non-linier riwayat waktu, harus diambil dari rekaman gerakan tanah akibat gempa yang didapat disuatu lokasi yang mirip kondisi geologi, topografi dan seismotektoniknya dengan lokasi tempat struktur gedung yang ditinjau berada. Untuk mengurangi ketidakpastian mengenai kondisi lokasi ini, paling sedikit harus ditinjau 4 buah akselerogram dari 4 gempa yang berbeda, salah satunya harus diambil akselerogram gempa ElCentroN-S yang telah direkam pada tanggal 15 mei 1940 di California.
5. Berhubungan gerakan tanah akibat gempa pada suatu lokasitidak mungkin dapatdiperkirakan dengan tepat, maka sebagai gempa masukan dapat juga dipakai gerakan tanah yang disimulasikan parameter-parameter yang menentukan gerakan tanah yang disimulasikan ini antara lain terdiri dari waktu getar predominan tanah, konfigurasi spectrum respons, jangka waktu gerakan dan intensitas gempanya.

2.3 Kombinasi Pembebanan

Sesuai dengan ketentuan yang tentera dalam SNI 03 – 2847 – 2013 pasal 9.2, agar struktur dan komponen struktur harus direncanakan hingga semua penampang mempunyai kuat rencana minimum sama dengan kuat perlu, yang dihitung berdasarkan kombinasi dan gaya terfaktor.

U = 1,4D

U = 1,2D + 1,6L + 0,5(Lr atau R)

U = 1,2D + 1,6(Lr atau R) + (1,0L atau 0,5W)

U = 1,2D + 1,0W + 1,0L + 0,5(Lr atau R)

U = 1,2D + 1,0E + 1,0L

U = 0,9D + 1,0W

U = 0,9D + 1,0E

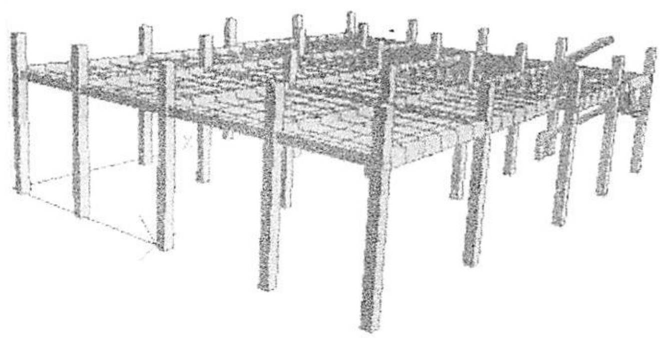
2.4 Struktur Tahan Gempa

Menurut SNI 03-2847-2013 pasal 21.1.1.2 Struktur harus ditetapkan sebagai suatu kategori desain seismik (KDS) sesuai dengan SNI 1726:2012 yang di adopsi secara legal dimana standar ini merupakan bagianya, atau ditetapkan oleh otoritas berwenang lainya di daerah tanpa tata cara bangunan gedung.

Yang mana semua struktur harus memenuhi persyaratan dari pasal 1 hingga 19 dan 22. Struktur yang ditetapkan seebagai KDS B, C, D, E, atau F juga harus memenuhi 21.1.1.4 hingga 21.1.1.8, Sebagamaimana sesuai dengan tabel 2.1. Untuk daerah malang ditentukan sebagai daerah dengan kategori desain seismik (KDS) "D"(penentuan ketegori pada Bab III).

Analisis struktur terhadap beban gempa mengacu pada Standar Perencanaan Ketahanan Gempa untuk Rumah dan Gedung (SNI 03- 1726-2012). Pada Skripsi kali ini Analisis struktur terhadap beban gempa pada gedung dilakukan dengan metode analisis Statik Ekuivalen dengan mencari Pusat Massa (CG) dan Berat Tiap Lantai.

Untuk mencari berat lantai tiap lantai dengan bantuan program bantu staad pro, dengan cara memotong tiap lantai dengan ketinggian kolom setengahnya keatas terhadap lantai yang ditinjau dan setengahnya tinggi kolom kebawah terhadap lantai yang ditinjau. Kemudian salah satu titik ujung bawah kolom diberi tumpuan jepit untuk mengetahui reaksi yang nantinya menjadi berat lantai tersebut. Mencari pusat massa pada masing-masing lantai dengan menambahkan perintah CG *(Center Gravity)* melilih *command* lalu pilih *post-analysis print*, klik *CG*.



Sumber : SNI-03-1726-2013, Pasal : 21.1

Gambar 3.2 Mencari Pusat Massa dan Berat lantai 1

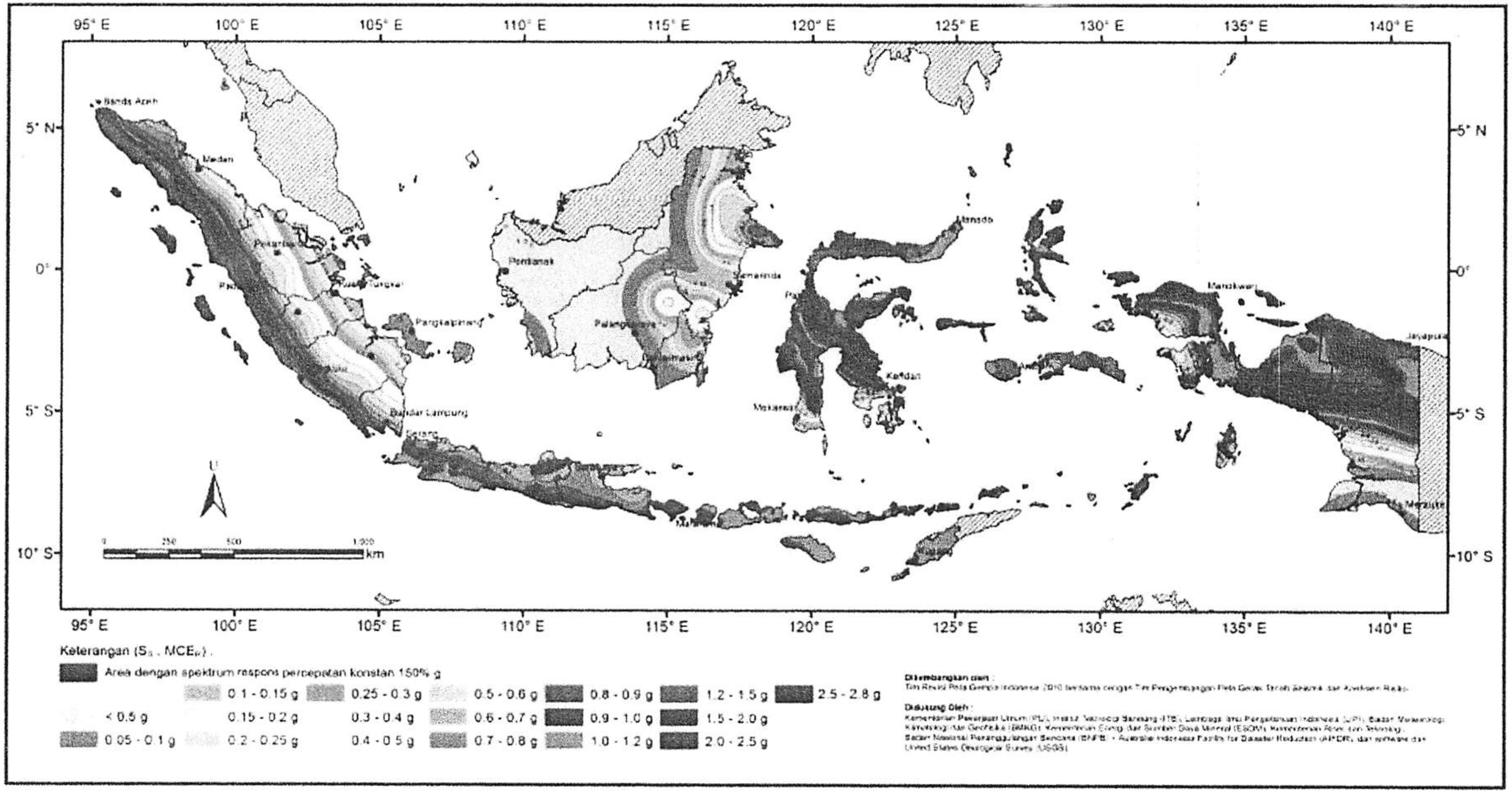
Untuk hasil perhitungan pusat masa dan berat tiap lantai dengan cara seperti diatas bisa dilihat dari tabel bab III di perhitungan berat tiap lantai.

Tabel 2.1 Aplikasi Tipikal Kategori Desain Seismik

Komponen yang menahan pengaruh gempa, kecuali jika sebaliknya diberitahu	Kategori Desain Seismik			
	Komponen yang menahan pengaruh	(Tidak ada) B (21.1.1.4)	C (21.1.1.5)	D, E, F (21.1.1.6)
Persyaratan analisis dan desain	Tidak ada	21.1.2	21.1.2	21.1.2, 21.1.3
Material		Tidak ada	Tidak ada	21.1.4-21.1.7
Komponen struktur rangka		21.2	21.3	21.5, 21.6, 21.7, 21.8
Dinding struktru dan balok kopel		Tidak ada	Tidak ada	21.9

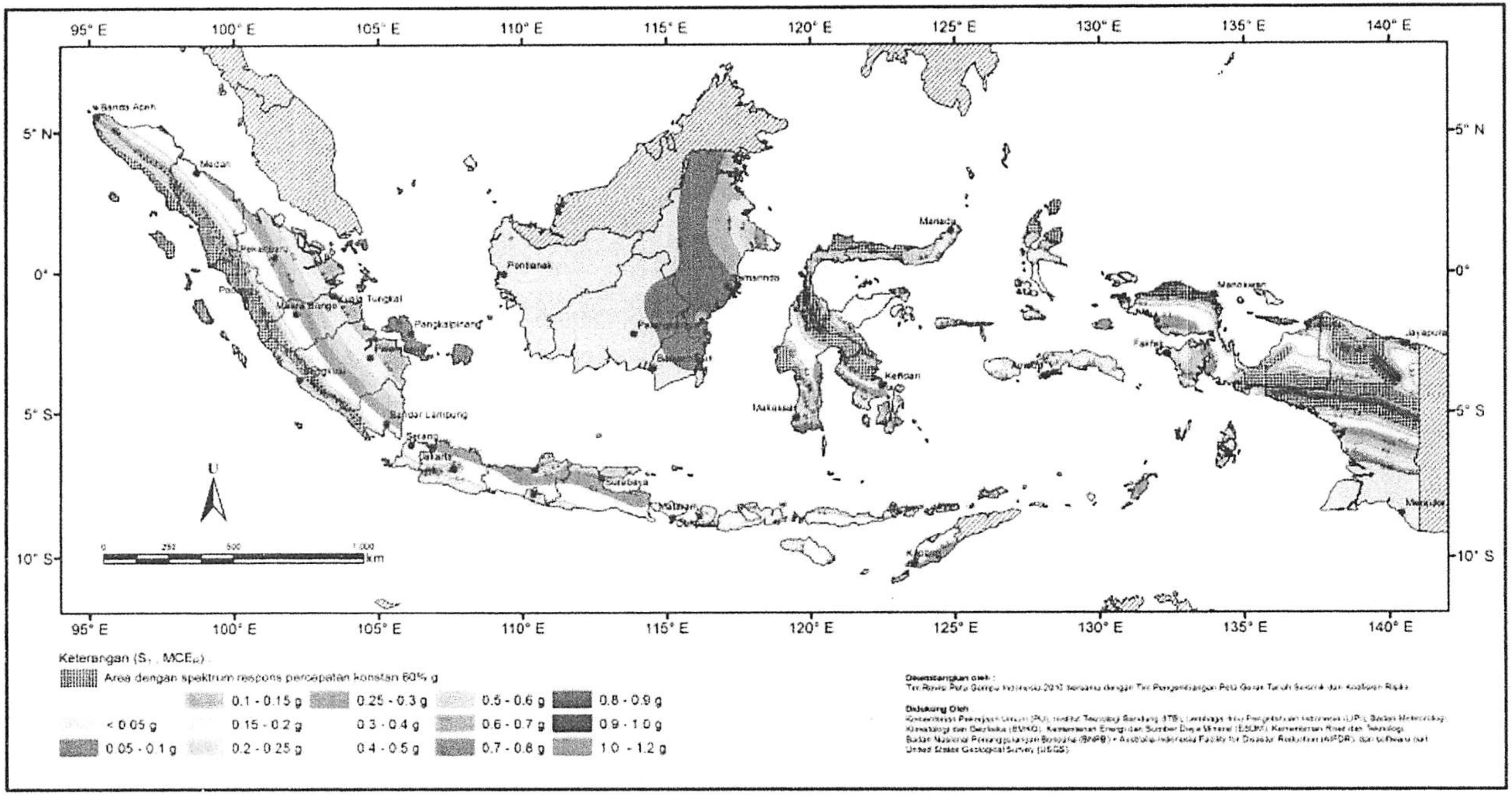
Dinding struktur pracetak		Tidak ada	21.4	† 21.4, 21.10
Diafrragma struktur dan rangka batang (*trusses*)		Tidak ada	Tidak ada	21.11
Fondasi		Tidak ada	Tidak ada	21.12
Komponen struktur rangka tidak diproporsikan untuk menahan gaya yang ditimbulkan oleh pergerakan gempa		Tidak ada	Tidak ada	21.13
Angkur		Tidak ada	21.1.8	21.1.8
*Sebagai tambahan pada persyaratan Pasal 1 sampai 19, kecuali seperti dimodifikasi oleh Pasal 21. Sub Pasal 22.10 juga berlaku dalam KDS D, E, dan F. † Seperti diizinkan oleh tata cara bangunan umum yang diadopsi secara legal dimana Standar ini merupakan bagiannya.				

Sumber : SNI-03-2847-2013, Pasal : 21.1.3



Sumber : SNI-03-1726-2012, Pasal : 14

Gambar 2.1 Peta respon spektra percepatan 0.2 detik (S_s) di batuan dasar (S_B) untuk probabilitas terlampaui 2% dalam 50 tahun.



Sumber : SNI-03-1726-2012, Pasal : 14

Tabel 1. Kategori resiko bangunan gedung dan non gedung untuk gempa

Jenis pemanfaatan	Kategori risiko
Gedung dan non gedung yang memiliki risiko rendah terhadap jiwa manusia pada saat terjadi kegagalan, termasuk, tapi tidak dibatasi untuk, antara lain: - Fasilitas pertanian, perkebunan, perternakan, dan perikanan - Fasilitas sementara - Gudang penyimpanan	I
Semua gedung dan struktur lain, kecuali yang termasuk dalam kategori risiko I,III,IV, termasuk, tapi tidak dibatasi untuk: - Perumahan - Rumah toko dan rumah kantor - Pasar - Gedung perkantoran - Gedung apartemen/ rumah susun - Pusat perbelanjaan/ mall - Bangunan industri	II
Gedung dan non gedung yang memiliki risiko tinggi terhadap jiwa manusia pada saat terjadi kegagalan, termasuk, tapi tidak dibatasi untuk: - Bioskop - Gedung pertemuan - Stadion - Fasilitas kesehatan yang tidak memiliki unit bedah dan unit gawat darurat - Fasilitas penitipan anak - Penjara - Bangunan untuk orang jompo Gedung dan non gedung, tidak termasuk kedalam kategori risiko IV, yang memiliki potensi untuk menyebabkan dampak ekonomi yang besar dan/atau gangguan massal terhadap kehidupan masyarakat sehari-hari bila terjadi kegagalan, termasuk, tapi tidak dibatasi untuk: - Pusat pembangkit listrik biasa - Fasilitas penanganan air - Fasilitas penanganan limbah - Pusat telekomunikasi Gedung dan non gedung yang tidak termasuk dalam kategori risiko IV, (termasuk, tetapi tidak dibatasi untuk fasilitas manufaktur, proses, penanganan, penyimpanan,	III

Sumber : SNI-03-1726-2012, Pasal : 4.1.2

Tabel 1. Kategori resiko bangunan gedung dan non gedung untuk gempa (lanjutan)

Jenis pemanfaatan	Kategori resiko
Gedung dan non gedung yang di tunjukan sebagai fasilitas yang penting, termasuk, tetapi tidak dibatasi untuk : - Bangunan-bangunan monumental - Gedung sekolah dan fasilitas pendidikan - Rumah sakit dan fasilitas kesehatan lainya yang memiliki fasilitas bedah dan unit gawat darurat - Fasilitas pemadam kebakaran, ambulans, dan kantor polisi, serta garasi kendaraan darurat lainnya - Fasilitas kesiapan darurat, komunikasi, pusat operasi dan fasilitas lainya untuk tanggap darurat - Pusat untuk pembangkit energi dan fasilitas publik lainnya yang dibutuhkan pada saat kendaraan darurat - Struktur tambahan (termasuk menara telekomunikasi, tangki penyiapan bahan bakar, menara pendingin, struktur stasiun listrik, tangki air pemadam kebakaran atau struktur rumah atau struktur prndukung air atau material atau peralatan pemadam kebakaran) yang disyaratkan untuk beroprasi pada saat keadaan darurat Gedung dan non gedung yang dibutuhkan untuk mempertahankan fungsi struktur bangunan lain yang masuk ke dalam kategori resiko IV.	IV

Sumber : SNI-03-1726-2012, Pasal : 4.1.2

Tabel 2 – Faktor keutamaan gempa

Kategori resiko	Faktor keutamaan gempa *Ie*
I atau II	1,0
III	1,25
IV	1,50

Sumber : SNI-03-1726-2012, Pasal : 4.1.2

Tabel 4. Koefisien situs Fa

Kelas situs	Parameter respons spektral percepatan gempa (MCE_R) terpetakan pada perioda pendek, T=0,2 detik, S_s				
	$S_s \le 0,25$	$S_s = 0,5$	$S_s = 0,75$	$S_s = 1,0$	$S_s \ge 1,25$
SA	0,8	0,8	0,8	0,8	0,8
SB	1,0	1,0	1,0	1,0	1,0
SC	1,2	1,2	1,1	1,0	1,0
SD	1,6	1,4	1,2	1,1	1,0
SE	2,5	1,7	1,2	0,9	0,9
SF			SS^b		

CATATAN:

(a) Untuk nilai-nilai antara S_s dapat dilakukan interpolasi linier

(b) SS= Situs yang memerlukan investigasi geoteknik spesifik dan analisis respons situs-spesifik, lihat 6.10.1

Sumber : SNI-03-1726-2012, Pasal : 6.2

Tabel 5. Koefisien situs Fv

Kelas situs	Parameter respons spektral percepatan gempa MCE_R terpetakan pada perioda 1 detik, S_1				
	$S_1 \le 0,1$	$S_1 = 0,2$	$S_1 = 0,3$	$S_1 = 0,4$	$S_1 \ge 0,5$
SA	0,8	0,8	0,8	0,8	0,8
SB	1,0	1,0	1,0	1,0	1,0
SC	1,7	1,6	1,5	1,4	1,3
SD	2,4	2	1,8	1,6	1,5
SE	3,5	3,2	2,8	2,4	2,4
SF			SS^b		

CATATAN :

(a) Untuk nilai-nilai antara S_1 dapat dilakukan interpolasi linier

(b) SS= Situs yang memerlukan investigasi geoteknik spesifik dan analisis respons situs-spesifik.lihat 6.10.1

Sumber : SNI-03-1726-2012, Pasal : 6..2

Tabel 6. Kategori dedsain seismic berdasarkan parameter respons percepatan pada periode pendek.

Nilai S_{DS}	Kategori risiko	
	I atau II atau III	IV
$S_{DS} < 0,167$	A	A
$0,167 \le S_{DS} < 0,33$	B	C
$0,33 \le S_{DS} < 0,50$	C	D
$0,50 \le S_{DS}$	D	D

Sumber : SNI-03-1726-2012, Pasal : 6.5

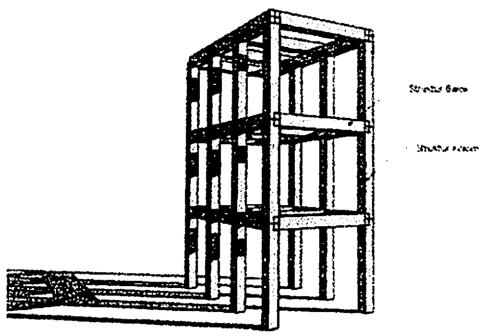
Tabel 7. Kategori desain seismik berdasarkan parameter respons percepatan pada periode 1 detik.

Nilai S_{D1}	Kategori risiko	
	I atau II atau III	IV
$S_{D1} < 0,167$	A	A
$0,067 \le S_{D1} < 0,133$	B	C
$0,133 \le S_{D1} < 0,20$	C	D
$0,20 \le S_{D1}$	D	D

Sumber : SNI-03-1726-2012, Pasal : 6.5

2.5 Struktur Rangka Terbuka

Untuk konsep rangka terbuka yang menjadi elemen struktur adalah struktur balok dan struktur kolom. Jadi pada struktur rangka terbuka ini hanya kolom yang menahan beban lateral. Pada struktur rangka terbuka, terdapat rangka ruang lengkap yang memikul beban gravitasi, sedangkan beban lateral dipikul oleh dinding struktural. Berikut contoh gambar struktur rangka terbuka :



Gambar 2.3 Struktur Rangka Terbuka

2.5.1 Struktur Balok

Balok adalah komponen struktur yang bertugas meneruskan beban yang disangga sendiri maupun dari plat kepada kolom penyangga. Balok menahan gaya-gaya yang bekerja dalam arah transversal terhadap sumbunya yang mengakibatkan terjadinya lenturan.

Berdasarkan jenis keruntuhannya, keruntuhan yang terjadi pada balok dapat dikelompokkan menjadi 3 kelompok, yaitu :

a. Penampang *balanced.*

Tulangan tarik mulai leleh tepat pada saat beton mencapai regangan batasnya dan akan hancur karena tekan. Pada saat awal terjadinya keruntuhan, regangan tekan yang diijinkan pada saat serat tepi yang tertekan adalah 0,003 sedangkan regangan baja sama dengan regangan lelehnya yaitu $\varepsilon_y = f_y/E_s$.

b. Penampang *over-reinforced.*

Keruntuhan ini ditandai dengan hancurnya beton yang tertekan. Pada awal keruntuhan, regangan baja ε_s yang terjadi masih lebih kecil daripada regangan lelehnya εy. Dengan demikian tegangan baja *fs* juga lebih kecil dari pada tegangan lelehnya *fy*. Kondisi ini terjadi apabila tulangan yang digunakan lebih banyak daripada yang diperlukan dalam keadaan *balanced.*

c. Penampang *under-reinforced.*

Keruntuhan ini ditandai dengan terjadinya leleh pada tulangan baja. Kondisi penampang yang demikian dapat terjadi apabila tulangan tarik yang dipakai pada balok kurang dari yang diperlukan untuk kondisi *balanced.*

2.5.2 Struktur Kolom

Definisi kolom menurut SNI 03-2847-2013 pasal 2.2 adalah Komponen struktur dengan rasio tinggi terhadap dimensi lateral terkecil melampaui 3 yang digunakan terutama untuk menumpu beban tekan aksial. Untuk komponen struktur dengan perubahan dimensi lateral, dimensi lateral terkecil adalah rata-rata dimensi atas dan bawah sisi yang lebih kecil.

Kolom tidak hanya menerima beban aksial vertikal, tetapi momen lentur, sehingga analisis kolom diperhitungkan untuk menyangga beban aksial desak dengan eksentrisitas tertentu. Berikut ketentuan-ketentuan yang harus diperhatikan dalam merencanakan kolom menurut SNI 03-2847-2013 pasal 8.10 :

- Kolom harus dirancang utuk menahan gaya aksial dari beban terfaktor pada semua lantai atau atap dan momen maksimum dari beban terfaktor pada suatu lantai atau atap bersebelahan yang ditinjau. Kondisi pembebanan yang memberikan rasio momen maksimum terhada beba aksial juga harus ditinjau.
- Pada rangka atau konstruksi menerus, pertimbangan harus diberikan pada pengaruh beban lantai atau atap tak seimbang pada baik kolom eksterior dan interior dan dari pembebanan eksentris akibat penyebab lainya.
- Dalam menghitung momen akibat beban gravitasi pada kolom, diizinkan untuk mengasumsikan ujung jauh kolom yang dibangun menyatu dengan struktur terjepit.

- Momen-momen pada setiap level lantai atau atap harus disediakan dengan mendistribusikan momen diantara kolom-kolom langsung diatas dan di bawah lantai ditetapkan dalam proporsi terhadap kekakuan kolom relatif dan kondisi kekangan pada ujung kolom.

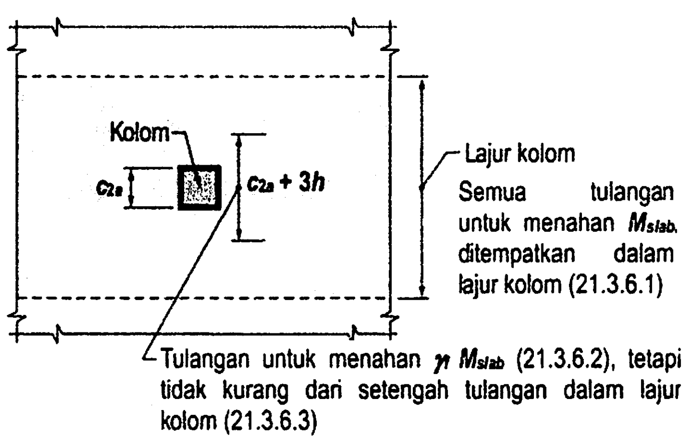
2.6 Komponen Struktur Rangka Momen Khusus dan yang dikenai beban lentur dan aksial

2.6.1 Lingkup

Untuk Persyaratan dari subpasal 21.6.1 berlaku untuk komponen struktur rangka momen khusus yang dikenai beban lentur dan aksial membentuk bagian sistem penahan gaya gempa dan yang menahan gaya tekan aksial terfaktor *Pu* akibat sebarang kombinasi beban yang melebihi *Ag fc/ 10.* Komponen struktur rangka ini harus juga memenuhi kondisi-kondisi dari SNI 03-2837-2013 pasal 21.6.1.1 dan 21.6.1.2.

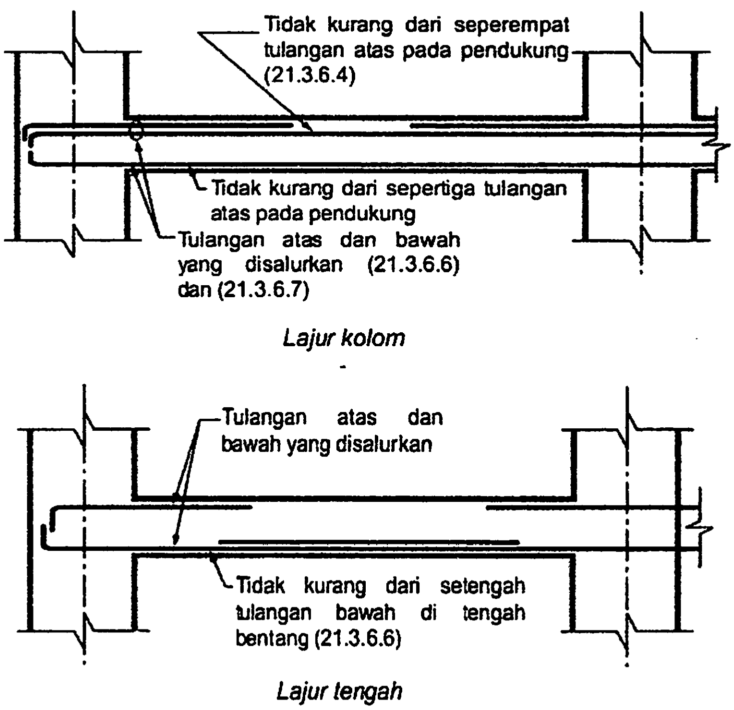
- Dimensi penampang terpendek, diukur pada garis lurus yang melalui pusat geometri, tidak boleh kurang dari *300 mm.*
- Rasio dimensi penampang terpendek terhadap dimensi tegak lurus tidak boleh kurang dari *0,4.*

Persyaratan dari 21.5 berlaku untuk komponen struktur rangka momen khusus yang membentuk bagian sistem penahan gaya gempa dan diproporsikan terutama untuk menahan lentur. Komponen struktur rangka ini juga harus memenuhi kondisi-kondisi dari 21.5.1.1 hingga 21.5.1.4.



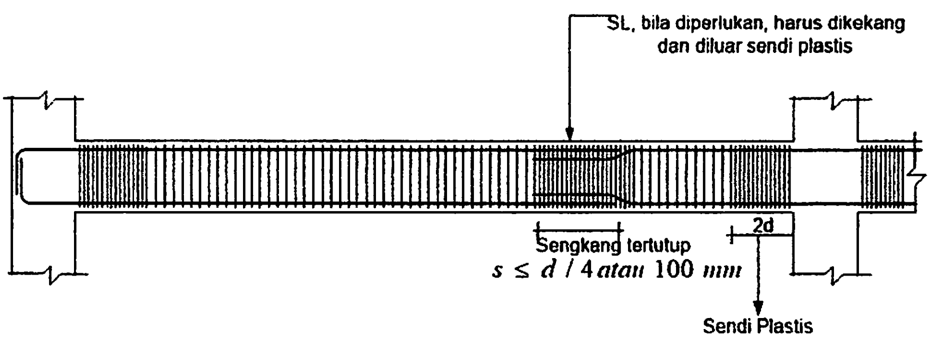
Sun Catatan: Berlaku untuk kedua tulangan atas dan bawah

Gambar 2.4 Lokasi Penulangan Pada Slab



Sumber : SNI-03-2847-2013, Pasal : 21.5.1

Gambar 2.5 Penempatan Tulangan Pada Slab



Gambar 2.6 Tipikal Sambungan Lewatan (SL)

1. Gaya tekan aksial terfaktor pada komponen struktur, Pu, tidak boleh melebihi $Ag fc'/10$.
2. Bentang bersih untuk komponen struktur, ℓ_n, tidak boleh kurang dari empat kali tinggi efektifnya.
3. Lebar komponen, b_w, tidak boleh kurang dari yang lebih kecil dari $0,3h$ dan $250\ mm$.

4. Lebar komponen struktur, b_w, tidak boleh melebihi lebar komponen struktur penumpu, C_2, ditambah suatu jarak pada masing-masing sisi komponen struktur penumpu yang sama dengan yang lebih kecil dari (a) dan (b):

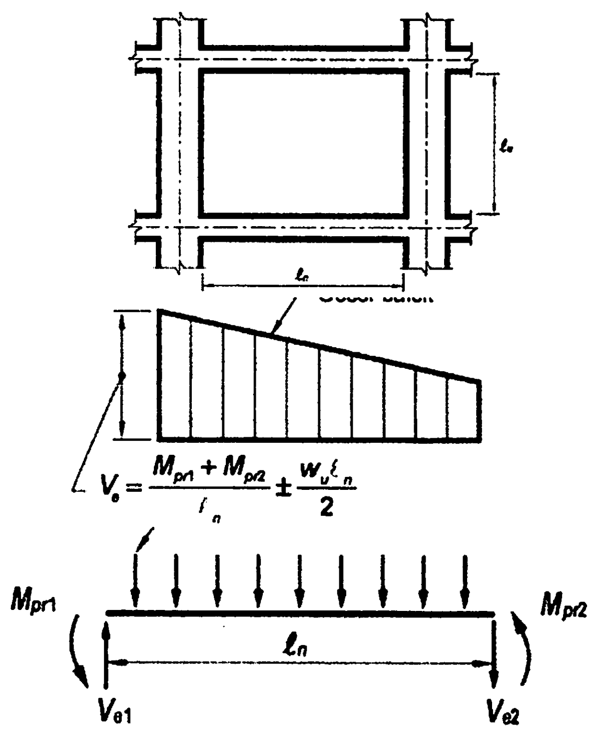
   (a) Lebar komponen struktur penumpu, C_2, dan

   (b) 0,75 kali dimensi keseluruhan komponen struktur penumpu, C_1.

2.6.2 Perencanaan Balok

2.6.2.1 Balok terhadap Geser

Pada SNI-2847-2013 pada pasal 21.5.4 Gaya geser desain, *Ve*, harus ditentukan dari peninjauan gaya statis pada bagian komponen struktur antara muka-muka joint. Harus diasumsikan bahwa momen-momen dengan tanda berlawanan yang berhubungan dengan kekuatan momen lentur yang mungkin, *Mpr*, bekerja pada muka-muka joint dan bahwa komponen struktur dibebani dengan beban gravitasi tributari terfaktor sepanjang bentangnya.



Sumber : SNI-03-2847-2013, Pasal : 21.6.2

Gambar 2.9 Geser desain untuk balok

Komponen struktur yang mengalami lentur akan mengalami juga kehancuran geser, selain kehancuran tarik / tekan. Sehingga dalam perencanaan struktur yang mengalami lentur selain direncanakan tulangan lentur, juga harus direncanakan tulangan geser. Pada Kuat geser pada struktur yang mengelami lentur SNI 03-2847-2013 Pasal 11.1.1 adalah:

$$\phi Vu \geq Vn$$

$$Vn = Vc + Vs$$

Dimana :

Vu = gaya geser terfaktor pada penampang yang ditinjau

Vc = kuat geser nominal yang disumbangkan oleh beton pada penampang yang ditinjau.

Vs = kuat geser nominal yang disumbangkan oleh tulangan pada penampang yang ditinjau.

Vn = kuat geser nominal pada penampang yang ditinjau.

Gaya geser terfaktor (Vu) ditinjau pada penampang sejarak (d) dari muka tumpuan dan untuk penampang yang jaraknya kurang dari d dapat direncanakan sama dengan pada penampang yang sejarak d.

Kuat geser yang disumbangkan oleh beton sesuai dengan SNI 03-2847-2013 Pasal 11.2 yang di kenai geser dan lentur adalah :

$$Vc = 0{,}17\lambda.\sqrt{fc'}.b_w.d$$

Dimana :

λ = dipakai nilai 1,0 untuk beton normal

b_w = lebar badan balok

d = jarak dari serat terkan terluar ke titik berat tulangan tarik longitudinal

Ada dua keadaan :

Bila Vu > ½ Ø Vc, maka harus dipasang tulangan geser minimum dengan luas tulangan :

$$Av = \frac{b_w.s}{3.fy}$$

Dan bila $Vu > \varnothing\ Vc$, maka harus dipasang tulangan geser, sedangkan besar gaya geser yang disumbangkan oleh tulangan adalah:

$$Vs = \frac{Av.fy.d}{s}$$

Dimana :

Av = luas tulangan geser dalam daerah sejarak s

$$Av = n.1/4.\pi.d^2$$

n = Jumlah kaki pada sengkang

S = spasi tulangan geser dalam arah pararel dengan tulangan longitudinal

Sedangkan untuk spasi sengkang adalah :

$S \leq 1/2\ d$

$S \leq 600\ mm$

Sedangkan bila Vs > $\left(\frac{\sqrt{fc'}}{3}\right) b_w\ d$,maka spasi tulangan adalah :

$S \leq 1/4\ d$

$S \leq 300\ mm$

Dalam hal ini Vs tidak boleh lebih besar dari $\left(\frac{2}{3}\right)\sqrt{fc'}\ b_w\ d$

Ada beberapa kondisi dalam menghitung tulangan geser :

1. Bila $Vu < ½\ \varnothing\ Vc$ maka pada kondisi ini tidak diperlukan tulangan geser.
2. Bila $\varnothing\ Vc > Vu > ½\ \varnothing\ Vc$ maka pada kondisi ini dipasang tulangan geser minimum.
3. Bila $\varnothing Vc > Vu > \varnothing\ (5/6\ \sqrt{fc'}\ .\ bw.d)$ maka diperlukan tulangan geser.
4. Bila $\varnothing vu > \varnothing\ (5/6\ \sqrt{fc'}\ bw.d)$ maka dimensi diperbesar
5. Dimana : $(Vc+Vs_{maks}) = (1/6+2/3)\ \sqrt{fc'}\ bw.d = 5/6\ \sqrt{fc'}\ .bw.d$

Syarat spasi maksimum pada daerah sendi plastis dan luar sendi plastis adalah :

a. Pada daerah sendi plastis SNI 2847-2013 pasal 21.5.3.2 tidak melebihi :

- Terletak sepanjang 2 x tinggi komponen struktur yang di ukur (h)
- d/4
- 6 x Diameter utama
- 150 mm

b. Pada daerah luar sendi plastis SNI 2847-2013 pasal 21.5.3.4 dan 21.5.3.3 tidak melebih :

- d/2
- 350 mm

2.6.2.2 Balok T

Berikut ketentuan-ketentuan yang harus diperhatikan dalam merencanakan balok T sesuai dengan SNI 03-2847-2013 pasal 8.12 :

- Pada konstruksi balok-T, bagian sayap dan badan balok harus dibuat menyatu (monolit) atau harus dilekatkan secara efektif sehingga menjadi satu kesatuan.
- Lebar pelat efektif sebagai bagian dari sayap balok-T tidak boleh melebihi seperempat bentang balok, dan lebar efektif sayap dari masing-masing sisi badan balok tidak boleh melebihi:

  - Delapan kali tebal pelat, dan
  - Setengah jarak bersih antara balok-balok yang bersebelahan.

- Untuk balok yang mempunyai pelat hanya pada satu sisi, lebar efektif sayap dari sisi badan tidak boleh lebih dari:

  - Seperduabelas dari bentang balok
  - Enam kali tebal pelat, dan
  - Setengah jarak bersih antara balok-balok yang bersebelahan.

- Balok-T tunggal, dimana bentuk T-nya diperlukan untuk menambah luas daerah tekan, harus mempunyai ketebalan sayap tidak kurang dari setengah lebar badan balok, dan lebar efektif sayap tidak lebih dari empat kali lebar badan balok.

- Bila tulangan lentur utama pelat, yang merupakan bagian dari sayap balok-T (terkecuali untuk konstruksi pelat rusuk), dipasang sejajar dengan balok, maka harus disediakan penulangan di sisi atas pelat yang dipasang tegak lurus terhadap balok berdasarkan ketentuan berikut:
  - Tulangan transversal tersebut harus direncanakan untuk memikul beban terfaktor selebar efektif pelat yang dianggap berperilaku sebagai kantilever. Untuk balok-T tunggal, seluruh lebar dari sayap yang membentang harus diperhitungkan. Untuk balok-T lainnya, hanya bagian pelat selebar efektifnya saja yang perlu diperhitungkan.
  - Tulangan transversal harus dipasang dengan spasi tidak melebihi lima kali tebal pelat dan juga tidak melebihi 500 mm.

2.6.2.2.1 Balok T Tulangan Rangkap

Perencanaan balok T tulangan rangkap adalah proses menentukan dimensi tebal dan lebar flens. Lebar dan tinggi efektif badan balok, dan luas tulangan baja tarik. Balok T juga didefenisikan sebagai balok yang menyatu edngan plat, dimana plat tersebut mengalami tekanan.

Dengan nilai $M_{D\,b}$, $M_{L\,b}$, $M_{E\,b}$, (Statika/ hasil STAAD PRO 2004), Dimana kombinasi untuk M_u balok :

$= 1{,}4\ M_{D\,b}$

$= 1{,}2\ M_{D\,b} + 1{,}6\ M_{L\,b}$

$= 1{,}2\ M_{D\,b} + 1{,}0\ M_{L\,b} \pm 1{,}0\ M_{E\,b}$

$= 0{,}9\ M_{D\,b} \pm 1{,}0\ M_{E\,b}$

Dimana :

M_D = Momen lentur komponen portal akibat beban mati tak terfaktor

M_{Lb} = Momen lentur komponen portal akibat beban hidup tak terfaktor

M_{Eb} = Momen lentur komponen portal akibat beban gempa tak terfaktor

Dari keempat kombinasi diatas maka diambil nilai M_u yang paling besar. Balok persegi memiliki tulangan rangkap apabila momen yang harus ditahan cukup besar dan $As\ perlu > AS\ maks$.

Untuk tulangan maksimum ada persyaratan bahwa balok atau komponen struktur lain yang yang menerima beban lentur murni harus bertulang lemah (under reinforced) SNI-03-2847-2013 lampirn B memberikan batasan tulangan tarik maksimum sebesar 75% dari yang diperlukan pada keadaan regang seimbang. $As\ maks = 0{,}75\ \rho_b$

$$\text{As maks} = 0{,}75\left(\frac{0{,}85.fc.\beta 1}{fy} x \frac{600}{600+fy}\right)$$

Untuk tulangan minimum agar menghindari terjadinya kehancuran getas pada balok, maka SNI 03-2847-2013 pada halaman 71-72 juga mengatur jumlah minimum tulangan yang harus terpasang pada balok, yaitu :

$\text{As min} = \dfrac{2.5\sqrt{fc'}}{fy}.bw.d$ dan tidak boleh lebih kecil dari As min = $1{,}4.bw.d / fy$.

Langkah – langkah perencanaan balok T tulangan rangkap

- Dapatkan nilai $M_{D\,b}, M_{L\,b}, M_{E\,b}$, (Statika/ hasil STAAD PRO 2004)

  Dimana kombinasi untuk M_u balok :

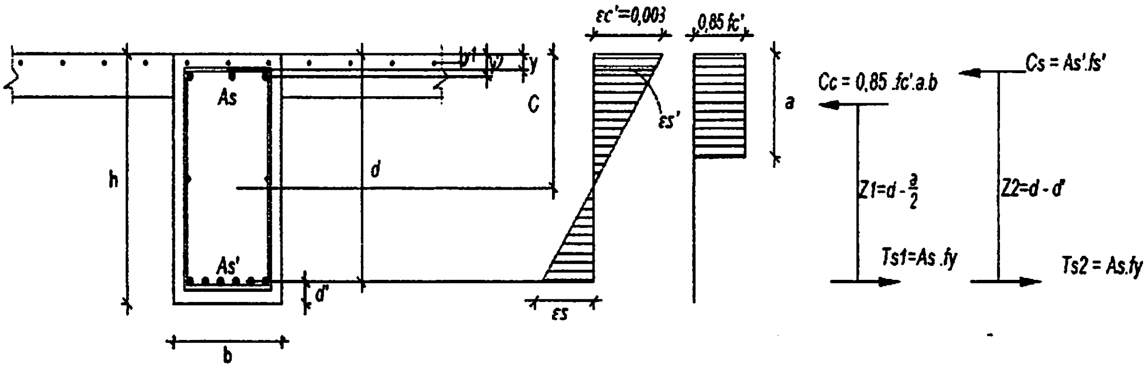
  $= 1{,}4\ M_{D\,b}$

  $= 1{,}2\ M_{D\,b} + 1{,}6\ M_{L\,b}$

  $= 1{,}2\ M_{D\,b} + 1{,}0\ M_{L\,b} \pm 1{,}0\ M_{E\,b}$

  $= 0{,}9\ M_{D\,b} \pm 1{,}0\ M_{E\,b}$

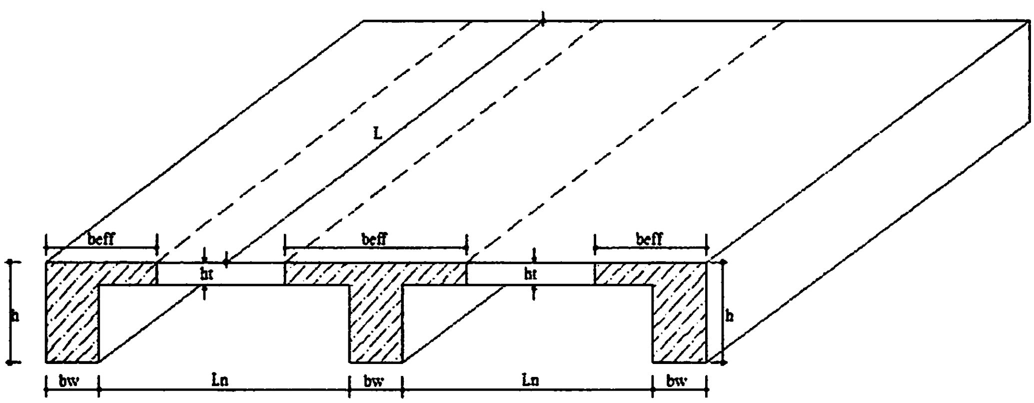
  1. Tentukan tulangan tarik dan tekan
  2. Hitung nlai d' = tebal selimut beton + diameter sengkang + ½ x diameter tulangan tarik. Setelah itu hitung d=h-d'.



Sumber : SNI-03-2847-2013, Pasal : 8.12

Gambar 2.10 Gambar Diagram tegangan Balok T

Menurut SNI 03-2847-2013 pasal 8.12, lebar slab efektif yang diperhitungkan bekerja sama dengan rangka menahan momen lentur di tentukan sebagai berikut :



Sumber : SNI-03-2847-2013, Pasal : 8.12

Gambar 2.11 Lebar efektif Slab

a. Jika balok memunyai plat dua sisi.

Lebar slab efektif sebagai sayap balok-T tidak boleh melebihi seperempat panjang bentang balok, dan lebar efektif sayap yang menggantung pada masing-masing sisi badan balok tidak boleh melebihi:

(a) Delapan kali tebal slab; dan

(b) Setengah jarak bersih ke badan di sebelahnya.

b. Jika balok hanya mempunyai plat satu sisi.

Untuk balok dengan slab pada satu sisi saja, lebar sayap efektif yang menggantung tidak boleh melebihi:

(a) Seperduabelas panjang bentang balok;

(b) Enam kali tebal slab; dan

c. Balok yang terpisah, dimana bentuk-T digunakan untuk memberikan sayap untuk luasan tekan tambahan, harus mempunyai ketebalan sayap tidak kurang dari setengah lebar badan dan lebar efektif sayap tidak lebih dari empat kali lebar badan.

d. Tulangan transversal harus dispasikan tidak lebih jauh dari lima kali tebal slab, atau juga tidak melebihi 450 mm.

Analisia balok bertulangan rangkap dimana tulangan tekan sudah leleh. Misalkan tulangan tarik dan tulangan tekan leleh.

Cc = 0,85 . f'c .ab

Cs = As' .fs' = As'.fy

Ts = As .fy

ΣH = 0 → Cc+Cs=Ts

0,85 . f'c .a .b + As' .fy = As .fy

0,85 .f'c .a .b = as .fy – As' .fy = fy (As - As')

Sehingga nilai : a = $\frac{fy(As - As')}{0{,}85.fc'a.b}.bw.d$

Dengan nilai tersebut kita kontrol regangan yang terjadi apakah tulangan tekan leleh apa belum. Jika leleh, perhitungan dapat dilanjutkan dan jika belum leleh nilai a kita hitung kembali dengan persamaan lain.

Tinggi garis netral $c = \frac{a}{\beta 1} = \frac{(As - As').fy}{\beta 1.0{,}85.fc'b}$

Dari diagram regangan $\frac{\varepsilon' s}{\varepsilon' c} = \frac{(c - d')}{c} \rightarrow \varepsilon' s = \frac{(c - d')}{c}\varepsilon' c$

Jika $\varepsilon_s' < \varepsilon_y = fy / \varepsilon_s$ → berarti tulangan tekan belum leleh maka perhitungan diulang.

Jika $\varepsilon_s' > \varepsilon_y = fy / ES$ → berarti tulangan tekan belum leleh maka perhitungan dilanjutkan.

Mn = Cc . z1 + Cs . z2 dimana : $z_1 = d - \frac{a}{2}$ dan $z_2 = z - z'$

Analisis balok bertulang rangkap dimana tulangan tekan belum leleh.

Ini terjadi jika nilai $\varepsilon' > \varepsilon y = \frac{fy}{Es}$

Untuk itu dicari nilai a dengan persamaan sebagai berikut :

ΣH = 0, maka Cc+Cs=Ts

0,85 . f'c . a.b + As' . fs' = As . fy

fs' = ε_s'. ES dimana : $\varepsilon' s = \frac{(c = d')}{c} \varepsilon' c$

$$fs' = \frac{(c - d')}{c} \varepsilon' c.Es = \frac{(c - d')}{c}.0{,}003.200000$$

$$fs' = \frac{(c - d')}{c}.600$$

Maka 0,85 $.f'c.a.b + As' . 600 = As.fy$

(0,85 $.f'c.a.b).x + As'.(c - d').600 = As.fy.c$

Dengan subsitusi nilai a = β1 . c

(0,85 $.f'c.\beta 1.c.b).c + As'.(c - d').600 = As.fy.c$

(0,85 $.f'c.\beta 1.b).c^2 + As'.(c - d').600 = As.fy.c$

(0,85 $.f'c.\beta 1.b).c^2 + 600.As'.c - As.fy.c - 600.As'.d = 0$

(0,85 $.f'c.\beta 1.b).c^2 + (600.As' - As.fy).c - 600.As'.d = 0$

Dengan rumus ABC nilai x dapat dihitung :

$$c_{1.2} \frac{-b \pm \sqrt{b^2 - 4.a.c}}{2.a}$$

Selanjutnya dapat dihitung dengan nilai-nilai :

$$fs' = \frac{(c - d')}{c}.600$$

Cc = 0,85 . $fc'.a.b$ dimana a = β1 . x

Cs = As' . fs'

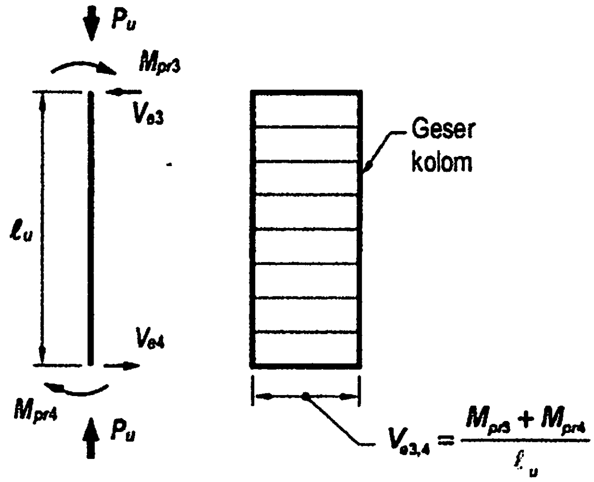
$z_1 = d - \frac{a}{2}$ dan $z_2 = d - d'$

$$Mn = Cc.z_1 + Cs.z_2$$

2.6.3 Perencanaan Kolom

2.6.3.1 Kekuatan Lentur Minimum Kolom

- Kekuatan lentur kolom harus memenuhi Pers. $\Sigma M_{nc} \geq 1,2\ \Sigma M_{nb}$ (21-1) SNI 03-2847-2013.



Sumber : SNI-03-2847-2013, Pasal : 21.6.2

Gambar 2.13 Geser Desain Untuk Balok Dan Kolom

Catatan pada gambar 2.5 :

1. Arah geser V_e tergantung pada besaran relatif beban gravitasi dan geser dihasilkan oleh momen-momen ujung.
2. Momemn-momen ujung M_{pr} berdasarkaan pada tegangan tarik baja sebesar $1,25F_y$ adalah kekuatan leleh yang ditetapkan.
3. Momen ujung M_{pr} untuk kolom tidak perlu lebih besar dari momen-momen yang dihasilkan oleh M_{pr} balok-balok yang merangka ke dalam joint balok-kolom.

Kuat lentur komponen struktur dapat ditentukan dengan menggunakan rumus :

$\Sigma M_{nc} \geq 1,2\ \Sigma M_{nb}$

Diamana :

ΣMnc = jumlah kekuatan lentur nominal kolom yang merangka ke dalam joint, yang dievaluasi di muka-muka joint. Kekuatan lentur kolom harus dihitung untuk gaya aksial terfaktor, konsisten dengan arah gaya-gaya lateral yang ditinjau, yang menghasilkan kekuatan lentur terendah.

ΣMnb = jumlah kekuatan lentur nominal balok yang merangka ke dalam joint, yang dievaluasi di muka-muka joint. Pada konstruksi balok-T, bilamana slab dalam kondisi Tarik akibat momen-momen di muka joint, tulangan slab dalam lebar slab efektif yang didefinisikan dalam 8.12 harus diasumsikan menyumbang kepada Mnb jika tulangan slab disalurkan pada penampang kriris untuk lentur.

2.6.3.1.1 Kolom Eksentrisitas Kecil

Kolom adalah komponen struktur bangunan yang berfungsi untuk menyangga beban aksial tekan vertikal dengan eksentrisitas tertentu yang mana bagian tinggi yang tidak ditopang paling tidak tiga kali dimensi lateral terkecil.

Jika nilai eksentrisitas $e = \frac{M}{P} \leq e_{min}$, maka kolom tersebut dikategorikan sebagai kolom dengan eksentrisitas kecil, yang mana harga *e* minimum adalah *0.01 h* jika menggunakan pengikat sengkang dan *0.05 h* jika menggunakan pengikat spiral.

Analisis kolom dengan beban aksial eksentrisitas kecil pada hakekatnya adalah pemeriksaan terhadap kekuatan maksimal bahan yang tersedia, yaitu :

$$\rho g = \frac{Ast}{Ag}$$

Yang mana harga tersebut harus berkisar $0.01 \leq \rho g \leq 0.08$, sehingga kuat beban aksial maksimum adalah sebagai berikut:

$\phi\ Pn = 0.85\ \phi\ \{0.85 f'c\ (Ag-Ast) + fy.Ast\}$ → pengikat spiral

$\phi\ Pn = 0.80\ \phi\ \{0.85 f'c\ (Ag-Ast) + fy.Ast\}$ → pengikat sengkang

2.6.3.1.2 Kolom Eksentrisitas Besar

Jika nilai eksentrisitas $e = \frac{M}{P} \geq e_{min}$, maka pada analisis selanjutnya, harus membandingkan nilai Pn dan Mn, Pb dan Mb.

Keadaan seimbang adalah pada saat regangan beton mencapai 0.003 dan bersamaan pula tegangan pada batang tulangan mencapai leleh.

Dengan definisi :

Pb = Kuat beban aksial nominal pada keadaan seimbang

cb = jarak dari serat tepi tekan kegaris netral keadaan seimbang

Maka berdasarkan diagram diagram regangan tegangan keadaan seimbang dapat diperoleh:

$$cb = \frac{600}{600+fy} d$$

$$ab = \beta_1 \cdot cb$$

$$Pb = D_1 + D_2 - T$$

$$Mnb = Pb\ eb$$

Jika Pu < Pb, maka model keruntuhan yang terjadi adalah keruntuhan tarik sehingga kapasitas penampang dapat dihitung dengan rumus sebagai berikut:

$$Pn = 0{,}85.fc'.b.d \left\{ \left(1-\frac{e'}{d}\right) + \sqrt{\left(1-\frac{e}{d}\right)^2 + 2m\rho\left(1-\frac{d'}{d}\right)} \right\}$$

Jika Pu > Pb, maka model keruntuhan yang terjadi adalah keruntuhan tekan sehingga kapasitas penampang dapat dihitung dengan rumus sebagai berikut :

$$Pn = \varphi \left[\frac{As'.fy}{\frac{e}{d-d'}+0{,}5} + \frac{b.h.fc'}{\frac{3he}{d^2}+1{,}18} \right]$$

2.6.3.2 Kolom Kuat Balok Lemah

Berdasarkan prinsip kolom kuat balok lemah dimana kolom harus diberi cukup kekuatan, sehingga kolom tidak leleh lebih runtuh sebelum balok. Goyangan lateral memungkinkan terjadinya sendi plastis di ujung-ujung kolom akan menyebabkan kerusakan berat, karena itu harus dihindarkan. Oleh sebab itu kolom-kolom selalu didesain 20% lebih kuat dari balok-balok di suatu joint balok kolom.

Komponen rangka yang termasuk dalam klasifikasi komponen struktur yang terkena beban lentur dan aksial dalam harus memenuhi persyaratan sebagai berikut :

Kuat lentur komponen strukturnya dapat ditentukan dengan menggunakan rumus :

$$\sum Me \geq (1,2)\sum Mg$$

$(M_{nt} + M_{nb}) \geq 6/5\ (Mn_{ki} + Mn_{ka})$

Dimana :

$\sum$Me = jumlah momen dimuka HBK sesuai dengan desain kuat lentur nominal kolom.

$\sum$Mg = jumlah momen dimuka HBK sesuai dengan desain kuat lentur nominal balok.

Mnt = Momen kolom nominal top (atas)

Mnb = Momen nominal kolom bawah

Mnki = Momen nominal balok kiri

Mnka = Momen nominal balok kanan

2.6.4 Tulangan Longitudinal

1. Pada sebarang penampang komponen struktur lentur, kecuali seperti diberikan dalam 10.5.3, untuk tulangan atas maupun bawah, jumlah tulangan tidak boleh kurang dari yang diberikan oleh Pers. (10-3) SNI 03-2847-2013 tetapi tidak kurang dari $1,4b_wd/fy$, dan rasio tulangan, ρ, tidak boleh melebihi 0,025. Paling sedikit dua batang tulangan harus disediakan menerus pada kedua sisi atas dan bawah.
2. Kekuatan momen positif pada muka joint harus tidak kurang dari setengah kekuatan momen negatif yang disedikan pada muka joint tersebut. Baik kekuatan momen negatif atau positif pada sebarang penampang sepanjang panjang komponen struktur tidak boleh kurang dari seperempat kekuatan momen maksimum yang disediakan pada muka salah satu dari joint tersebut.
3. Sambungan lewatan tulangan lentur diizinkan hanya jika tulangan sengkang atau disediakan sepanjang panjang sambungan. Spasi tulangan transversal yang melingkupi batang tulangan yang

disambung lewatkan tidak boleh melebihi yang lebih kecil dari *d/4* dan 100 mm. Sambungan lewatan tidak boleh digunakan:

(a) Dalam joint;

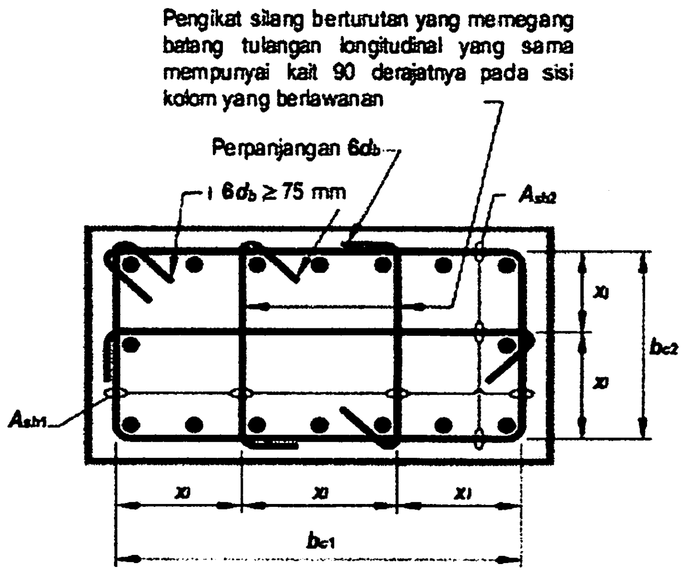
(b) Dalam jarak dua kali tinggi komponen struktur dari muka joint;

(c) Bila analisis menunjukkan pelelehan lentur diakibatkan oleh perpindahan lateral inelastis rangka.

2.6.4 Tulangan transversal

- Tulangan transversal yang disyaratkan dalam 21.6.4.2 sampai 21.6.4.4 harus dipasang sepanjang panjang ℓ_o dari setiap muka joint dan pada kedua sisi sebarang penampang dimana pelelehan lentur sepertinya terjadi sebagai akibat dari perpindahan lateral inelastis rangka. Panjang ℓ_o tidak boleh kurang dari yang terbesar dari (a), (b), dan (c):
  (a) Tinggi komponen struktur pada muka joint atau pada penampang dimana pelelehan lentur sepertinya terjadi;
  (b) Seperenam bentang bersih komponen struktur; dan
  (c) 450 mm.
- Spasi tulangan transversal sepanjang ℓ_o (daerah sendi plastis) komponen struktur tidak boleh melebihi yang terkecil (a), (b), dan (c) :
  (a) Seperempat dimensi komponen struktur minimum;
  (b) Enam kali diameter batang tulangan longitudinal yang terkecil; dan
  (c) *so*, seperti didefinisikan oleh $S_o = 100 + (\frac{350 - hx}{3})$

  Dimana nilai S_o tidak boleh melibihi *150 mm* dan tidak perlu diambil kurang dari *100 mm*.



Dimensi x_i dari garis pusat ke garis pusat kaki-kaki pengikat tidak melebihi 350 mm. Rumus h_x yang digunakan dalam persamaan 21-2 diambil sebagai nilai terbesar dari x_i.

Sumber : SNI-03-2847-2013, Pasal : 21.6.4.3

Gambar 2.14 Contoh Tulangan Transversal Pada Kolom.

- Di luar panjang ℓ_o yang di tetapkan dalam 21.6.4.1 kolom harus mengandung tulangan sepiral atau sengkang yang memenuhi 7.10 dengan spasi pusat ke pusat , s, tidak melebihi yang lebih kecil dari 6 x diameter batang tulangan kolom longitudinal yang terkecil dan 150 mm, kecuali bila jumlah tulangan transfersal yang lebih besar yang di syaratkan oleh 21.6.3.2 atau 21.6.5.

Persyaratan Tulangan Transversal (TT) di SNI-2847-2013 pada Pasal 21.6.4.4 adalah sebagai berikut :

a. Rasio Volumetrik tulangan spiral atau sengkang cincin tidak boleh kurang dari

$$\rho_s = 0{,}12 f_c' / f_{yh}$$

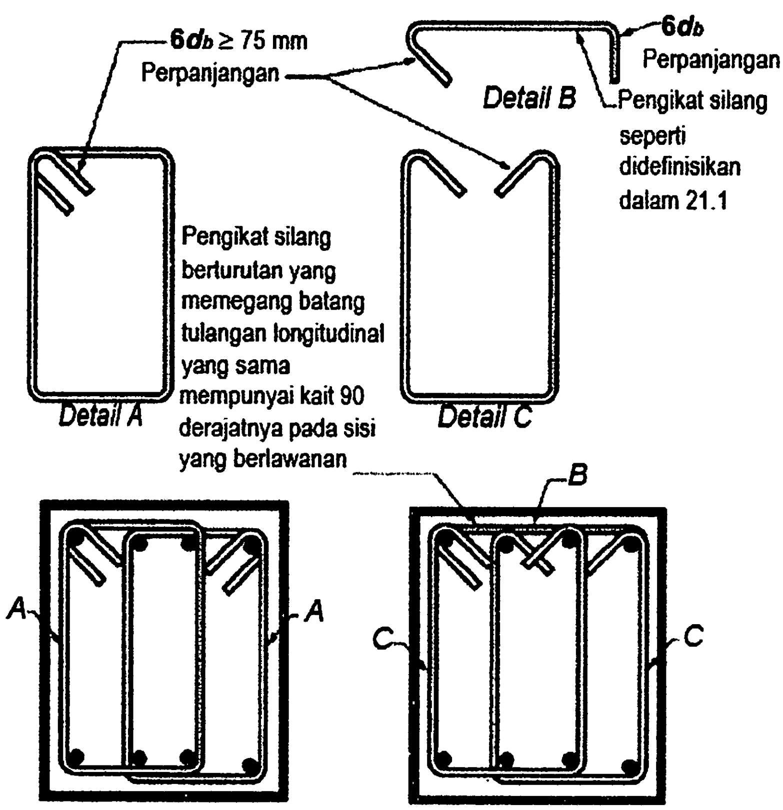
b. Total luas penampang tulangan hoops persegi panjang untuk pengekangan harus tidak boleh kurang dari nilai dua persamaan ini :

$$A_{sh} = 0{,}3\left(s h_c f_c' / f_{yh}\right)\left[\left(A_g / A_{ch}\right) - 1\right]$$

$$A_{sh} = 0{,}09\left(s h_c f_c' / f_{yh}\right)$$

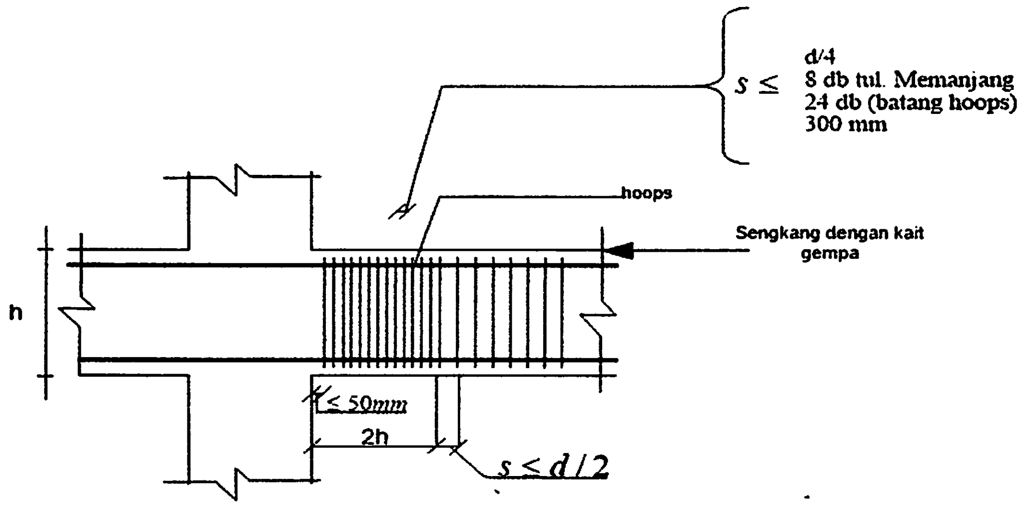
2.6.4.1 Tulangan tranfersal pada sengkang

1. Sengkang harus dipasang pada daerah komponen struktur rangka berikut :
   (a) Sepanjang suatu panjang yang sama dengan dua kali tinggi komponen struktur yang diukur dari muka komponen struktur penumpu ke arah tengah bentang, di kedua ujung komponen struktur lentur;
   (b) Sepanjang panjang-panjang yang sama dengan dua kali tinggi komponen struktur pada kedua sisi suatu penampang dimana pelelehan lentur sepertinya terjadi dalam hubungan dengan perpindahan lateral inelastis rangka.



Sumber : SNI-03-2847-2013, Pasal : 21.5.3

Gambar 2.7 Contoh-contoh sengkang tertutup saling tumpuk dan ilustrasi batasan spasi horizontal maksimum batang tulangan longitudinal yang ditumpu.



Sumber : SNI-03-2847-2013, Pasal : 11.11.2.2

Gambar 2.8 Penulangan Transversal Untuk Komponen Lentur Pada SRPMK

2. Sengkang tertutup pertama harus ditempatkan tidak lebih dari 50 mm dari muka komponen struktur penumpu. Spasi sengkang tertutup tidak boleh melebihi yang terkecil dari (a), (b), dan (c):
   (a) *d/4*;
   (b) Enam kali diameter terkecil batang tulangan lentur utama tidak termasuk tulangan kulit longitudinal yang disyaratkan oleh 10.6.7; dan
   (c) *150 mm*
3. Bila sengkang tertutup diperlukan, batang tulangan lentur utama yang terdekat ke muka tarik dan tekan harus mempunyai tumpuan lateral yang memenuhi 7.10.5.3 atau 7.10.5.4. Spasi batang tulangan lentur yang tertumpu secara transversal tidak boleh melebihi 350 mm. Tulangan kulit yang disyaratkan oleh 10.6.7 tidak perlu tertumpu secara lateral.
4. Bila sengkang tertutup tidak diperlukan, sengkang dengan kait gempa pada kedua ujung harus dispasikan dengan jarak tidak lebih dari *d/2* sepanjang panjang komponen struktur

Tulangan transversal sepanjang panjang yang diidentifikasi diatas harus diproporsikan untuk menahan geser dengan mengasumsikan $Vc = 0$ bilamana keduanya (a) dan (b) terjadi:

(a) Gaya geser yang ditimbulkan gempa yang dihitung sesuai dengan 21.5.4.1 mewakili setengah atau lebih dari kekuatan geser perlu maksimum dalam panjang tersebut;

(b) Gaya tekan aksial terfaktor, *Pu*, termasuk pengaruh gempa kurang dari *Ag fc'/ 20.*

2.6.5 Tulangan memanjang

- Luas tulangan memanjang A_{st} tidak boleh kurang dari $0,01A_g$ atau lebih dari $0,06\,A_g$
- Pada kolom dengan sengkang tertutup bulat, jumlah batang tulangan longitudinal minimum harus *6*.
- Sambungan mekanis harus memenuhi pasal 21.1.6 (SNI 03-2847-2013) dan sambungan las memenuhi pasal 21.1.7 (SNI 03-2847-2013). Sambungan lewatan diizinkan hanya setengah pusat panjang komponen struktur, harus didesain sebagai sambungan lewatan tarik, dan harus dilingkupi tulangan transversal memenuhi 21.6.4.2 dan 21.6.4.3.

2.6.6 Joint Rangka Momen Khusus

- Gaya-gaya pada tulangan balok longitudinal di muka joint harus ditentukan dengan mengasumsikan bahwa tegangan pada tulangan tarik lentur adalah *1,25fy*.
- Tulangan longitudinal balok yang dihentikan dalam suatu kolom harus diteruskan ke muka jauh inti kolom terkekang dan diangkur dalam kondisi tarik menurut pasal 21.7.5 dan dalam kondisi tekan menurut Pasal 12.
- Bila tulangan balok longitudinal menerus melalui joint balok-kolom, dimensi kolom yang sejajar terhadap tulangan balok tidak boleh kurang dari 20 kali diameter batang tulangan balok longitudinal terbesar untuk beton normal (normalweight). Untuk beton ringan (lightweight), dimensinya tidak boleh kurang dari 26 kali diameter batang tulangan.
- Tulangan transversal joint harus memenuhi salah satu dari 21.6.4.4(a) atau 21.6.4.4(b), dan harus juga memenuhi 21.6.4.2, 21.6.4.3, dan 21.6.4.7, kecuali seperti diizinkan dalam 21.7.3.2.
- Bilamana komponen-komponen struktur merangka ke dalam semua

empat sisi joint dan bilamana setiap lebar komponen struktur adalah paling sedikit tiga perempat lebar kolom, jumlah tulangan yang ditetapkan dalam 21.6.4.4(a) atau 21.6.4.4(b) diizinkan untuk direduksi dengan setengahnya, dan spasi yang disyaratkan dalam 21.6.4.3 diizinkan untuk ditingkatkan sampai 150 mm dalam tinggi keseluruhan h komponen struktur rangka yang terpendek.

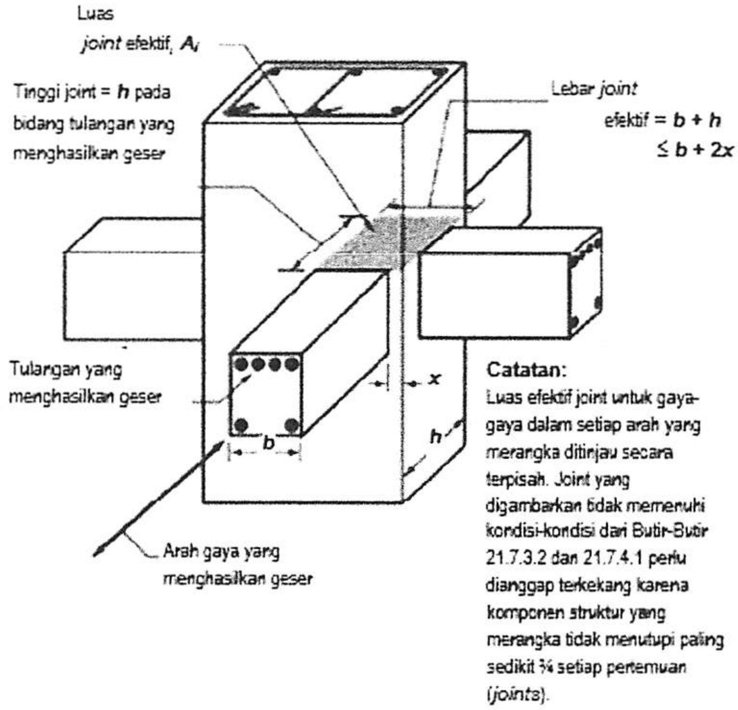
- Tulangan balok longitudinal di luar inti kolom harus dikekang dengan tulangan transversal yang melewati kolom yang memenuhi persyaratan spasi dari 21.5.3.2, dan persyaratn dari 21.5.3.3 dan 21.5.3.6, jika pengekangan tersebut tidak disediakan oleh suatu balok yang merangka ke dalam joint.
- Untuk kuat geser Untuk beton berat normal, Vn joint tidak boleh diambil sebagai yang lebih besar dari nilai yang ditetapkan di bawah (Gambar 2.7).

- Untuk joint yang terkekang oleh balok-balok pada semua empat muka

  $1{,}7\sqrt{fc'}\,Aj$

- Untuk joint yang terkekang oleh balok-balok pada tiga muka atau pada dua muka yang berlawanan $1{,}2\sqrt{fc'}\,Aj$ dan untuk kasus-kasus lainnya

  $1{,}0\sqrt{fc'}\,Aj$



Sumber : SNI-03-2847-2013, Pasal : 21.7.4

Gambar 2.17 Luas Joint efektif.

- *Aj* adalah luas penampang efektif dalam suatu joint yang dihitung dari tinggi joint kali lebar joint efektif. Tinggi joint harus merupakan tinggi keseluruhan kolom, h. Lebar joint efektif harus merupakan lebar keseluruhan kolom, kecuali bilamana suatu balok merangka ke dalam suatu kolom yang lebih lebar, lebar joint efektif tidak boleh melebihi yang lebih kecil dari (a) dan (b):

  (a) Lebar balok ditambah tinggi joint

  (b) Dua kali jarak tegak lurus yang lebih kecil dari sumbu longitudinal balok ke sisi kolom.
- Untuk beton ringan (lightweight), kekuatan geser nominal joint tidak boleh melebihi tiga perempat batasan yang diberikan dalam 21.7.4.1
- Untuk ukuran batang tulangan Ø-10 sampai D-36, panjang penyaluran, l_{dh}, untuk batang tulangan dengan kait 90 derajat standar pada beton normal (normalweight) tidak boleh kurang dari yang terbesar dari $8d_b$, $150\ mm$, dan panjang yang disyaratkan oleh Pers :

$$L_{dh} = \frac{fy.db}{5{,}4\sqrt{fc'}}$$

- Untuk ukuran batang tulangan Ø-10 sampai D-36, ℓ_d, panjang penyaluran dalam kondisi tarik untuk batang tulangan lurus, tidak boleh kurang dari yang lebih besar dari (a) dan (b):

  (a) *2,5 kali* panjang yang disyaratkan oleh 21.7.5.1 bila tinggi beton yang dicetak dalam satu kali angkat di bawah batang tulangan tidak melebihi *300 mm*;

  (b) *3,25 kali* panjang yang disyaratkan oleh 21.7.5.1 bila tinggi beton yang dicetak dalam satu kali angkat di bawah batang tulangan melebihi *300 mm*.

BAB III

METODOLOGI STUDI

1.1 Lokasi Studi

Studi Perencanaan Portal Struktur Gedung dengan dasar SNI-031726 2012 ini dilaksanakan dan bertempatkaan pada Hotel Aria Centra Jl. Taman AIS. Nasution Surabaya yang memiliki data umum bangunan seperti berikut :

Nama Gedung	: Hotel Aria Centra Surabaya.
Lokasi	: Surabaya.
Fungsi	: Penginapan.
Zona Gempa	: 2
Jumlah Lantai	: 16 Lantai.
Tinggi Bangunan	: 64.400 meter.
Panjang Bangunan	: 61.800 meter.
Lebar Bangunan	: 32.500 meter.
Mutu Beton	: $f'c$ 25 Mpa.
Mutu Baja Ulir	: fy 400 Mpa
Mutu Baja Polos	: fy' 240 Mpa

1.2 Sumber Data

Langkah-langkah yang dilakukan dalam pengumpulan data perencanaan portal struktur gedung ini, sebagai berikut ;

a) Pengamatan
Dilakukan agar penulis mengetahui terlebih dahalu pada objek yang akan dikajinya.

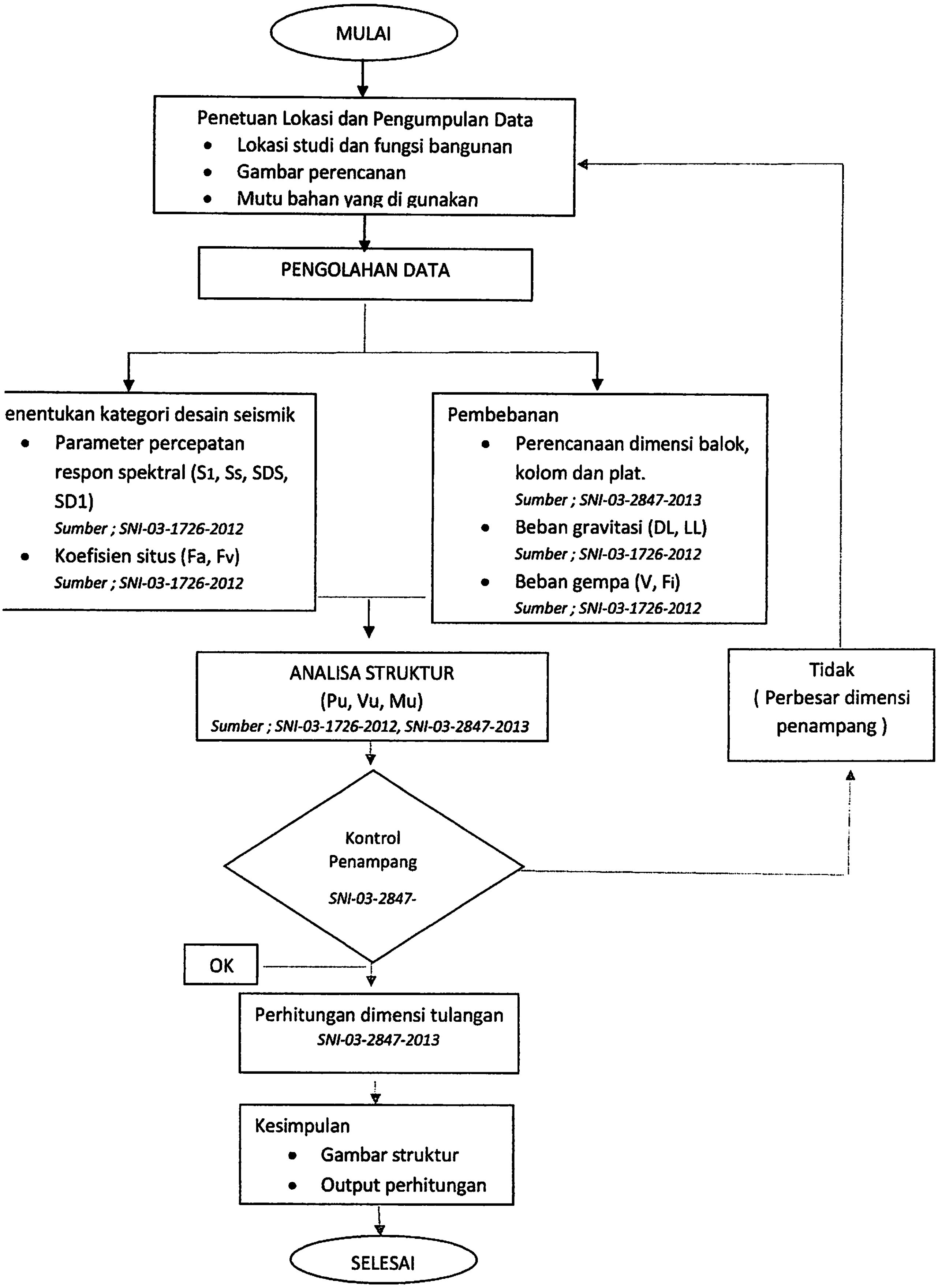
b) Data Primer

Untuk pengumpulan data primer di dapat dari pengambilan data yang sudah ada kepada perencana utama bagunan Hotel Aria Centra, Surabaya yang berupa gambar denah proyek beserta potongan strukturnya.

c) Data Sekunder

Dalam Data sekunder didapakan dari data yang telah di publikasikan yang berupakan peraturan-perutan yang sudah ditetapkan, lisensi dari buku-buku dan internet yang yelah dipublikasikan oleh pihak yang sudah meneliti dalam dunia struktur.

3.3 Diagram Alir



3.4 Menentukan Kategori Desain Seismik Menurut SNI 03-1726-2012 Pada Kota Surabaya

3.4.1 Menentukan parameter percepatan respon spektral

Dari gambar 8. Peta respon spektra percepatan 1 detik (S_s) di batuan dasar (S_B) untuk probabilitas terlampaui 2% dalam 50 tahun. Dan gambar 9. Peta respon spektra percepatan 0.2 detik (S_S) di batuan dasar (S_B) untuk probabilitas terlampaui 2% dalam 50 tahun.

Didapat : $S_1 \sim 0.247$

$S_s \sim 0.663$

} Sumber www.puskim.go.id

Dimana :

S_s ; parameter percepatan respons spektral MCE dari peta gempa pada perioda pendek, redaman 5 persen.

S_1 : parameter percepatan respons spektral MCE dari peta gempa pada perioda 1 detik, redaman 5 persen

3.4.2 Menentukan koefisien situs Fa dan Fv

Kemudian langkah selanjutnya menetukan klasifikasi situs tanah dasar pada tabel 3. Hal ini digunkan untuk mencari koefisien situs Fa dan Fv yang terdapat pada tabel 5 dan tabel 6 SNI 03-1726-2012 dengan menggunakan metode interpolasi dengan kelas situs tanah sedang (SD), sehingga didapat :

S_s : 0.663 ~ Fa : 1.27

S_1 : 0.247 ~ Fv : 1.906

Dimana :

F_A : koefisien situs untuk perioda pendek (pada perioda 0,2 detik)

F_V : koefisien situs untuk perioda panjang (pada perioda 1 detik)

3.4.3 Menentukan S_{DS} dan S_{D1}

Selanjutnya mentukan parameter percepatan respon di bawah ini :

S_{DS} = 2/3 Fa. Ss

= 2/3 1,27 .0,663

= 0,561 g

S_{D1} = 2/3 Fv. S_1

= 2/3. 1,906 .0,247

= 0,313 g

Dimana :

S_{DS}: : parameter percepatan respons spektral pada perioda pendek, redaman 5Persen

S_{DS}: : parameter percepatan respons spektral pada perioda 1 detik , redaman 5 persen

3.4.4 Menentukan Kategori desain Seismik

Dari hasil perhitungan S_{DS} dan S_{D1} bisa didapatkan kategori desain seismik dengan menggunakan tabel 6 dan 7 SNI-03-1726-2012. Dengan demikian diadapat kesimpulan bahwa Kota Surabaya ditetapkan dengan kategori desain seismik (KDS) "***D***".

3.5 Perencanaan Dimensi Plat, Balok dan Kolom

3.5.1 Perencanaan Dimensi Plat

Untuk perencanaan dimensi plat lantai pada pembangunan Hotel Aria Centra-Surabaya, menggunakan dimensi setebal 12 cm.

3.5.2 Perencanaan Dimensi Balok

Berikut perhitungan rencana dimensi balok sesuai dengan ketentuan diatas:

- Untuk balok bentang 10 m (B1)

$$h = \frac{1}{10}L - \frac{1}{15}L = \frac{1}{10}1000 - \frac{1}{15}1000$$

$= 100$ cm – 66,666 cm ..... Dipakai dimensi 80 cm

$$b = \frac{1}{2}h - \frac{2}{3}h = \frac{1}{2}80 - \frac{2}{3}80$$

$= 40$ cm – 53.33 cm ..... Dipakai dimensi 45 cm

Balok anak $= \frac{b}{h} = \frac{45}{80} = 0.5 > 0.3 \ldots$(ok)

Jadi, untuk bentang 10 m dipakai dimensi balok induk 45/80

- Untuk balok bentang 6.5 m (B2)

$$h = \frac{1}{10}L - \frac{1}{15}L = \frac{1}{10}650 - \frac{1}{15}650$$

$= 65$ cm – 43.333 cm ..... Dipakai dimensi 60 cm

$$b = \frac{1}{2}h - \frac{2}{3}h = \frac{1}{2}60 - \frac{2}{3}60$$

$= 30$ cm – 40 cm ..... Dipakai dimensi 40 cm

Balok anak = $\frac{b}{h} = \frac{40}{60} = 0.6 > 0.3$ ...(ok)

Jadi, untuk bentang 6.5 m dipakai dimensi balok induk 40/60

- Untuk balok bentang 8 m (B3)

$h = \frac{1}{10}L - \frac{1}{15}L = \frac{1}{10}800 - \frac{1}{15}800$

= 80 cm – 53.333 cm ..... Dipakai dimensi 60 cm

$b = \frac{1}{2}h - \frac{2}{3}h = \frac{1}{2}60 - \frac{2}{3}60$

= 30 cm – 40 cm ..... Dipakai dimensi 35 cm

Balok anak = $\frac{b}{h} = \frac{35}{60} = 0.6 > 0.3$ ...(ok)

Jadi, untuk bentang 8 m dipakai dimensi balok induk 35/60

3.5.3 Perencanaan Dimensi Kolom.

Pada perencanaan dimensi kolom menurut SNI 2847-2013 tentang kekutan lentur minimum kolom harus memenuhi pasal 21.6.1.1 dimensi penampang terpendek tidak boleh kurang dari 300mm, dan pasal 21.6.1.2 rasio dimensi penampang terpendek terhadap dimensi tegak lurus tidak boleh kurang dari 0.4.

- Direncanakan kolom k1 berukuran 80/120

Kontrol = $\frac{80}{120} = 0.6 > 0.4$ ...(ok)

- Direncanakan kolom k2 berukuran 70/90

Kontrol = $\frac{70}{90} = 0.7 > 0.4$ ...(ok)

- Direncanakan kolom k3 berukuran 65/85

Kontrol = $\frac{65}{85}$ = 0.7 > 0.4 ...(ok)

3.6 Perhitungan Pembebanan

3.6.1 Beban Mati (Dead Load)

a. Beban berat sendiri : Untuk berat sendiri pada perhitungan struktur ini menggunakan perintah selfweight (Y = -1) pada program bantu Staad Pro 2004.

b. Beban tembok

- Tembok lantai 2-15 setinggi 4 m dengan tebal setengah batu

  Berat tembok = tinggi tembok x berat jenis (sesuai PPIUG 1983)

  = 3.5 m x 250 kg/m^2

  = 875 kg/m

- Beban urugan pasir bawah keramik (t = 5 cm)

  Beban urugan pasir = Tebal urukan pasir x berat jenis

  = 0.05 x 1600 Kg/m^3

  = 80 Kg/m^2

- Beban keramik + adukan (t = 3 cm)

  Beban keramik = tebal (keramik+adukan) x berat jenis

  = 0.03 x 2200 Kg/m^3

  = 66 Kg/m^2

- Beban plafon dan rangka plafon

Beban plafon = Berat plafon + berat penggantung

$= 11 kg/m^2 + 7 kg/m^2$

$= 18 kg/m^2$

c. Beban Plat Lantai

Untuk beban plat lantai (qd) terdiri dari total jumlah beban tembok seperti diatas, jadi qd = 1164

3.6.2 Beban Hidup (Live Load)

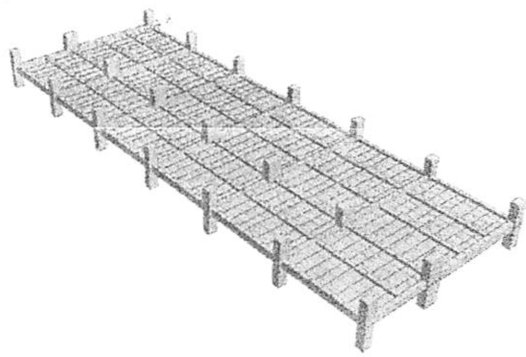
Menurut Peraturan Pembebanan Indonesia Untuk Gedung, 1987 beban hidup untuk lantai gedung yang berfungsi sebagai hotel adalah 250 kg/m^2 sedangkan untuk lantai atap adalah 100 kg/m^2, kemudian untuk beban hidup pada tangga sebesar 300 kg/m^2.

3.6.3 Beban gempa

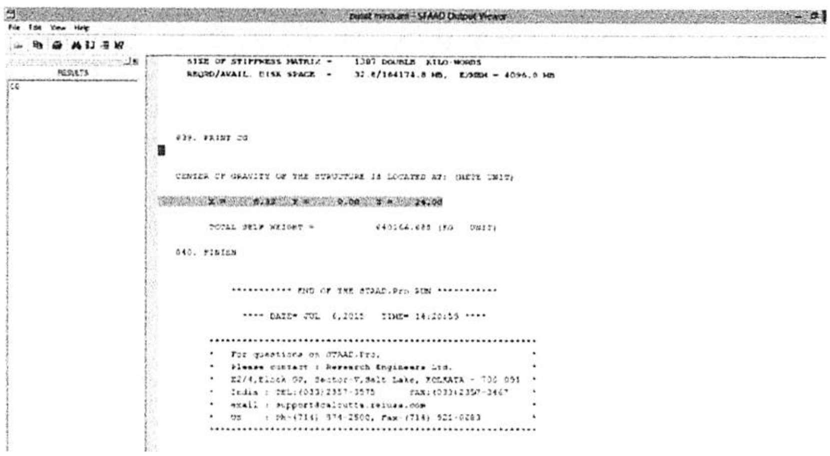
Analisis struktur terhadap beban gempa mengacu pada Standar Perencanaan Ketahanan Gempa untuk Rumah dan Gedung (SNI 03-1726-2012). Pada Skripsi kali ini Analisis struktur terhadap beban gempa pada gedung dilakukan dengan metode analisis Statik Ekuivalen. Langkah-langkah pemberian beban gempa adalah sebagai berikut :

a. Mencari Pusat Massa (CG) dan Berat Tiap Lantai

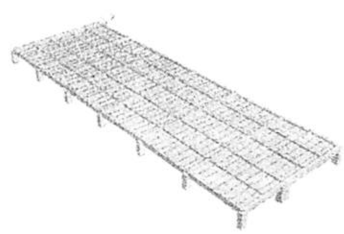
Untuk hasil perhitungan pusat masa dan berat tiap lantai bisa dilihat dari tabel dibawah ini:



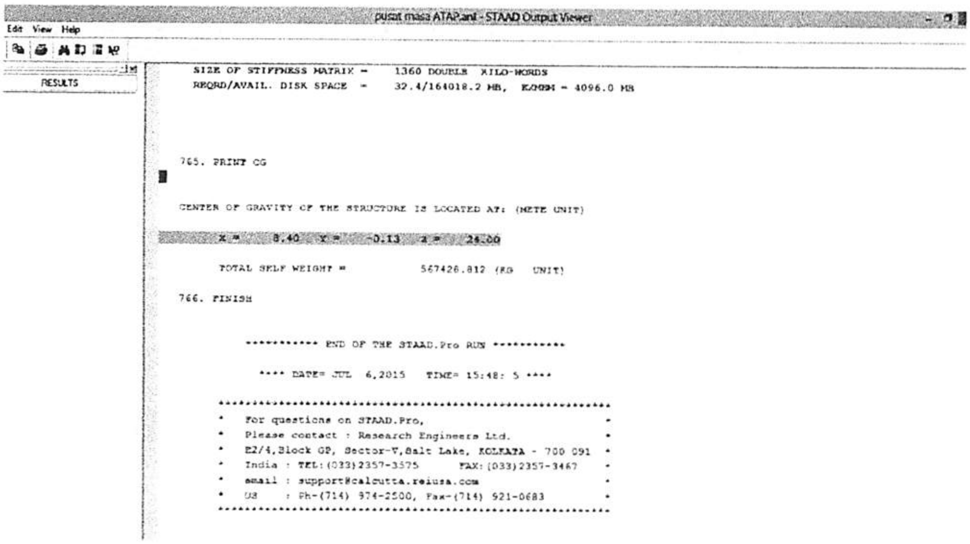
Gambar 3.1 pemodelan struktur lantai 2-16



Gambar 3.2 output pusat masa struktur lantai 2-16



Gambar 3.3 pemodelan struktur lantai atap



Gambar 3.4 output pusat masa lantai atap

Dari hasil perhitungan STAADPRO, didapatkan kooordinasi pusat masa pada tiap lantai berdasarkan sumbu X, Y, dan Z.

Tabel 3.1 perhitungan pusat masa hasil STAAD Pro

Lantai	PUSAT MASSA		
	X	Y	Z
17	8.642	63.867	23.9724
16	8.544	59.684	23.9756
15	8.544	55.684	23.9756
14	8.544	51.684	23.9756
13	8.544	47.684	23.9756
12	8.544	43.684	23.9756
11	8.544	39.684	23.9756
10	8.544	35.684	23.9756
9	8.544	31.684	23.9756
8	8.544	27.684	23.9756
7	8.544	23.684	23.9756
6	8.544	19.684	23.9756
5	8.544	15.684	23.9756
4	8.544	11.684	23.9756
3	8.544	7.684	23.9756
2	8.544	3.684	23.9781

Untuk menentukan berat lantai bisa di lihat seperti dibawah ini :

- Lantai Atap

  Berat lantai = luas lantai x qd lantai

  = (48.000 x 16.650) x 1164 = 905126 kg

  Beban Mati

  Elemen horisontal

  Berat balok = A x L x Bj x ∑ balok

  Berat balok memanjang

  Balok (45/80) = 0.36 x 10.13 x 2400 x 7 = 61236 kg

  Balok (40/60) = 0.24 x 6.52 x 2400 x 7 = 26309 kg

  Balok (20/40) = 0.8 x 3.4 x 2400 x 6 = 3916.8 kg

  Balok (20/40) = 0.8 x 3.175 x 2400 x 6 = 3657.6 kg

  Balok (20/40) = 0.8 x 3.790 x 2400 x 6 = 4366.1 kg

  Balok (20/40) = 0.8 x 3.160 x 2400 x 6 = 3640.3 kg

  Balok (20/40) = 0.8 x 3.170 x 2400 x 6 = 3651.8 kg

  Berat balok melintang

  Balok (35/65) = 0.228 x 8 x 2400 x 6 = 152880 kg

  Elemen Vertikal

  Berat Kolom = A (h Lantai atap + $\frac{1}{5}$ Lantai bawah) x Bj x ∑

  Kolom

  Kolom (70/140) = 0.98 x 2.12 x 2400 x 7 = 34903.6 kg

  Kolom (70/90) = 0.36 x 2.12 x 2400 x 7 = 22438 kg

  Kolom (65/85) = 0.63 x 2.12 x 2400 x 7 ≐ 12821.7 kg

Berat Dinding = b x h x L x Bj

Memanjang = 0.15 x 2 x 2 x 250 = 5400 kg

Melintang = 0.15 x 2 x 2 x 250 = 1873 kg +

Wd Lantai = 448618 kg

Beban Hidup

Beban Hidup =250 kg/m^2

Reduksi Gempa = 0.5

Wi Lantai atap = 250 x 0.5 x (48.0 x 16.2) = 97200 kg

Beban total lantai = Wd + Wi

= 448618 + 97200 = 545818.4 kg

- Lantai 2-16

Baban mati

Berat lantai = luas lantai x qd lantai

= (48.000 x 16.650) x 1164 = 905126 kg

Elemen horisontal

Berat balok = A x L x Bj x $\sum$ balok

Berat balok memanjang

Balok (45/80) = 0.36 x 10.13 x 2400 x 7 = 61236 kg

Balok (40/60) = 0.24 x 6.52 x 2400 x 7 = 26309 kg

Balok (20/40) = 0.8 x 3.4 x 2400 x 6 = 3916.8 kg

Balok (20/40) = 0.8 x 3.175 x 2400 x 6 = 3657.6 kg

Balok (20/40) = 0.8 x 3.790 x 2400 x 6 = 4366.1 kg

Balok (20/40) = 0.8 x 3.160 x 2400 x 6 = 3640.3 kg

Balok (20/40) = 0.8 x 3.170 x 2400 x 6 = 3651.8 kg

Berat balok melintang

Balok (35/65) = 0.228 x 8 x 2400 x 6 = 152880 kg

Elemen Vertikal

Berat Kolom = A ($\frac{1}{5}$ Lantai atas + $\frac{1}{5}$ Lantai bawah) x Bj x $\sum$

Kolom

Kolom (70/140) = 0.98 x 4 x 2400 x 7 = 69586 kg

Kolom (70/90) = 0.36 x 4 x 2400 x 7 = 42336 kg

Kolom (65/85) = 0.63 x 4 x 2400 x 7 = 24192 kg

Berat Dinding = b x h x L x Bj

Memanjang = 0.15 x 4.5 x 777.60 x 250 = 131220 kg

Melintang = 0.15 x 4.5 x 777.60 x 250 = 131220 kg +

Wd Lantai = 523658.8 kg

Beban Hidup

Beban Hidup =250 kg/m^2

Reduksi Gempa = 0.5

Wi Lantai atap = 250 x 0.5 x (48.0 x 16.2) = 97200 kg

Beban total lantai = Wd + Wi

= 523658.8 + 97200 = 620858.8 kg

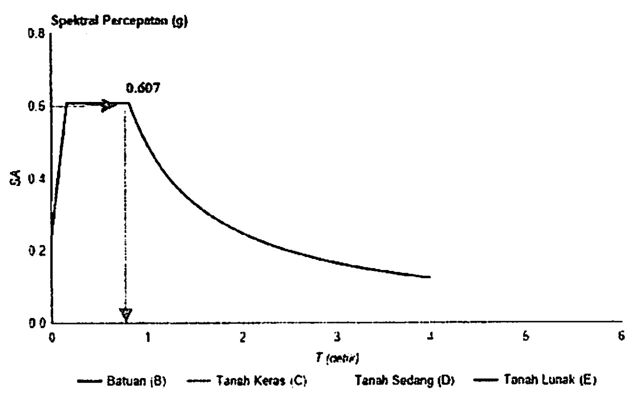
Tabel 3.1 Hasil Perhitungan Berat Tiap Lantai

Lantai	Tinggi hx (m)	Berat Lantai Wx (Kg)
17	64	545818.438
16	60	620858.875
15	56	620858.875
14	52	620858.875
13	48	620858.875
12	44	620858.875
11	40	620858.875
10	36	620858.875
9	32	620858.875
8	28	620858.875
7	24	620858.875
6	20	620858.875
5	16	620858.875
4	12	620858.875
3	8	620858.875
2	4	620858.875
Σ		**9858701.563**

b. Respons Spektrum

Diasumsikan pada Kota Surabaya termasuk tanah sedang maka :

T	C
0	0,224
0,163	0,60
0,817	0,607
1	0.56
1.5	0,211
2	0,247
3	0,163
4	0,112



Sumber puskim.go.id

Dengan :

C : Faktor respon gempa dinyatakan dalam percepatan grafitasi.

T : Waktu getar alami struktur gedung dinyatakan dalam detik.

c. Menetukan periode Fundamental Struktur (T)

Ta untuk struktur dengan ketinggian > 12 tingkat dimana system penahan penahan gaya seismic terdiri dari rangka penahan momen beton atau baja seacara keseluruhan dan tinggi tingkat paling sedikit 3 m.

$$T_a = C_t h_n^x$$

Dimana :

h_n adalah ketinggian struktur, dalam (m), di atas dasar sampai tingkat tertinggi struktur, dan koefisien C_t dan x ditentukan pada tabel 15.

Tabel 14 Koefisien untuk batas atas pada perioda yang dihitung

Parameter percepatan respons spektral desain pada 1 detik, S_{D1}	Koefisien C_u
≥ 0,4	1,4
0,3	1,4
0,2	1,5
0,15	1,6
≤ 0,1	1,7

Tabel 15 Nilai parameter perioda pendekatan C_t dan x

Tipe struktur	C_t	x
Sistem rangka pemikul momen di mana rangka memikul 100 persen gaya gempa yang disyaratkan dan tidak dilingkupi atau dihubungkan dengan komponen yang lebih kaku dan akan mencegah rangka dari defleksi jika dikenai gaya gempa:		
Rangka baja pemikul momen	0,0724 [a]	0,8
Rangka beton pemikul momen	0,0466 [a]	0,9
Rangka baja dengan bresing eksentris	0,0731 [a]	0,75
Rangka baja dengan bresing terkekang terhadap tekuk	0,0731 [a]	0,75
Semua sistem struktur lainnya	0,0488 [a]	0,75

Sumber : SNI-03-1726-2012, Pasal : 7.8.2.1

Ta $= 0,0724 \; x \; 64,4^{0,8}$

$= 0,202$

$Ta_{(max)}$ = Cu Ta

= 1,5. 0,202

= 0,303

$S_{D1} = 0.313$ g ; $S_{Ds} = 0.561$ g

Control :

Ts = S_{Ds} / S_{D1}

= 0,618 detik

35Ts = 1.952 detik

Tc < 3.5Ts, sehingga digunakan prosedur analisa gempa statik

d. Menetukan Gaya Geser Horizontal Akibat Gempa (V)

Mencari gaya geser total yang terjadi pada struktur menurut SNI 1726-2012 pasal 7.8.1 dapat dihitung dengan rumus

$V = Cs x Wt$

Dimana:

V = Beban geser dasar nominal

C_s = Koefisien respon seismic $(Sds / R.Ie)$

Wt = Berat Total Gedung

Sebelum menghitung beban geser nominal terlebih dahulu menentukan kategori resiko bangunan dan faktor keutamaan *Ie* dari tabel 1. Kategori resiko bangunan gedung dan non gedung untuk beban gempa SNI 03-1726-2012 pasal 4.1.2. Serta menentukan faktor R sesuai Tabel 9-faktor untuk penahan gaya gempa pasal 7.2.2 SNI 1726:2012. Sehingga didapat :

R : 8 dan *Ie* : 1,0

Selanjutnya mencari berat total lantai (Wt), untuk berat total lantai didapat dengan bantuan program bantu staad proo 2004 dengan cara mencari reaksi tiap lantai dari lantai 2 sampai lantai 16, kemudian dari hasil tiap lantai dijumlahkan semua sehingga mendapatkan berat total struktur tersebut. Seperti pada tabel 3.1 diatas dengan

nilai Wt = 9931442,5 g.

Dari data diatas maka beban geser dasar nominal struktur dapat dihitung. Perhitungan beban geser :

$$V = \frac{Sds}{R / Ie} x Wt$$

$$= \frac{0,561}{8/1,0} x 9931442,5 = 696442,405 \text{ kg} = 696.442405 \text{ ton}$$

e. Menentukan Gaya Geser Tiap Lantai

Menentukan gaya geser masing-masing lantai setelah ditentukan nilai *Vtotal*, dengan persamaan sebagai berikut ;

$Fi = C_{vx} \text{ x } V$ atau $Fi = \frac{Wi \times hi}{\sum_{i=1}^{n} Wixhi} xV$

Dimana :

F_i = Gaya lantai ke-i

W_i = Berat lantai ke-i

h_i = Ketinggian tingkat lantai ke-i

V = Beban geser dasar nominal

Perhitungan pada Lantai 2

$$F_2 = \frac{69359.813 \times 4}{2774399,25} x 84221,265$$

$$= 1843,05 \text{ kg} = 1,84305 \text{ ton}$$

Dengan perhitungan yang sama berikut ini adalah tabel gaya geser masing-masing lantai (F_i)

Tabel 3.5 Gaya Geser Masing-Masing Lantai (F_i)

LANTAI	ELEVASI(hi)	BERAT(Wi)	Wi x hi^kx	Wi x hi^ky	Fix=Wi x hi^kx/Σwi x hi*Vx	Fiy=Wi x hi^ky/Σwi x hi*Vy
2	4	693599.813	5457254.833	2990062.029	1843.05	4352.17
3	8	620858.875	13702215.522	5557119.551	4627.57	8088.64
4	12	620858.875	25050395.544	8520202.699	8460.12	12401.54
5	16	620858.875	38434694.004	11538128.074	12980.32	16794.26
6	20	620858.875	53570478.696	14597500.844	18092.03	21247.32
7	24	620858.875	70266321.957	17690314.031	23730.62	25749.05
8	28	620858.875	88382044.618	20811216.089	29848.74	30291.66
9	32	620858.875	107809259.080	23956367.724	36409.77	34869.57
10	36	620858.875	128460916.089	27122875.435	43384.33	39478.56
11	40	620858.875	150265112.455	30308477.757	50748.13	44115.35
12	44	620858.875	173161155.915	33511357.018	58480.67	48777.29
13	48	620858.875	197096927.781	36730019.407	66564.36	53462.19
14	52	620858.875	222027039.931	39963214.843	74983.85	58168.25
15	56	620858.875	247911502.695	43209881.380	83725.66	62893.92
16	60	620858.875	274714734.804	46469105.377	92777.75	67637.87
17	64	545818.438	265854492.238	43728229.576	89785.44	63648.40
	Σ	9931442.500	2062164546	406704071.834	696442.41	591976.04

3.7 Kombinasi Beban

Sesuai dengan ketentuan yang tertera dalam SNI 03 – 2847 – 2013 pasal 11, agar struktur dan komponen struktur harus direncanakan hingga semua penampang mempunyai kuat rencana minimum sama dengan kuat perlu, yang dihitung berdasarkan kombinasi dan gaya terfaktor.

- U = 1,4 D
- U = 1,2 D + 1,6 L + 0,5 (Lr aau R)
- U = 1,2 D + 1,6 (Lr atau R) + (1,0 L atau W)
- U = 1,2 D + 1,0 W + 1,0 L + 0,5 (Lr atau R)
- U = 1,2 D + 1,0 E + 1,0 L

- U = 0,9 D + 1,0 W
- U = 0,9 D + 1,0 E

Dimana :

U = Kombinasi Pembebanan

D = Beban mati

L = Beban hidup

E= Beban Gempa

Lr = Beban Atap

W = Beban Angin

R = Beban Hujan

3.8 Simpangan Antarlari (stori drift)

Berdasarkan SNI 1726-2012, simpangan antar lari hanya ada kondisi kinerja batas ultimit saja.

Perhitungan simpangan antar lari (stori drift) kinerja batas ultimit pada lantai atap :

- Nilai perpindahan elastis (total drift) dari STAAd PRO yang dihitung akibat keseluruhan beban gempa pada lantai atap yaitu 1,506 cm = 15,068mm.

  Perhitungan simpangan antar lari (stori drift) kinerja batas ultimit pada lantai 16 :

- Nilai perpindahan elastis (total drift) dari STAAd PRO yang dihitung akibat keseluruhan beban gempa pada lantai atap yaitu 1,324 cm = 13,24 mm.

Perhitungan simpangan antar lantai untuk lantai atap dengan menghitung selisih antara nilai perpindahan elastis lantai atap dengan perpindahan elastis lantai 16 yaitu :

$(\delta_e atap - \delta_{e16})$ = 15,068 mm – 13,24 mm = 1,828 mm

Hitung nilai perpindahan antar lantai (stori drift) yang di perbesar dengan persamaan :

$\frac{(\delta_e atap - \delta_{e3})CD}{Ie} = \frac{15{,}06x5{,}5}{1{,}0} = 10{,}5$ 1mm

Dimana diketahui :

Cd = pembesaran defleksi (5,5)

Ie = Faktor keutamaan gempa (1,0)

Simpangan antar lantai tingkat deain tidak boleh melebihi simpangan antar lantai tingkat ijin (Δ_a). Untuk gedung dengan kategori resiko II dan merupakan struktur rangka pemikul momen, maka digunakan rumus simpangan antar lantai ijin Δ_a = 0,020 h_{sx} sesuai SNI 1726-2012 pasal 7.12.1 tabel 16.

Untuk lantai atap, dengan tinggi kolom di bawahnya sebesar 4 m, jadi simpangan antar lantai ijinya adalah sebesar :

Δ_a = 0,020 x h

Δ_a = 0,020 x 4 m = 0,080 m atau 80 mm.

Kontrol ijin simpangan anatr lantai tingkat desain harus lebih kecil dari simpangan antar lantai tingkat ijin.

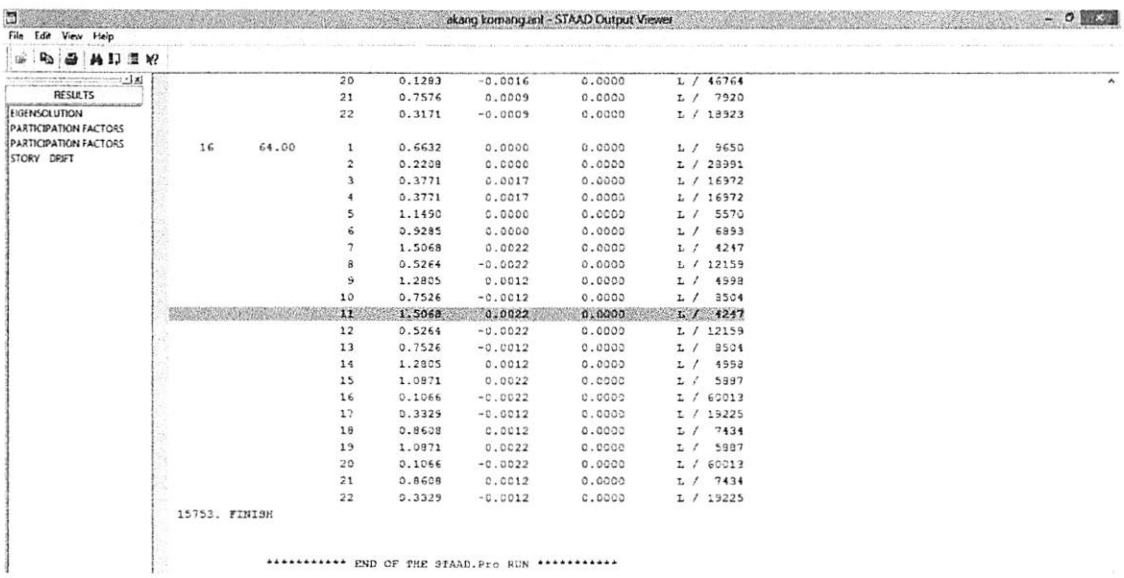
15,675 mm < 80 mm … ….(ok)

Untuk perhitungan simpangan antar lantai (stori drift) selanjutnya, dapat di lihat di tabel dibawah ini :

Tabel 3.6 perhitungan story drift kinerja batas ultimit

Lantai	Tinggi lantai (m)	Total drift (mm)	Perpindahan (mm)	Stori drift (mm)	Stori ijin (mm)	Kontrol
Atap	4	15,068	1,83	10,054	80	Ok
16	4	13,24	1,99	10,923	80	Ok
15	4	11,254	2,06	11,319	80	Ok
14	4	9,196	1,83	10,801	80	Ok
13	4	7,363	0,67	3,663	80	Ok
12	4	6,697	-0,11	-0,583	80	Ok
11	4	6,803	-0,13	-0,698	80	Ok
10	4	6,930	0,02	0,11	80	Ok
9	4	6,910	0,24	1,298	80	Ok
8	4	6,674	0,49	2,700	80	Ok
7	4	6,183	0,77	4,213	80	Ok
6	4	5,417	0,58	3,212	80	Ok
5	4	4,833	1,70	9,355	80	Ok
4	4	3,132	1,35	7,419	80	Ok
3	4	1,783	1,19	6,561	80	Ok
2	4	0,590	0,59	3,245	80	Ok

Nilai perpindahan elastis (total drift) dari STAAD Pro yang dihitung akibat gaya gempa bisa dilihat pada gambar dibawah ini :



RESULTS
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STORY DRIFT

		20	0.1703	-0.0013	0.0000	L / 32876
		21	0.6454	0.0007	0.0000	L / 8676
		22	0.3129	-0.0007	0.0000	L / 17899
15	60.00	1	0.5970	0.0000	0.0000	L / 10050
		2	0.1990	0.0000	0.0000	L / 30158
		3	0.3146	0.0013	0.0000	L / 19070
		4	0.3146	0.0013	0.0000	L / 19070
		5	1.0347	0.0000	0.0000	L / 5798
		6	0.8358	0.0000	0.0000	L / 7178
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
		8	0.5064	-0.0016	0.0000	L / 11849
		9	1.1356	0.0009	0.0000	L / 5283
		10	0.6951	-0.0009	0.0000	L / 8631
		11	1.3244	0.0016	0.0000	L / 4530
		12	0.5064	-0.0016	0.0000	L / 11849
		13	0.6951	-0.0009	0.0000	L / 8631
		14	1.1356	0.0009	0.0000	L / 5283
		15	0.9463	0.0016	0.0000	L / 6340
		[illegible]	[illegible]	[illegible]	[illegible]	[illegible]

STORY DRIFT

		3	0.2375	0.0010	0.0000	L / 23574
		4	0.2375	0.0010	0.0000	L / 23574
		5	0.9232	0.0000	0.0000	L / 6066
		6	0.7453	0.0000	0.0000	L / 7513
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
		8	0.5078	-0.0013	0.0000	L / 11029
		9	0.9828	0.0007	0.0000	L / 5698
		10	0.6503	-0.0007	0.0000	L / 8611
		11	1.1254	0.0013	0.0000	L / 4976
		12	0.5078	-0.0013	0.0000	L / 11029
		13	0.6503	-0.0007	0.0000	L / 8611
		14	0.9828	0.0007	0.0000	L / 5698
		15	0.7879	0.0013	0.0000	L / 7107
		16	0.1703	-0.0013	0.0000	L / 32876
		17	0.3129	-0.0007	0.0000	L / 17899
		18	0.6454	0.0007	0.0000	L / 8676
		19	0.7879	0.0013	0.0000	L / 7107
		20	0.1703	-0.0013	0.0000	L / 32876
		21	0.6454	0.0007	0.0000	L / 8676
		22	0.3129	-0.0007	0.0000	L / 17899
15	60.00	1	0.5970	0.0000	0.0000	L / 10050
		2	0.1990	0.0000	0.0000	L / 30158
		3	0.3146	0.0013	0.0000	L / 19070
		4	0.3146	0.0013	0.0000	L / 19070

PARTICIPATION FACTORS
PARTICIPATION FACTORS
STORY DRIFT

		--	------	------	------	- . -----
13	52.00	1	0.4705	0.0000	0.0000	L / 11053
		2	0.1574	0.0000	0.0000	L / 33042
		3	0.1521	0.0010	0.0000	L / 34196
		4	0.1521	0.0010	0.0000	L / 34196
		5	0.8164	0.0000	0.0000	L / 6370
		6	0.6587	0.0000	0.0000	L / 7895
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
		8	0.5243	-0.0013	0.0000	L / 9919
		9	0.8284	0.0007	0.0000	L / 6277
		10	0.6155	-0.0007	0.0000	L / 8448
		11	0.9196	0.0013	0.0000	L / 5654
		12	0.5243	-0.0013	0.0000	L / 9919
		13	0.6155	-0.0007	0.0000	L / 8448
		14	0.8284	0.0007	0.0000	L / 6277
		15	0.6211	0.0013	0.0000	L / 8372
		16	[illegible]	[illegible]	[illegible]	[illegible]

EIGENSOLUTION
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PARTICIPATION FACTORS
STORY DRIFT

		21	0.3863	0.0008	0.0000	L / 11390
		22	0.2495	-0.0008	0.0000	L / 17638
12	48.00	1	0.4107	0.0000	0.0000	L / 11689
		2	0.1377	0.0000	0.0000	L / 34855
		3	0.0814	0.0010	0.0000	L / 58967
		4	0.0814	0.0010	0.0000	L / 58967
		5	0.7131	0.0000	0.0000	L / 6731
		6	0.5749	0.0000	0.0000	L / 8349
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
		8	0.5247	-0.0013	0.0000	L / 9148
		9	0.6875	0.0007	0.0000	L / 6982
		10	0.5735	-0.0007	0.0000	L / 8369
		11	0.7363	0.0013	0.0000	L / 6519
		12	0.5247	-0.0013	0.0000	L / 9148
		13	0.5735	-0.0007	0.0000	L / 8369
		14	0.6875	0.0007	0.0000	L / 6982
		15	0.4754	0.0013	0.0000	L / 10096
		16	0.2638	-0.0013	0.0000	L / 18196
		17	0.3126	-0.0007	0.0000	L / 15355

RESULTS
EIGENSOLUTION
PARTICIPATION FACTORS
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STORY DRIFT

		22	0.1494	-0.0008	0.0000	L / 26767
11	44.00	1	0.3532	0.0000	0.0000	L / 12457
		2	0.1188	0.0000	0.0000	L / 37036
		3	0.0977	0.0011	0.0000	L / 45015
		4	0.0977	0.0011	0.0000	L / 45015
		5	0.6139	0.0000	0.0000	L / 7167
		6	0.4945	0.0000	0.0000	L / 8898
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
		8	0.4156	-0.0015	0.0000	L / 10587
		9	0.6111	0.0008	0.0000	L / 7200
		10	0.4742	-0.0008	0.0000	L / 9278
		11	0.6697	0.0015	0.0000	L / 6570
		12	0.4156	-0.0015	0.0000	L / 10587
		13	0.4742	-0.0008	0.0000	L / 9278

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EIGENSOLUTION
PARTICIPATION FACTORS
PARTICIPATION FACTORS
STORY DRIFT

		22	0.0537	-0.0005	0.0000	L / 67056
10	40.00	1	0.2985	0.0000	0.0000	L / 13401
		2	0.1008	0.0000	0.0000	L / 39699
		3	0.1703	0.0012	0.0000	L / 23489
		4	0.1703	0.0012	0.0000	L / 23489
		5	0.5194	0.0000	0.0000	L / 7701
		6	0.4179	0.0000	0.0000	L / 9572
[illegible]						
		8	0.2376	-0.0015	0.0000	L / 16837
		9	0.5781	0.0008	0.0000	L / 6919
		10	0.3397	-0.0008	0.0000	L / 11774
		11	0.6803	0.0015	0.0000	L / 5879
		12	0.2376	-0.0015	0.0000	L / 16837
		13	0.3397	-0.0008	0.0000	L / 11774
		14	0.5781	0.0008	0.0000	L / 6919
		15	0.4900	0.0015	0.0000	L / 8163
		16	0.0473	-0.0015	0.0000	L / 84634
		17	0.1494	-0.0008	0.0000	L / 26767
		18	0.3878	0.0008	0.0000	L / 10313

RESULTS
EIGENSOLUTION
PARTICIPATION FACTORS
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		22	-0.0279	-0.0009	0.0000	L /114553
9	36.00	1	0.2469	0.0000	0.0000	L / 14579
		2	0.0837	0.0000	0.0000	L / 43007
		3	0.2408	0.0012	0.0000	L / 14951
		4	0.2408	0.0012	0.0000	L / 14951
		5	0.4302	0.0000	0.0000	L / 8367
		6	0.3457	0.0000	0.0000	L / 10414
		7	0.6930	0.0016	0.0000	L / 5194
		8	0.0670	-0.0016	0.0000	L / 53728
[illegible]						
		10	0.2115	-0.0009	0.0000	L / 17024

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STORY	HEIGHT	LOAD	DRIFT(CM)		ECCENTRICITY	RATIO
	(METE)		X	Z	(METE)	

BASE=

EIGENSOLUTION
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PARTICIPATION FACTORS
STORY DRIFT

		3	0.2957	0.0013	0.0000	L / 10821
		4	0.2957	0.0013	0.0000	L / 10821
		5	0.3472	0.0000	0.0000	L / 9216
		6	0.2785	0.0000	0.0000	L / 11488
		7	0.6910	0.0017	0.0000	L / 4631
		8	-0.0779	-0.0017	0.0000	L / 41080
		9	0.5136	0.0009	0.0000	L / 6231
[illegible]						
		11	0.6910	0.0017	0.0000	L / 4631
		12	-0.0779	-0.0017	0.0000	L / 41080
		13	0.0995	-0.0009	0.0000	L / 32145
		14	0.5136	0.0009	0.0000	L / 6231
		15	0.5635	0.0017	0.0000	L / 5679
		16	-0.2054	-0.0017	0.0000	L / 15582
		17	-0.0279	-0.0009	0.0000	L /114553
		18	0.3861	0.0009	0.0000	L / 8288
		19	0.5635	0.0017	0.0000	L / 5679
		20	-0.2054	-0.0017	0.0000	L / 15582
		21	0.3861	0.0009	0.0000	L / 8288
		22	-0.0279	-0.0009	0.0000	L /114553
9	36.00	1	0.2469	0.0000	0.0000	L / 14579
		2	0.0837	0.0000	0.0000	L / 43007
		3	0.2408	0.0012	0.0000	L / 14951

RESULTS
EIGENSOLUTION
PARTICIPATION FACTORS
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STORY DRIFT

7	28.00	1	0.1551	0.0000	0.0000	L / 18055
		2	0.0532	0.0000	0.0000	L / 52660
		3	0.3293	0.0014	0.0000	L / 8501
[illegible]						
		5	0.2712	0.0000	0.0000	L / 10326
		6	0.2171	0.0000	0.0000	L / 12897
		7	0.6674	0.0018	0.0000	L / 4195

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STAAD SPACE -- PAGE NO. 290

STORY	HEIGHT	LOAD	DRIFT(CM)		ECCENTRICITY	RATIO
	(METE)		X	Z	(METE)	

BASE=

		9	-0.1995	-0.0012	0.0000	L / 14323

EIGENSOLUTION
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STORY DRIFT

		21	0.2971	0.0010	0.0000	L / 6732
		22	-0.1506	-0.0010	0.0000	L / 13284
6	24.00	1	0.1157	0.0000	0.0000	L / 20737
		2	0.0400	0.0000	0.0000	L / 60019
		3	0.3381	0.0014	0.0000	L / 7099
		4	0.3381	0.0014	0.0000	L / 7099
		5	0.2029	0.0000	0.0000	L / 11831
		6	0.1620	0.0000	0.0000	L / 14812
		7	0.6183	0.0019	0.0000	[illegible]
		8	-0.2606	-0.0019	0.0000	L / 9209
		9	0.4155	0.0010	0.0000	L / 5776
		10	-0.0578	-0.0010	0.0000	L / 41540
		11	0.6183	0.0019	0.0000	L / 3881
		12	-0.2606	-0.0019	0.0000	L / 9209
		13	-0.0578	-0.0010	0.0000	L / 41540
		14	0.4155	0.0010	0.0000	L / 5776
		15	0.5436	0.0015	0.0000	L / 4415

PARTICIPATION FACTORS
STORY DRIFT

BASE=

		5	0.1431	0.0000	0.0000	L / 13974
		6	0.1140	0.0000	0.0000	L / 17550
		[illegible]	[illegible]	0.0019	0.0000	[illegible]
		8	-0.2896	-0.0019	0.0000	L / 6906
		9	0.3499	0.0010	0.0000	L / 5716
		10	-0.0977	-0.0010	0.0000	L / 20462
		11	0.5417	0.0019	0.0000	L / 3692
		12	-0.2896	-0.0019	0.0000	L / 6906
		13	-0.0977	-0.0010	0.0000	L / 20462
		14	0.3499	0.0010	0.0000	L / 5716
		15	0.4889	0.0019	0.0000	L / 4090
		16	-0.3424	-0.0019	0.0000	L / 5841
		17	-0.1506	-0.0010	0.0000	L / 13284
		18	0.2971	0.0010	0.0000	L / 6732
		19	0.4889	0.0019	0.0000	L / 4090
		20	-0.3424	-0.0019	0.0000	L / 5841
		21	0.2971	0.0010	0.0000	L / 6732
		22	-0.1506	-0.0010	0.0000	L / 13284
6	24.00	1	0.1157	0.0000	0.0000	L / 20737
		2	0.0400	0.0000	0.0000	L / 60019
		3	0.3381	0.0014	0.0000	L / 7099

RESULTS
EIGENSOLUTION
PARTICIPATION FACTORS
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STORY DRIFT

4	16.00	1	0.0525	0.0000	0.0000	L / 30448
		2	0.0186	0.0000	0.0000	L / 86205
		3	0.2744	0.0015	0.0000	L / 5831
		4	0.2744	0.0015	0.0000	L / 5831
		5	0.0928	0.0000	0.0000	L / 17250
		6	0.0736	0.0000	0.0000	L / 21748
		[illegible]	[illegible]	0.0019	0.0000	L / 3650
		8	-0.2751	-0.0019	0.0000	L / 5816
		9	0.2737	0.0010	0.0000	L / 5846
		10	-0.1105	-0.0010	0.0000	L / 14485
		11	0.4383	0.0019	0.0000	L / 3650
		12	-0.2751	-0.0019	0.0000	L / 5816
		13	-0.1105	-0.0010	0.0000	L / 14485
		14	0.2737	0.0010	0.0000	L / 5846
		15	0.4040	0.0019	0.0000	L / 3960
		16	-0.3094	-0.0019	0.0000	L / 5171
		17	-0.1448	-0.0010	0.0000	L / 11051
		18	0.2394	0.0010	0.0000	L / 6684
		19	0.4040	0.0019	0.0000	L / 3960
		20	-0.3094	-0.0019	0.0000	L / 5171

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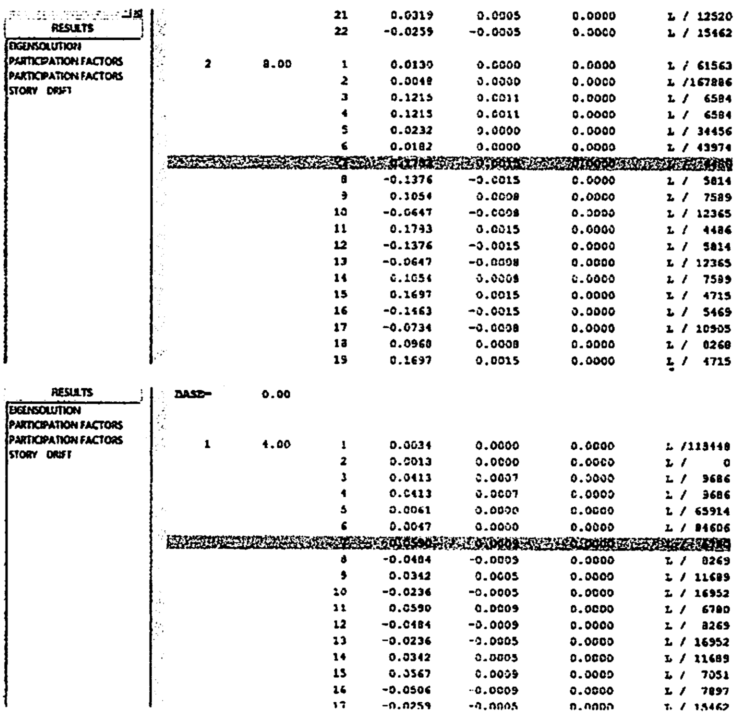
		2	0.0106	0.0000	0.0000	L /112843
		3	0.2054	0.0014	0.0000	L / 5843
		4	0.2054	0.0014	0.0000	L / 5843
		5	0.0526	0.0000	0.0000	L / 22822
		6	0.0415	0.0000	0.0000	L / 28931
		7	0.3132	0.0018	0.0000	[illegible]
		8	-0.2208	-0.0018	0.0000	L / 5435
		9	0.1899	0.0010	0.0000	L / 6318
		10	-0.0976	-0.0010	0.0000	L / 12299
		11	0.3132	0.0018	0.0000	L / 3832
		12	-0.2208	-0.0018	0.0000	L / 5435
		13	-0.0976	-0.0010	0.0000	L / 12299
		14	0.1899	0.0010	0.0000	L / 6318
		15	0.2936	0.0018	0.0000	L / 4087
		16	-0.2403	-0.0018	0.0000	L / 4993
		17	-0.1171	-0.0010	0.0000	L / 10248
		18	0.1704	0.0010	0.0000	L / 7041
		19	0.2936	0.0018	0.0000	L / 4087
		20	-0.2403	-0.0018	0.0000	L / 4993
		21	0.1704	0.0010	0.0000	L / 7041
		22	-0.1171	-0.0010	0.0000	L / 10249
4	16.00	1	0.0525	0.0000	0.0000	L / 30448
		2	0.0186	0.0000	0.0000	L / 86205
		3	0.2744	0.0015	0.0000	L / 5831



RESULTS

EIGENSOLUTION
PARTICIPATION FACTORS
PARTICIPATION FACTORS
STORY DRIFT

		21	0.0319	0.0005	0.0000	L / 12520
		22	-0.0259	-0.0005	0.0000	L / 15462
2	8.00	1	0.0130	0.0000	0.0000	L / 61563
		2	0.0048	0.0000	0.0000	L /167886
		3	0.1215	0.0011	0.0000	L / 6584
		4	0.1215	0.0011	0.0000	L / 6584
		5	0.0232	0.0000	0.0000	L / 34456
		6	0.0182	0.0000	0.0000	L / 43974
		[illegible]	[illegible]	[illegible]	[illegible]	[illegible]
		8	-0.1376	-0.0015	0.0000	L / 5814
		9	0.1054	0.0008	0.0000	L / 7589
		10	-0.0647	-0.0008	0.0000	L / 12365
		11	0.1793	0.0015	0.0000	L / 4486
		12	-0.1376	-0.0015	0.0000	L / 5814
		13	-0.0647	-0.0008	0.0000	L / 12365
		14	0.1054	0.0008	0.0000	L / 7589
		15	0.1697	0.0015	0.0000	L / 4715
		16	-0.1463	-0.0015	0.0000	L / 5469
		17	-0.0734	-0.0008	0.0000	L / 10505
		18	0.0968	0.0008	0.0000	L / 8268
		19	0.1697	0.0015	0.0000	L / 4715

RESULTS

EIGENSOLUTION
PARTICIPATION FACTORS
PARTICIPATION FACTORS
STORY DRIFT

BASE=	0.00					
1	4.00	1	0.0034	0.0000	0.0000	L /113448
		2	0.0013	0.0000	0.0000	L / 0
		3	0.0413	0.0007	0.0000	L / 9686
		4	0.0413	0.0007	0.0000	L / 9686
		5	0.0061	0.0000	0.0000	L / 65914
		6	0.0047	0.0000	0.0000	L / 84606
		[illegible]	0.0590	0.0009	0.0000	[illegible]
		8	-0.0484	-0.0009	0.0000	L / 8269
		9	0.0342	0.0005	0.0000	L / 11689
		10	-0.0236	-0.0005	0.0000	L / 16952
		11	0.0590	0.0009	0.0000	L / 6780
		12	-0.0484	-0.0009	0.0000	L / 8269
		13	-0.0236	-0.0005	0.0000	L / 16952
		14	0.0342	0.0005	0.0000	L / 11689
		15	0.0567	0.0009	0.0000	L / 7051
		16	-0.0506	-0.0009	0.0000	L / 7897
		17	-0.0259	-0.0005	0.0000	L / 15462

Gambar 3.5 nilai perpindahan elastis (total drift) dari STAAD Pro yang dihitung akibat gaya gempa

BAB IV

PERHITUNGAN PENULANGAN STRUKTUR

4.1 Perhitungan Penulangan Balok

4.1.1 Perhitungan Penulangan Lentur Balok

Penulangan yang direncanakan adalah pada balok memanjang line 5

- Data Perencanaan

b = 450 mm

h = 800 mm

f'c = 30 MPa

fy_{ulir} = 400 MPa

fy_{polos} = 240 MPa

selimut beton 40 mm

dipakai tulangan pokok D 22 mm

dipakai tulangan sengkang Ø 10 mm

bentang balok L = 10000 mm

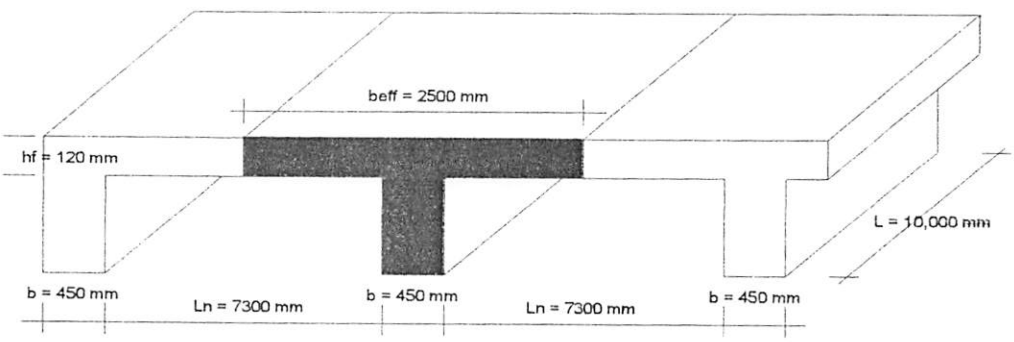
bentang bersih balok (Ln) = 8850 mm

d = h - selimut beton - diameter sengkang - ½ diameter tulangan rencana

= 800 - 40 - 10 - ½ 22

= 739.0 mm

- Perencanaan Penulangan



Gambar 4.1 Lebar efektif balok *(b_{eff})*

Lebar flens efektif (beff)

- beff = ¼ L = ¼ x 10000 = 2500 mm
- beff = bw + 8 hf_{kr} + 8 hf_{kn} = 450+(8.120)+(8.120) = 2586 mn
- beff = bw + ½Ln_{kr} + ½Ln_{kn} = 450+(½.7300)+(½.7300) = 7750 mm

dipakai nilai beff terkecil yaitu = 2500 mm

Tulangan minimal sedikitnya harus dihitung menurut SNI 2847-2013

Pasal 10.5.1 :

$$A_{s\,min} = \frac{0.25\sqrt{fc'}}{fy}\, bw\, d = \frac{0.25 \times \sqrt{30}}{400}\, 450 \times 739 = 1138.4 \text{ nm}^2$$

dan

$$A_{s\,min} = \frac{1.4 \quad bw\, d}{fy} = \frac{1.4 \times 450 \times 739}{400} = 1163.925 \text{ mm}^2$$

Maka dipakai tulangan minimal 4 D 22 (As = 1519.76 mm^2 > 1163.93 mm^2)

A. Perhitungan penulangan tumpuan kiri joint 21807

Mu^- = 400.373 kNm (1,2 + 1,6L)

= 400373000 Nmm

Mu^+ = 128.084 kNm (0,9DL - 0,3EX -1,0EY)

= 128084000 Nmm

Dicoba pemasangan tulangan sebagai berikut :

- Tulangan yang terpasang pada daerah tarik 6 D 22 (As = 3039.52 mm^2)
- Tulangan yang terpasang pada daerah tekan 4 D 22 (As' = 1519.76 mm^2)
- Tulangan bagi plat terpasang di sepanjang beff 6 Ø 10 (As_{plat} = 471.00 mm^2)

Kontrol Momen Negatif

Tulangan tarik As_{plat} = 6 Ø 10 = 471.00 mm^2

As_{balok1} = 4 D 22 = 1519.76 mm^2

As_{balok2} = 2 D 22 = 759.88 mm^2

Tulangan tekan As' = 4 D 22 = 1519.76 mm^2

$y1 = 20 + 1/2 \; 10 = 25$ mm

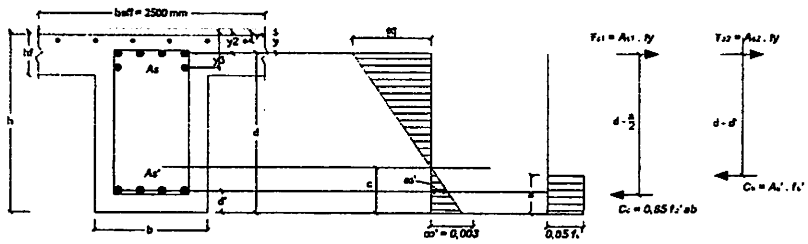
$y2 = 40 + 10 + 1/2 \; 22 = 61$ mm

$y3 = 40 + 10 + 22 + 30 + 1/2 \; 22 = 113$ mm

$$y = \frac{471 \times 25 + 3039.52 \times 61 + 759.88 \times 113}{1990.76} = 142.183$$

$d = 800 - 142.183 = 657.817$ mm

$d' = 40 + 10 + 1/2 \; 22 = 61.0$ mm



Gambar 4.2 Penampang balok dan diagram tegangan momen negatif tumpuan kiri

Dimisalkan garis netral > d' maka perhitungan garis netral harus dicari menggunakan persamaan :

$0,85 \,.\, f'c \,.\, a \,.\, b + As' \,.\, fs' = As \,.\, fy$

Substitusi nilai : $fs' = \dfrac{(c - d')}{c} \times 600$

$$(0,85 \,.\, f'c \,.\, a \,.\, b) + As' \; \frac{(c - d')}{c} \times 600 = \; ; + As_{balok} \,.\, fy_{ulir}$$

$(0,85.f'c'.a.b).c + As'(c\text{-}d').600 = As_{plat}.fy_{polos}.c + As_{balok}.fy_{ulir}.c$

Substitusi nilai : $a = \beta 1.c$

$(0,85.f'c'.\beta 1.c .b).c + As'(c\text{-}d').600 = As_{plat}.fy_{polos}.c + As_{balok}.fy_{ulir}.c$

$(0,85.f'c'.\beta 1.b).c^2 + 600As'.c - 600As'.d' = As_{plat}.fy_{polos}.c + As_{balok}.fy_{ulir}.c$

$(0,85.f'c.\beta 1.b)c^2 + 600As'.c - 600As'.d' - As_{plat}.fy_{polos}.c - As_{balok}.fy_{ulir}.c = 0$

$(0,85.f'c.\beta 1.b)c^2 + (600As' - As_{plat}.fy_{polos} - As_{balok}.fyulir).c - 600As'.d' = 0$

$(0,85.30.0,85.300)c^2$+(600.850,16`-471.240-1133,54.390).c-

600.850,16.59,5 = 0

$9753.75 \; c^2 + 416992 \; c - 55623216 = 0$

$c = 99.860$ mm

$a = \beta . c$

$= 0.85 \times 99.860 = 84.881$ mm

$$\varepsilon s' = \frac{c - d'}{c} \times \varepsilon c = \frac{99.860 - 61.0}{99.860} \times 0.003 = 0.00117$$

$$\varepsilon s = \frac{d - c}{c} \times \varepsilon c = \frac{657.817 - 99.860}{99.860} \times 0.003 = 0.01676$$

$$\varepsilon y = \frac{fy}{Es} = \frac{400}{200000} = 0.0020$$

Karena $\varepsilon s > \varepsilon y > \varepsilon s'$ maka tulangan baja tarik telah leleh,baja tekan belum

Dihitung tegangan pada tulangan baja tekan

$f's = \varepsilon s' \times Es$

$= 0.00117 \times 200000$

$= 233.486 < 400$ MPa

Menghitung gaya tekan dan tarik

$Cc = 0,85 . f'c . a . b$

$= 0.85 \times 30 \times 84.881 \times 450$

$= 974005.977$ N

$Ss = As' \times f's$

$= 1519.76 \times 233.486$

$= 354842.023$ N

$Ts_1 = As_{plat} \times fy_{polos}$

$= 471 \times 240$

$= 113040$ N

$Ts_2 = As_{balok} \times fy_{ulir}$

$= 3039.52 \times 400$

$= 1215808.0$ N

$Cc + Cs = Ts_1 + Ts_2$

$974005.977 + 354842.023 = 113040 + 1215808.0$

$1328848.0 = 1328848.0$

$Z1 = d - (½ . a)$

$= 657.817 - (1/2 . 84.881)$

$= 615.377$ mm

$Z2 = d - d'$

$= 657.817 - 61.0$

$= 596.817$ mm

$Mn = (Cc . Z1) + (Cs . Z2)$

$= 974005.977 \times 615.377 + 354842.023 \times 596.817$

$= 811156335.311$ Nmm

$Mr = \phi\ Mn$

$= 0.9 . 811156335.311$

$= 730040701.780$ Nmm $> Mu = 400373000$ Nmm (Aman)

$Mpr = 1.25 . Mn$

$= 1.25 . 811156335.311$

$= 1013945419.139$ Nmm

Kontrol Momen Positif

Tulangan tekan $As'_{plat} = 6\ Ø\ 10 = 471.00\ mm^2$

$As'_{balok1} = 4\ D\ 22 = 1519.76\ mm^2$

$As'_{balok2} = 2\ D\ 22 = 759.88\ mm^2$

$As' = 471.00 + 1519.76 + 759.88 = 2750.64$ mn

Tulangan tarik $As = 4\ D\ 22 = 1519.76\ mm^2$

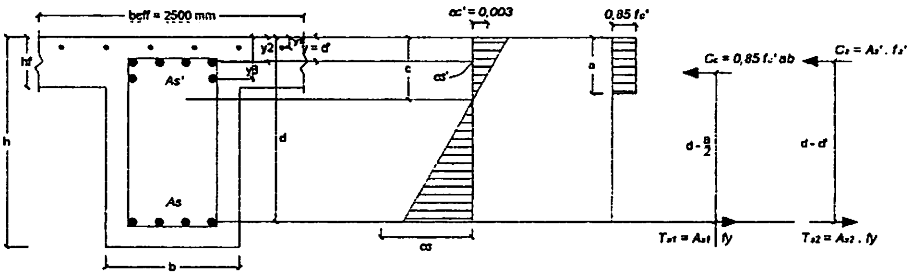
$y1 = 20 + 1/2\ 10 = 25$ mm

$y2 = 40 + 10 + 1/2\ 22 = 61$ mm

$y3 = 40 + 10 + 22 + 30 + 1/2\ 22 = 113$ mm

$$y = d' = \frac{471 \times 25 + 1519.76 \times 61 \times 759.88 \times 113}{2750.64} = 69$$

$d = 800 - 61.0 = 739.0$ mm



Gambar 4.3 Penampang balok dan diagram tegangan momen positif tumpuan kiri

Dimisalkan garis netral > y3 maka perhitungan garis netral harus dicari menggunakan persamaan :

$0,85 \,.\, f'c \,.\, a \,.\, b + As' \,.\, fs' = As \,.\, fy$

Substitusi nilai : $fs' = \dfrac{(c - d')}{c} \times 600$

$$(0,85 \,.\, f'c \,.\, a \,.\, b) + As' \; \frac{(c - d')}{c} \times 600 = As \,.\, fy$$

$$(0,85 \,.\, f'c \,.\, a \,.\, b) \,.\, c + As' \; (c - d') \times 600 = As \,.\, fy \,.\, c$$

Substitusi nilai : $a = \beta 1.c$

$$(0,85 \,.\, f'c \,.\, \beta 1.c \,.\, b) \,.\, c + As' \; (c - d')\, 600 = As \,.\, fy \,.\, c$$

$$(0,85.f'c.\beta 1.b)\, c^2 + 600As'.c - 600As'.d' = As \,.\, fy \,.\, c$$

$$(0,85.f'c.\beta 1.b)\, c^2 + 600As'.c - 600As'.d' - As.\, fy \,.\, c = 0$$

$$(0,85.f'c.\beta 1.b)\, c^2 + (600As' - As \,.\, fy).c - 600As'.d' = 0$$

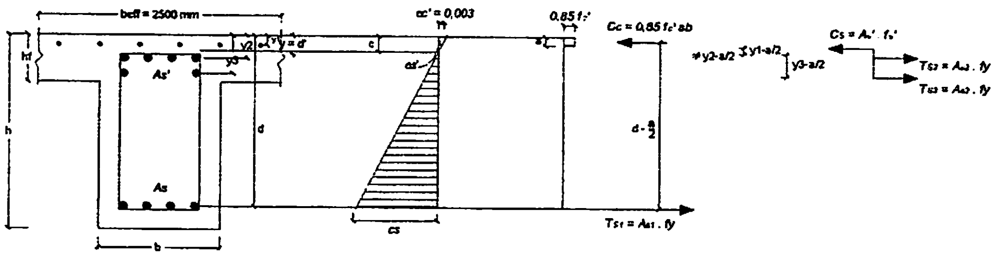
$(0,85.30.0,85.300)c^2 + (600.1604,54-850,16.390).c -$

$600.1604,54.49,373 = 0$

$$9753.75\, c^2 + 1042480.0\, c - 114208080 = 0$$

$c = 23.526$ mm

Karena c < y2, tulangan tekan sebagian mengalami gaya tarik maka nilai c harus dihitung ulang.



Gambar 4.4 Penampang balok dan diagram tegangan momen positif tumpuan kiri yang sudah dihitung ulang

Dimisalkan garis netral diantara y1 dan y3 maka perhitungan garis netral dicari dengan menggunakan persamaan :

$0,85 . f'c . a . beff + As_{plat}' . fs' = As1 . fs + As2 . fy_{ulir} + As3 . fy_{ulir}$

Substitusi nilai : $fs' = \frac{(c - y_1)}{c} \times 600$ dan $fs = fy_{ulir}$

$$(0,85.f'c.a.beff) + As_{plat}' \frac{(c - y_1)}{c} \times 600 = As1 . fy_{ulir} + As2 . fy_{ulir} + As3 . fy$$

$(0,85.f'c.a.beff).c + As_{plat}'.(c-y_1).600 = As1 . fy_{ulir}.c + As2 . fy_{ulir} . c + As3 . fy_{ulir} . c$

Substitusi nilai : $a = \beta 1.c$

$(0,85.f'c.\beta 1.c.beff).c + As_{plat}'.(c-y_1).600 = As1 . fy_{ulir}.c + As2 . fy_{ulir} . c + As3 . fy_{ulir} . c$

$(0,85.f'c.\beta 1.beff).c^2 + 600.As_{plat}'.c - 600.As_{plat}'.y_1 = As1 . fy_{ulir}.c + As2 . fy_{ulir} . c +$

$As3 . fy_{ulir} . c$

$(0,85.f'c.\beta 1.beff).c^2 + (600.As_{plat}' - As1.fy_{ulir} - As2.fy_{ulir} - As3 . fy_{ulir}).c -$

$600.As_{plat}'.y1 = 0$

$(0,85.30.0,85.1350).c^2 + (600.471 - 1133,54.390 - 850,16.390 - 1519.390).c -$

$600.471.25 = 0$

$54187.50 \quad c^2 \quad - \quad 1237160.000 \quad - \quad 7065000 \quad = \quad 0$

$c \quad = \quad 27.562 \quad mm$

$a \quad = \beta . c$

$\quad = \quad 0.85 \quad x \quad 27.562 \quad = \quad 23.427 \quad mm$

$fs' \; = \; \varepsilon s' . Es$

$\quad = \frac{c - y_1}{c} . \varepsilon c . Es$

$$= \frac{27.562 - 25}{27.562} \times 0.003 \times 200000 = 55.765 \text{ MPa}$$

$fs = fy_{ulir} = 400$ MPa

Menghitung gaya tekan dan tarik

Cc = 0,85 . f'c . a . beff

= 0.85 x 30 x 23.427 x 2500

= 1493494.798 N

Cs = As' x f's

= 471.00 x 55.765

= 26265.202 N

Ts_1 = As1 x fy

= 1519.76 x 400

= 607904.000 N

Ts_2 = As2 x fy

= 1519.76 x 400

= 607904.000 N

Ts_3 = As3 x fy

= 759.880 x 400

= 303952.000 N

$C_C + C_S = Ts_1 + Ts_2 + Ts_3$

1493494.798 + 26265.202 = 607904.000 + 607904.00 + 303952.(

1519760.00 = 1519760.00

Z1 = d - (1/2 . a)

= 739.0 - (1/2 . 23.427)

= 727.286 mm

Z2 = y3 - (1/2 . a)

= 142.183 - (1/2 . 23.427)

= 130.469 mm

Z3 = y2 - (1/2 . a)

= 61.00 - (1/2 . 23.427)

= 49.286 mm

Z4 = y1 - (1/2 . a)

= 25.00 - (1/2 . 23.427)

= 13.286 mm

Mn = (Ts1 . Z1) + (Ts2 . Z2) - (Cs . Z3)

= 607904.00 x 727.3 + 607904.00 x 130.5 - 26265.20 x 49

= 520138542.757 Nmm

Mr = ϕ Mn

= 0.9 . 520138542.757

= 468124688.482 Nmm > Mu = 128084000 Nmm (Aman)

Mpr = 1.25 . Mn

= 1.25 . 520138542.757

= 650173178.447 Nmm

Syarat kuat momen yang terpasag menurut SNI 2847-2013 pasal 21.5.2.2 :

$Mn^+ \geq ½\ Mn^-$

520138542.757 Nmm ≥ ½ . 811156335.311 Nmm

520138542.757 Nmm ≥ 405578167.655 Nmm

B. Perhitungan penulangan lapangan

Mu^- = 0.000 kNm

= 0 Nmm

Mu^+ = 215.548 kNm (1,2D + 1,6L)

= 215548000 Nmm

Dicoba pemasangan tulangan sebagai berikut :

- Tulangan yang terpasang pada daerah tarik 5 D 22 (As = 1899.70 mm^2)
- Tulangan yang terpasang pada daerah tekan 4 D 22 (As' = 1519.76 mm^2)
- Tulangan bagi plat terpasang di sepanjang beff 6 Ø 10 (As_{plat} = 471.00 mm^2)

Kontrol Momen Positif

Tulangan tekan As'_{plat} = 6 Ø 10 = 471.00 mm^2

As'_{balok} = 4 D 22 = 1519.76 mm^2

As' = 471.00 + 1519.76 = 1990.76 mm^2

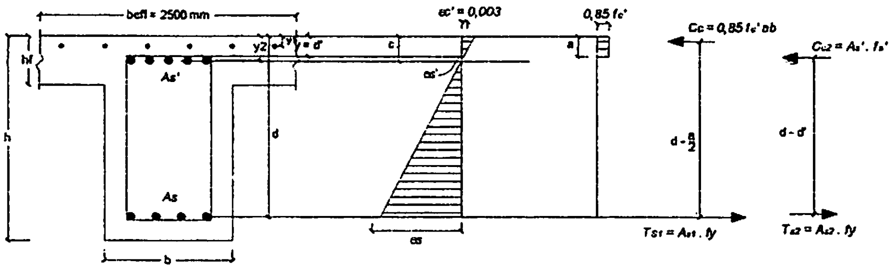
Tulangan tarik As = 5 D 22 = 1899.70 mm^2

y1 = 20 + 1/2 10 = 25 mm

y2 = 40 + 10 + 1/2 22 = 61 mm

$$y = d' = \frac{471 \times 25 + 1519.76 \times 61}{1990.76} = 52.483 \text{ mm}$$

d = 800 - 61.0 = 739.0 mm



Gambar 4.6 Penampang balok dan diagram tegangan momen positif lapangan

Dimisalkan garis netral > y2 maka perhitungan garis netral harus dicari menggunakan persamaan :

0,85 . f'c . a . b + As' . fs' = As . fy

Substitusi nilai : $fs' = \dfrac{(c - d')}{c} \times 600$

$$(0{,}85 . f'c . a . b) + As' \frac{(c - d')}{c} \times 600 = As . fy$$

$$(0{,}85 . f'c . a . b) . c + As' \quad (c - d') \times 600 = As . fy . c$$

Substitusi nilai : $a = \beta 1.c$

$$(0{,}85 . f'c . \beta 1.c . b) . c + As' \quad (c - d') 600 = As . fy . c$$

$$(0{,}85.f'c.\beta 1.b)\, c^2 + 600As'.c - 600As'.d' = As . fy . c$$

$$(0{,}85.f'c.\beta 1.b)\, c^2 + 600As'.c - 600As'.d' - As. fy . c = 0$$

$$(0{,}85.f'c.\beta 1.b)\, c^2 + (600As' - As . fy).c - 600As'.d' = 0$$

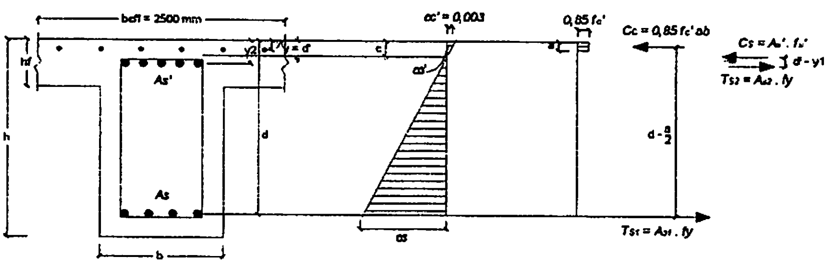
$(0,85.30.0,85.300)c^2 + (600.1321,16-1133,54.390).c -$

$600.1321,16.47,201 = 0$

$9753.75\ c^2 \quad + \quad 434576.0 \quad c \quad - \quad 62688216.00 \quad = \quad 0$

$c \quad = \quad 21.247 \quad mm$

Karena c < y2, tulangan tekan sebagian mengalami gaya tarik maka nilai c harus dihitung ulang.



Gambar 4.7 Penampang balok dan diagram tegangan momen positif lapangan yang sudah dihitung ulang

Dimisalkan garis netral diantara y1 dan y2 maka perhitungan garis netral dicari dengan menggunakan persamaan :

$0,85\ .\ f'c\ .\ a\ .\ beff + As_{plat}'\ .\ fs' = As1\ .\ fs + As2\ .\ fy_{ulir}$

$$\text{Substitusi nilai : } fs' \;=\; \frac{(c - y_1)}{c} \times 600 \quad \text{dan} \quad fs \;=\; fy_{ulir}$$

$$(0,85.f'c.a.beff) + As_{plat}' \; \frac{(c - y_1)}{c} \times 600 \;=\; As1\ .\ fy_{ulir} + As2\ .\ fy_{ulir}$$

$(0,85.f'c.a.beff).c + As_{plat}'.(c-y_1).600 = As1\ .\ fy_{ulir}.c + As2\ .\ fy_{ulir}\ .\ c$

Substitusi nilai : $a = \beta 1.c$

$(0,85.f'c.\beta 1.c.beff).c + As_{plat}'.(c-y_1).600 = As1\ .\ fy_{ulir}.c + As2\ .\ fy_{ulir}\ .\ c$

$(0,85.f'c.\beta 1.beff).c^2 + 600.As_{plat}'.c - 600.As_{plat}'.y_1 = As1\ .\ fy_{ulir}.c +$

$As2\ .\ fy_{ulir}\ .\ c$

$(0,85.f'c.\beta 1.beff).c^2 + (600.As_{plat}' - As1.fy_{ulir} - As2.fy_{ulir}).c -$

$600.As_{plat}'.y1 = 0$

$(0,85.30.0,85.1350).c^2 + (600.471 - 850,16.390 - 1133,54.390).c -$

$600.471.25 = 0$

$54187.50 \quad c^2 \quad - \quad 1085184.000 \quad - \quad 7065000 \quad = \quad 0$

$c = 25.200$ mm

$a = \beta . c$

$= 0.85 \times 25.200 = 21.420$ mm

$fs' = \varepsilon s' . Es$

$$= \frac{c - y_1}{c} . \varepsilon c . Es$$

$$= \frac{25.200 - 25}{25.200} \times 0.003 \times 200000 = 4.768 \text{ MPa}$$

$fs = fy_{ulir} = 400$ MPa

Menghitung gaya tekan dan tarik

$Cc = 0,85 . f'c . a . beff$

$= 0.85 \times 30 \times 21.420 \times 2500$

$= 1365538.393$ N

$Cs = As' \times f's$

$= 471.00 \times 4.768$

$= 2245.607$ N

$Ts_1 = As1 \times fy$

$= 1519.76 \times 400$

$= 607904.000$ N

$Ts_2 = As2 \times fy$

$= 1899.70 \times 400$

$= 759880.000$ N

$Cc + Cs = Ts_1 + Ts_2$

$1365538.393 + 2245.607 = 607904.000 + 759880.000$

$1367784.0 = 1367784.0$

$Z1 = d - (1/2 . a)$

$= 739.000 - (1/2 . 21.420)$

$= 728.290$ mm

Z2 = d' - y1

= 52.483 - 25

= 27.483 mm

Mn = (Cc . Z1) + (Cs . Z2)

= 607904.000 x 728.290 + 759880.000 x 27.483

= 463613856.265 Nmm

Mr = ϕ Mn

= 0.9 . 463613856.265

= 417252470.639 Nmm > Mu = 215548000 Nmm (Aman)

Mpr = 1.25 . Mn

= 1.25 . 463613856.265

= 579517320.332 Nmm

Syarat kuat momen yang terpasag menurut SNI 2847-2013 pasal 21.5.2.2 :

$Mn^{+} \geq ½ \; Mn^{-}$

463613856.265 Nmm ≥ ½ . 0.000 Nmm

463613856.265 Nmm ≥ 0.000 Nmm

C. Perhitungan penulangan tumpuan kanan

Mu^{-} = 411.230 kNm (1,2D + 1,6L)

= 411230000 Nmm

Mu^{+} = 187.81 kNm (0,9DL + 0,3EX + 1,0EY)

= 187809000 Nmm

Dicoba pemasangan tulangan sebagai berikut :

- Tulangan yang terpasang pada daerah tarik 6 D 22 (As = 2279.64 mm^2)
- Tulangan yang terpasang pada daerah tekan 4 D 22 (As' = 1519.76 mm^2)
- Tulangan bagi plat terpasang di sepanjang beff 6 Ø 10 (As_{plat} = 471 mm^2)

Kontrol Momen Negatif

Tulangan tarik As_{plat} = 6 Ø 10 = 471.00 mm^2

As_{balok1} = 4 D 22 = 1519.76 mm^2

As_{balok2} = 2 D 22 = 759.88 mm^2

As = 471.00 + 1519.76 + 759.88 = 2750.6 mn

Tulangan tekan As' = 4 D 22 = 1519.76 mm^2

y1 = 20 + 1/2 10 = 25 mm

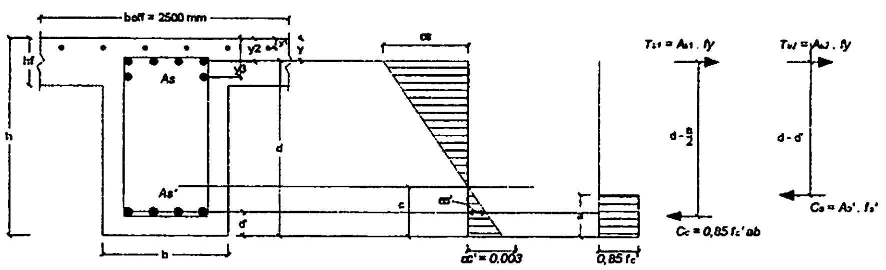
y2 = 40 + 10 + 1/2 22 = 61 mm

y3 = 40 + 10 + 22 + 30 + 1/2 22 = 113 mm

$$y = \frac{471 \times 25 + 2279.64 \times 61 + 759.88 \times 113}{2750.64} = 54.836$$

d = 800 - 54.836 = 745.164 mm

d' = 40 + 10 + 1/2 22 = 61.0 mm



Gambar 4.8 Penampang balok dan diagram tegangan momen negatif tumpuan kanan

Dimisalkan garis netral > d' maka perhitungan garis netral harus dicari menggunakan persamaan :

0,85 . f'c . a . b + As' . fs' = As . fy

Substitusi nilai : $fs' = \frac{(c - d')}{c} \times 600$

$(0,85 . f'c . a . b) + As' \frac{(c - d')}{c} \times 600 =$; $+ As_{balok} . fy_{ulir}$

$(0,85.f'c'.a.b).c + As'(c\text{-}d').600 = As_{plat}.fy_{polos}.c + As_{balok}.fy_{ulir}.c$

Substitusi nilai : $a = \beta 1.c$

$(0,85.f'c'.\beta 1.c .b).c + As'(c\text{-}d').600 = As_{plat}.fy_{polos}.c + As_{balok}.fy_{ulir}.c$

$(0,85.f'c'.\beta 1.b).c^2 + 600As'.c - 600As'.d' = As_{plat}.fy_{polos}.c + As_{balok}.fy_{ulir}.c$

$(0,85.f'c.\beta 1.b)c^2 + 600As'.c - 600As'.d' - As_{plat}.fy_{polos}.c - As_{balok}.fy_{ulir}.c = 0$

$(0,85.f'c.\beta 1.b)c^2 + (600As' - As_{plat}.fy_{polos} - As_{balok}.fy_{ulir}).c - 600As'.d' = 0$

$(0,85.30.0,85.300)c^2$+(600.850,16`-471.240-1133,54.390).c-

600.850,16.59,5 = 0

$9753.75\ c^2 \quad - \quad -190912\ c \quad - \quad 55623216 = 0$

c = 85.935 mm

a = β. c

= 0.85 x 85.935 = 73.044 mm

$$\varepsilon s' = \frac{c - d'}{c} \times \varepsilon c = \frac{85.935 - 61.0}{85.935} \times 0.003 = 0.00087$$

$$\varepsilon s = \frac{d - c}{c} \times \varepsilon c = \frac{745.164 - 85.935}{85.935} \times 0.003 = 0.02301$$

$$\varepsilon y = \frac{fy}{Es} = \frac{400}{200000} = 0.0020$$

Karena $\varepsilon s > \varepsilon y > \varepsilon s'$ maka tulangan baja tarik telah leleh,baja tekan belum

Dihitung tegangan pada tulangan baja tekan

f's = εs' x Es

= 0.00087 x 200000

= 174.095 < 400 MPa

Menghitung gaya tekan dan tarik

Cc = 0,85 . f'c . a . b

= 0.85 x 30 x 73.044 x 450

= 838185.285 N

Cs = As' x f's

= 1519.76 x 174.095

= 264582.715 N

Ts_1 = As_{plat} x fy_{polos}

= 471 x 240

= 113040 N

Ts_2 = As_{balok} x fy_{ulir}

= 2279.64 x 400

= 911856.0 N

Cc + Cs = Ts_1 + Ts_2

838185.285 + 264582.715 = 113040 + 911856.0

1102768.0 = 1024896.0

Z1 = d - (1/2 . a)

= 745.164 - (1/2 . 73.044)

= 708.642 mm

Z2 = d - d'

= 745.164 - 61.0

= 684.164 mm

Mn = (Cc . Z1) + (Cs . Z2)

= 838185.285 x 708.642 + 264582.715 x 684.164

= 774991491.664 Nmm

Mr = ϕ Mn

= 0.9 . 774991491.664

= 697492342.497 Nmm > Mu = 411230000 Nmm (Aman)

Mpr = 1.25 . Mn

= 1.25 . 774991491.664

= 968739364.580 Nmm

Kontrol Momen Positif

Tulangan tekan As'_{plat} = 6 Ø 10 = 471.00 mm^2

As'_{balok1} = 4 D 22 = 1519.76 mm^2

As'_{balok2} = 2 D 22 = 759.88 mm^2

As' = 471.00 + 1519.76 + 759.88 = 2750.6 mn

Tulangan tarik As = 4 D 22 = 1519.76 mm^2

y1 = 20 + 1/2 10 = 25 mm

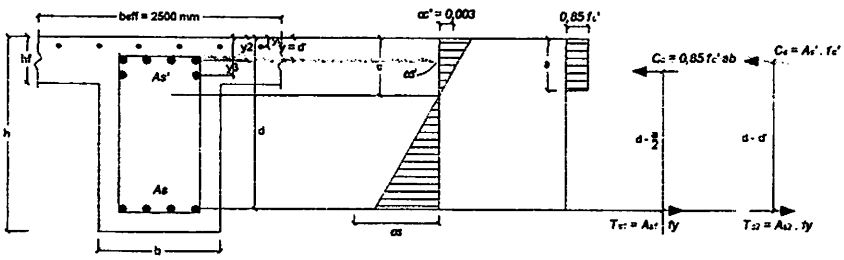
y2 = 40 + 10 + 1/2 22 = 61 mm

$$y3 = 40 + 10 + 22 + 30 + 1/2 \quad 22 = 113 \text{ mm}$$

$$y = d' = \frac{471 \times 25 + 1519.76 \times 61 + 759.88 \times 113}{2750.64}$$

$$= 37.984 \text{ mm}$$

$$d = 800 - 61.0 = 739.0 \text{ mm}$$



Gambar 4.9 Penampang balok dan diagram tegangan momen positif tumpuan kanan

Dimisalkan garis netral > y2 maka perhitungan garis netral harus dicari menggunakan persamaan :

$0{,}85 \cdot f'c \cdot a \cdot b + As' \cdot fs' = As \cdot fy$

Substitusi nilai : $fs' = \dfrac{(c - d')}{c} \times 600$

$$(0{,}85 \cdot f'c \cdot a \cdot b) + As' \; \frac{(c - d')}{c} \times 600 = As \cdot fy$$

$$(0{,}85 \cdot f'c \cdot a \cdot b) \cdot c + As' \quad (c - d') \times 600 = As \cdot fy \cdot c$$

Substitusi nilai : $a = \beta 1.c$

$$(0{,}85 \cdot f'c \cdot \beta 1.c \cdot b) \cdot c + As' \quad (c - d')\, 600 = As \cdot fy \cdot c$$

$$(0{,}85.f'c.\beta 1.b)\, c^2 + 600As'.c - 600As'.d' = As \cdot fy \cdot c$$

$$(0{,}85.f'c.\beta 1.b)\, c^2 + 600As'.c - 600As'.d' - As. fy \cdot c = 0$$

$$(0{,}85.f'c.\beta 1.b)\, c^2 + (600As' - As \cdot fy).c - 600As'.d' = 0$$

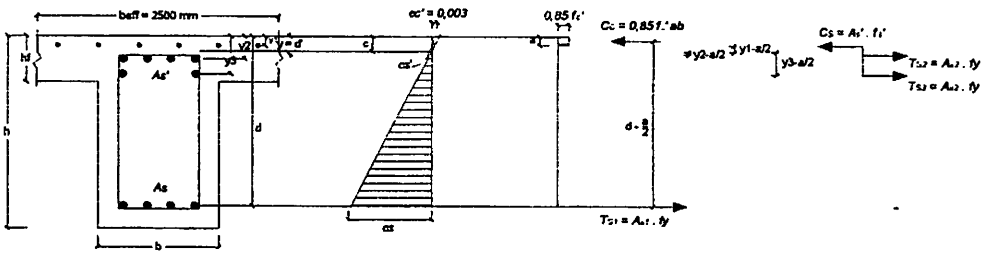
$(0{,}85.30.0{,}85.300)c^2 + (600.1604{,}54\text{-}850{,}16.390).c -$

$600.1604{,}54.49{,}373 = 0$

$$9753.75\, c^2 + 1042480.0\; c - 62688216 = 0$$

$$c = 38.780 \text{ mm}$$

Karena c < y2, tulangan tekan sebagian mengalami gaya tarik maka nilai c harus dihitung ulang.



Gambar 4.10 Penampang balok dan diagram tegangan momen positif tumpuan kanan yang sudah dihitung ulang

Dimisalkan garis netral diantara y1 dan y2 maka perhitungan garis netral dicari dengan menggunakan persamaan :

$0{,}85 \cdot f'c \cdot a \cdot beff + As_{plat}' \cdot fs' = As1 \cdot fs + As2 \cdot fy_{ulir}$

Substitusi nilai : $fs' = \dfrac{(c - y_1)}{c} \times 600$ dan $fs = fy_{ulir}$

$(0{,}85.f'c.a.beff) + As_{plat}' \dfrac{(c - y_1)}{c} \times 600 = As1 \cdot fy_{ulir} + As2 \cdot fy_{ulir}$

$(0{,}85.f'c.a.beff).c + As_{plat}'.(c\text{-}y_1).600 = As1 \cdot fy_{ulir}.c + As2 \cdot fy_{ulir} \cdot c$

Substitusi nilai : $a = \beta 1.c$

$(0{,}85.f'c.\beta 1.c.beff).c + As_{plat}'.(c\text{-}y_1).600 = As1 \cdot fy_{ulir}.c + As2 \cdot fy_{ulir} + As3 \cdot fy_{ulir} \cdot c$

$(0{,}85.f'c.\beta 1.beff).c^2 + 600.As_{plat}'.c - 600.As_{plat}'.y_1 = As1 \cdot fy_{ulir}.c + As2 \cdot fy_{ulir}.c +$ $As3 \cdot fy_{ulir} \cdot c$

$(0{,}85.f'c.\beta 1.beff).c^2 + (600.As_{plat}' - As1.fy_{ulir} - As2.fy_{ulir} - As3.fy_{ulir}).c -$ $600.As_{plat}'.y1 = 0$

$(0{,}85.30.0{,}85.1350).c^2 + (600.471 - 1133{,}54.390 - 850{,}16.390\text{-}15919.390).c -$ $600.471.25 = 0$

$54187.50 \quad c^2 \; - \; 1237160.000 \; - \; 7065000 \; = \; 0$

$c = 27.562$ mm

$a = \beta.c$

$= 0.85 \times 27.562 = 23.427$ mm

$f's = \varepsilon s' \times Es$

$= \frac{c - y_1}{c} . \varepsilon c . Es$

$= \frac{27.562 - 25}{27.562} \times 0.003 \times 200000 = 55.765$ MPa

$fs = fy_{ulir} = 400$ MPa

Menghitung gaya tekan dan tarik

Cc = 0,85 . f'c . a . beff

= 0.85 x 30 x 23.427 x 2500

= 1493494.798 N

Cs = As' x f's

= 471.00 x 55.765

= 26265.202 N

Ts_1 = As1 x fy

= 1519.76 x 400

= 607904.000 N

Ts_2 = As2 x fy

= 1519.76 x 400

= 607904.000 N

Ts_3 = As1 x fy

= 759.880 x 400

= 303952.000 N

Cc + Cs = $Ts_1 + Ts_2 + Ts_3$

1493494.798 + 26265.202 = 607904.00 + 607904.00 + 303952.(

1519760.00 = 1519760.00

Z1 = d - (1/2 . a)

= 739.000 - (1/2 . 23.427)

= 727.286 mm

$Z2 = y3 - (1/2 . a)$

$= 113.000 - (1/2 . 23.427)$

$= 101.286$ mm

$Z3 = y2 - (1/2 . a)$

$= 61.000 - (1/2. 23.427)$

$= 49.286$ mm

$Z4 = y3 - (1/2 . a)$

$= 25.000 - (1/2. 23.427)$

$= 13.286$ mm

$Mn = (Ts1 . Z1) + (Ts2 . Z2) - (Cs . Z3)$

$= 607904.00 \times 727.3 + 607904.00 \times 101.3 - 1493494.8 \times 49$

$= 430083760.913$ Nmm

$Mr = \phi \ Mn$

$= 0.9 . 430083760.913$

$= 387075384.821$ Nmm $> Mu = 187809000.0$ Nmm (Aman)

$Mpr = 1.25 . Mn$

$= 1.25 . 430083760.913$

$= 537604701.141$ Nmm

Syarat kuat momen yang terpasag menurut SNI 2847-2013 pasal 21.5.2.2 :

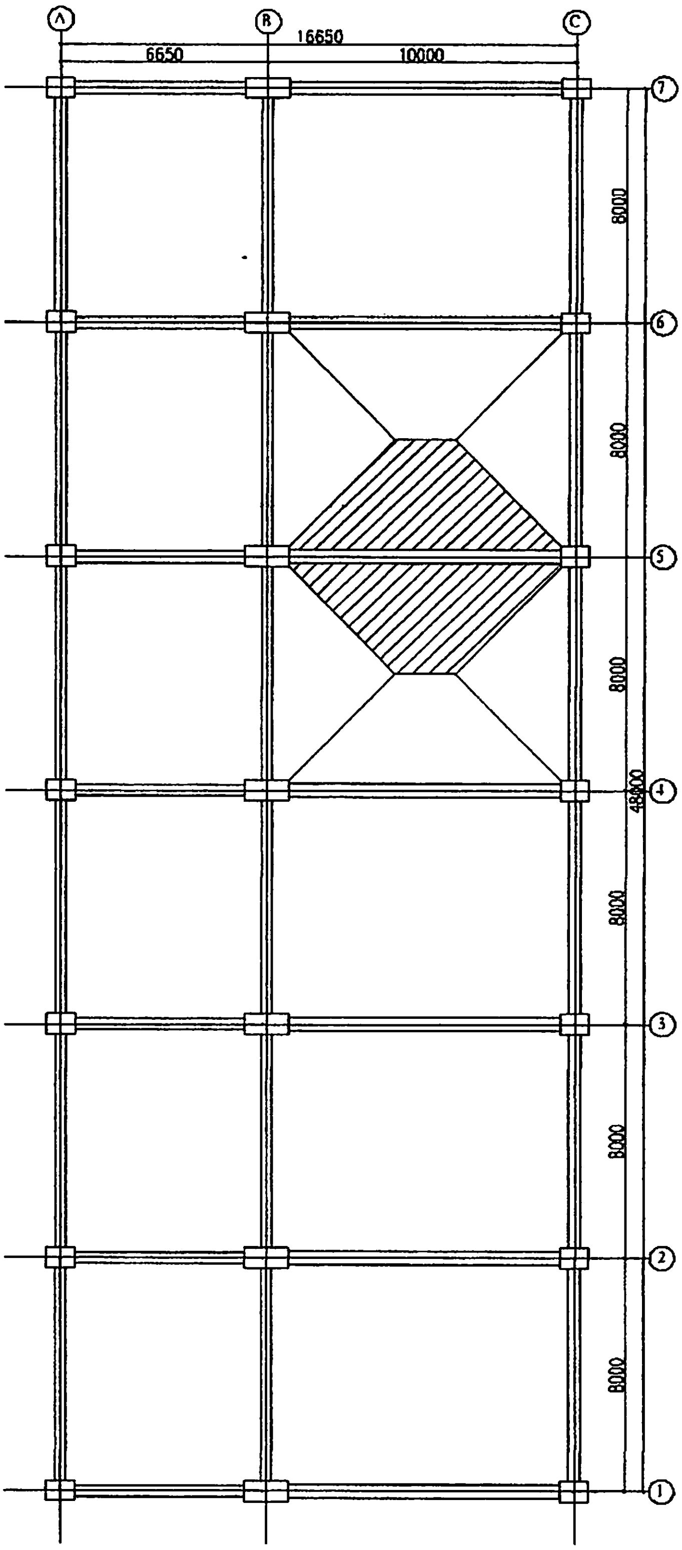
$Mn^{+} \geq \frac{1}{2} Mn^{-}$

430083760.913 Nmm $\geq \frac{1}{2} . 774991491.664$ Nmm

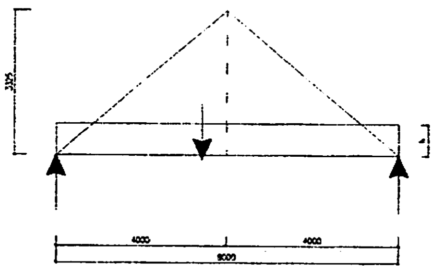
430083760.913 Nmm ≥ 387495745.832 Nmm

4.1.2 Desain Tulangan Geser Balok

4.1.2.1 Perataan Beban Plat Lantai

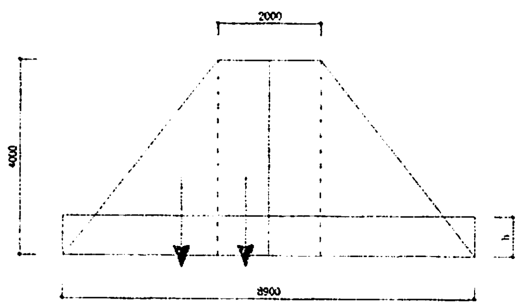


- **Perataan Plat Tipe A**



H_1	=	½	x	4	x	4	=	8.000	m^2	
R_A	=	R_B	=	H_1			=	8.000	m^2	
$M_{max\,1}$	=	R_A	x	4	-	H_1	(⅓ . 2,7)			
	=	8.000	x	4	-	8.000	x	1.3		
	=	21.33333 m^2								
$M_{max\,2}$	=	⅛	x h_A	x	ln^2					
	=	⅛	x h_A	x	$8{,}0^2$					
	=	8	h_A							
$M_{max\,1}$	=	$M_{max\,2}$								
21.33333	=	8	h_A							
h_A	=	21.33333	/	8						
	=	2.67 m								

- **Perataan Plat Tipe B**



H_1	=	½	x	4	x	4	=	8.000	m^2	
H_2	=	1	x	4			=	4.000	m^2	
R_A	=	R_B	=	H_1	+	H_2	=	8.000	+	4.000
							=	12.000	m^2	

$M_{max\,1}$ = R_A x 4.0 - H_1 (⅓.4+1,0) - H_2 (½.1,0)

= 12.000 x 4.0 - 18.667 - 2.000

= 27.333 m^2

$M_{max\,2}$ = ⅛ x h_B x ln^2

= ⅛ x h_B x $5,4^2$

= 8 h_B

$M_{max\,1}$ = $M_{max\,2}$

27.333 = 8 h_B

h_B = 27.333 / 8

= 3.417 m

- **Pembebanan Balok Induk Portal Line 5**

Beban Mati Merata (q_d)

Untuk Bentang (L) = 10,000 m

Lantai 2-16

Beban Mati (q_d)

- Berat sendiri balok	= b . (h - hf) . BJ beton	
	= 0.45 . (0,8 - 0,12) . 24	= 7.344 kN/m
- Perataan beban plat	= q_d plat x perataan plat	
	= 1.64 x 3.42 + 3.42	= 11.2067 kN/m
- Berat dinding	= tinggi tembok x berat dinding	
	= 3.5 x 2.5	= 8.8 kN/m
	q_d	= 27.301 kN/m

Beban Hidup Merata (q_L)

Lantai 2 sampai 16, fungi bangunan sebagai hunian, dimana :

$q_{L2\text{-}16}$ = 2.5 kN/m , ϕ = 0.9 dan perataan beban plat = 6.83

jadi q_L = 2.5 x 1 x 6.8 = 15.375

4.1.2.2 Penulangan Geser Balok

Diketahui :

b = 450 mm

h = 800 mm

d = 739.00 mm

L = 10000 mm

Ln = 10000 - (½ . 900 + ½ . 1400)

= 8850 mm

f'c = 30 MPa

fy_{ulir} = 400 MPa

fy_{polos} = 240 MPa

Mpr^- = 1013945419.139 Nmm

= 1013.95 kNm

Mpr^+ = 537604701.141 Nmm

= 537.60 kNm

Momen Negatif :

$$a = \frac{(As\ balok \times 1{,}25\ fy_{ulir}) + (Asplat \times 1{,}25 \times fy_{polos})}{0{,}85 \times f'c \times b}$$

$$a = \frac{3039.52 \times 1.25 \times 400 + 471 \times 1.25 \times 240}{0.85 \times 30 \times 450}$$

a = 144.755 mm

Mpr1 = *(As balok + Asplat) x (1,25 x fy) x (d - a/2)*

= (1133,54 x 1,25 x 390) + (471 x 1,25 x 240) x (441 - (90,706/2))

= 1107300232.218 Nmm

= 1107.30 kNm

Momen Positif

$$a = \frac{(As'\ balok) \times (1{,}25 \times fy)}{0{,}85 \times f'c \times b}$$

$$a = \frac{1519.76 \times 1.25 \times 400}{}$$

$$\overline{0.85} \times \overline{30} \times \overline{450}$$

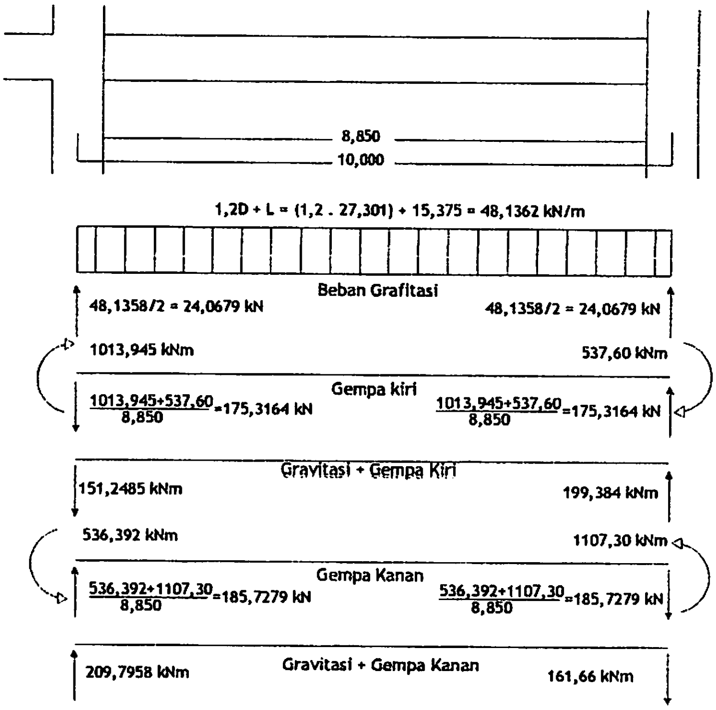
a = 66.22048 mm

Mpr2 = *(As' balok) x (1,25 x fy) x (d - a/2)*

= (850,16 x 1,25 x 390) x (441 - (54,177/2))

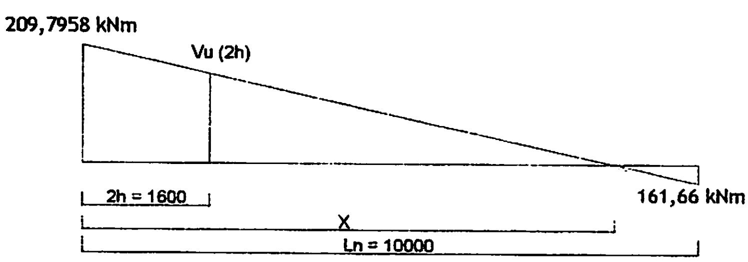
= 536391511.094 Nmm

= 536.392 kNm



Gambar 4.11 Desain gaya geser balok

Perhitungan Vu akibat beban gravitasi + gempa :



$$\frac{209.796}{x} = \frac{-161.660}{8.85 - x}$$

$161.66\ x = 1856.693 - 209.796\ x$

$$x = \frac{1856.693}{48.136} = 38.572 \text{ m} = 38572 \text{ mm}$$

- **Tulangan geser pada daerah sendi plastis**

$$Vu\,(d) = 209.796\ \frac{38572 - 739.0}{38572} = 205.776 \text{ kN}$$

$Vc = 0.17\ \lambda\ \sqrt{fc'}\ bw\ .\ d$

$= 0.17 \times 1.0\ \sqrt{30}\ 450 \times 739^{-3}$

$= 0.00000104$ kN

Pada daerah sendi plastis, $Vc = 0.00000104$

$$Vs = \frac{Vu\,(d)}{\phi} - Vc = \frac{205.776}{0.75} - 0.00000104 = 274.4 \text{ kN}$$

Direncanakan tulangan sengkang ϕ 10 (3 kaki)

$$S = \frac{Av\,.\,fy\,.d}{Vs}$$

$$= \frac{(3.1/4.\pi.10^2) \times 240 \times 739 \times 10^{-3}}{274.4} = 101.49 \text{ mm}$$

Persyaratan spasi maksimum pada daerah sendi plastis SNI 2847-2013 pasal 21.5.3.2, S_{maks} sepanjang sendi plastis diujung balok 2h = 2 . 800 = 1600 mm, spasi maksimum tidak boleh melebihi :

- $\frac{d}{4} = \frac{739.0}{4} = 184.75$
- 6 x diameter tulangan utama = 6 . 22 = 132 mm
- 150 mm

Jadi dipakai sengkang Ø 10 - 100 mm

$$Vs\ terpasang = \frac{Av\,.\,fy\,.d}{S}$$

$$= \frac{(3.¼.\pi.10^2) \times 240 \times 739.0 \times 10^{-3}}{100} = 278.46 \text{ kN}$$

$Vn = Vc + Vs\ terpasang$

$= 0.0000 + 278.455$

$= 278.455$ kN

$\phi Vn = 0.75 \ . \ Vn$

$= 0.75 \ . \ 278.455$

$= 208.841$ kN $> Vu(d) = 205.776$ kN .... (Aman)

Kontrol kuat geser nominal menurut SNI 2847-2013 pasal 11.4.5.3

$Vs\ maks \leq 0.66 \sqrt{fc'}\ bw \, . \, d$

$Vs\ maks \leq 0.66 \sqrt{30} \times 450 \times 739.0 \times 10^{-3}$

278.455 kN < 1202.16 kN .... OK

- **Tulangan geser pada daerah luar sendi plastis**

$$Vu\ (2h) = 209.796 \ \frac{38572 - 1600}{38572} = 201.093 \text{ kN}$$

$Vc = 0.17\ \lambda \sqrt{fc'}\ bw \, . \, d$

$= 0.17 \times 1.0 \ \sqrt{30} \ 450 \times 739 \ ^{-3}$

$= 0.000001$ kN

$$Vs = \frac{Vu\ (2h)}{\phi} - Vc = \frac{201.093}{0.75} - 0.000001 = 268.12 \text{ kN}$$

Direncanakan tulangan sengkang ϕ 10 (3 kaki)

$$S = \frac{Av \, . \, fy \, . \, d}{Vs}$$

$$= \frac{(3.\tfrac{1}{4}.\pi.10^2) \times 240 \times 739.0 \times 10^{-3}}{268.12} = 155.8 \text{ mm}$$

Syarat jarak spasi sengkang maksimum pada daerah luar sendi plastis menurut SNI 2847-2013 pasal 21.5.3.4 dan 21.5.3.3 :

- $\frac{d}{2} = \frac{739.0}{2} = 369.500$ mm
- 350 mm

Jadi dipakai sengkang Ø 10 - 150 mm

$$Vs\ terpasang = \frac{Av \cdot fy \cdot d}{S}$$

$$\frac{(3.\frac{1}{4}.\pi.10^2) \times 240 \times 739.0 \times 10^{-3}}{150} = 278.455 \text{ kN}$$

$$Vn = Vc + Vs\ terpasang$$
$$= 0.000 + 278.455$$
$$= 278.455 \text{ kN}$$
$$\phi Vn = 0.75 \cdot Vn$$
$$= 0.75 \cdot 278.455$$
$$= 208.841 \text{ kN} > Vu(2h) = 201.093 \text{ kN}$$ .... (Aman)

Kontrol kuat geser nominal menurut SNI 2847-2013 pasal 11.4.5.3

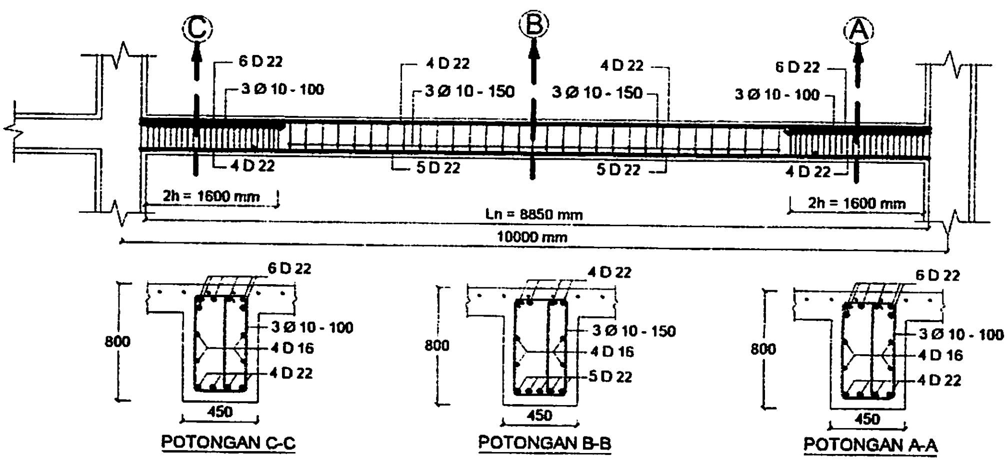
$$Vs\ maks \leq 0.66\sqrt{fc'}\ bw \cdot d$$

$$Vs\ maks \leq 0.66\sqrt{30} \times 450 \times 739.0 \times 10^{-3}$$

278.455 kN < 1202.16 kN .... OK

Dari hasil perhitungan dan ketentuan-ketentuan di atas maka dipasang tulangan sengkang sebagai berikut :

- Joint 21807
  - Daerah sendi plastis = 3 kaki Ø 10 - 100
  - Daerah luar sendi plastis = 3 kaki Ø 10 - 150



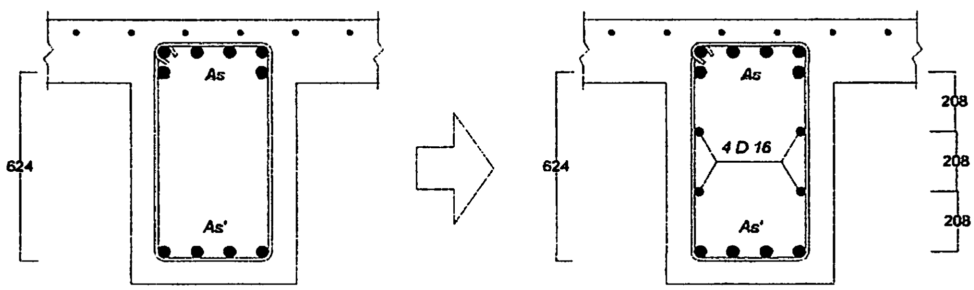
Gambar 4.12 Penulangan geser pada balok

- Penulangan Torsi

Menurut dasar perencanaan beton bertulang SNI 2847-2013 Pasal 11.5.6, jarak maksimum tulangan torsi harus memenuhi syarat - syarat sebagai berikut :

- spasi maksimum antara tulangan longitudinal untuk torsi di sekeliling parimeter sengka adalah = 300 mm
- batang tulangan longitudinal harus berada dalam sengkang dengan setiap sudunya harus ada minimal satu batang tulangan longitudinal.
- batang tulangan longitudinal harus mempunyai diameter paling sedikit 0,042 x spasi sengkang, tetapi tidak kurang dari 10 mm.

pada analisa penulangan lentur balok diketahui bentang bersih antara tulangan tarik dan tulangan tekan = 624 mm. Untuk memenuhi persyaratan pada pasal 11.5.6, maka diperlukan 2 tulangan torsi longitudinal sebagai tulangan torsi. Sehingga jarak maksimum antara tulangan tekan dan tulangan tarik = 208 mm. sedangkan untuk diameter tulangan, dengan spasi terbesar yang direncanakan = 312 mm, maka 0,042 x 312 mm = 13,104 digunakan D 16.



4.2 Perhitungan Penulangan Kolom

4.2.1 Perhitungan Penulangan Lentur Kolom

Penulangan kolom yang dihitung adalah pada kolom yang berada pada struktur portal memanjang line 5, kolom no 1146

Diketahui :

b = 700 mm

h = 900 mm

Tulangan sengkang Ø 10

Tulangan utama dipakai D 22

Tebal selimut beton 40 mm

Tinggi kolom = h kolom - h balok

= 4000 - 800 = 3200 mm

f'c = 30 MPa

fy = 400 MPa

Dicoba tulangan D 22 mm

d = h - selimut beton - Ø sengkang - ½ Ø tulangan pokok

= 900 - 40 - 10 - ½ 22

= 839.0 mm

d' = 900 - 839 = 61 mm

- Luas Penampang kolom (Ag)

Ag = b . h

= 700 . 900

= 630000 mm^2

Jumlah tulangan pada kolom 1% - 6% dicoba dengan jumlah tulangan 1.15 % ,ρ = 0.0115

As_{perlu} = ρ . Ag

= 0.0115 . 630000

= 7245 mm^2

Maka dipakai tulangan 20 D 22 ,As ada = 7598.80 mm^2 > As|

- Beban Sentris

Po = 0,85 . f'c (Ag - Ast) + fy . Ast

= (0,85 . 30 (480000 - 5667,70) + 390 . 5667,70) . 10^{-3}

= 18910.751 kN

Pn = 0,80 . Po

= 0.80 . 18910.751

= 15128.600 kN

ϕ Pn = 0.65 . 15128.600

= 9833.590 kN

- Kondisi Seimbang

$$cb = \frac{600.\,d}{600 + fy} = \frac{600 \times 839.0}{600 + 400} = 503.400 \text{ mm}$$

ab = cb . β

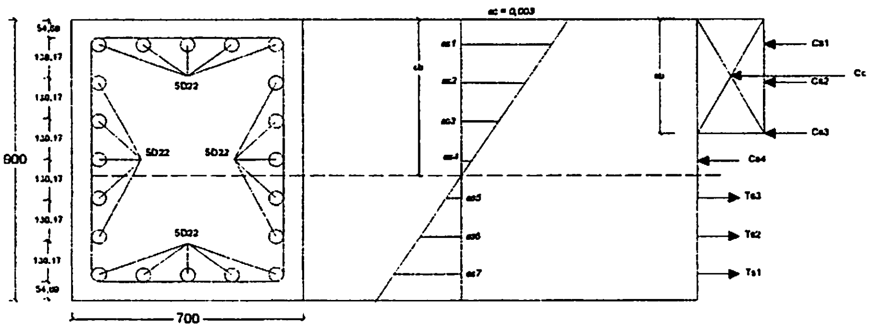
= 503.400 . 0.85

= 427.890 mm

Cc = 0,85 . f'c . ab . b

= 0.85 . 30 . 427.890 . 700 . 10^{-3}

= 7637.8365 kN



Gambar 4.13 Diagram tegangan dan regangan kolom kondisi seimbang

$$\varepsilon y = \frac{fy}{Es} = \frac{400}{200000} = 0.00200$$

$$\varepsilon s_1 = \frac{503.400 - 61.0}{503.400} \times 0.003$$

$= 0.00264 > \varepsilon y$; maka fs = fy = 400 MPa

$Cs_1 = 1416.925 \;.\; 400 \;.\; 10^{-3} = 566.770$ kN

$$\varepsilon s_2 = \frac{503.400 - 191.17}{503.400} \times 0.003$$

$= 0.00186 < \varepsilon y$;

maka fs = 0.00186 . 200000 = 372.149 MPa

$Cs_2 = 566.770 \;.\; 372.149 \;.\; 10^{-3} = 210.923$ kN

$$\varepsilon s_3 = \frac{503.400 - 321.33}{503.400} \times 0.003$$

$= 0.00109 < \varepsilon y$;

maka fs = 0.00109 . 200000 = 217.004 MPa

$Cs_3 = 566.770 \;.\; 217.004 \;.\; 10^{-3} = 122.992$ kN

$$\varepsilon s_4 = \frac{503.400 - 452}{503.400} \times 0.003$$

$= 0.0003 < \varepsilon y$;

maka fs = 0.00031 . 200000 = 61.859 MPa

$Cs_4 = 566.770 \;.\; 61.859 \;.\; 10^{-3} = 35.060$ kN

$$\varepsilon s_5 = \frac{581.67 - 503.400}{503.400} \times 0.003$$

$= 0.00047 < \varepsilon y$;

maka fs = 0.00047 . 200000 = 93.286 MPa

$Ts_3 = 566.770 \;.\; 93.286 \;.\; 10^{-3} = 52.872$ kN

$$\varepsilon s_6 = \frac{712 - 503.400}{503.400} \times 0.003$$

$= 0.00124 < \varepsilon y$;

maka fs = 0.00124 . 200000 = 248.431 MPa

$Ts_2 = 566.770 \;.\; 248 \;.\; 10^{-3} = 140.803$ kN

$$\varepsilon s_7 = \frac{842.00 - 503.400}{503.400} \times 0.003$$

$= 0.00202 = \varepsilon y$; maka $fs = fy = 400$ MPa

$Ts_1 = 1416.925 \;.\; 400.000 \;.\; 10^{-3} = 566.770$ kN

$Pnb = Cc + Cs_1 + Cs_2 + Cs_3 + Cs_4 - Ts_1 - Ts_2$
Ts_3

$= 7637.8365 + 566.770 + 210.923 + 122.992 + 35.060 -$
$566.770 - 140.803 - 52.872$

$= 7813.137$ kN -

$\phi\ Pnb = 0.65 \;.\; 7813.137$

$= 5078.538814$ kN

$Mnb = Cc(h/2 - ab/2)+\{(Cs_1+Ts_1).(h/2 - 59,5)\}+\{(Cs_2+Ts_2).$
$h/2 - 173\}+\{(Cs_3+Ts_3).(h/2 - 286,50)\}$

$= [5836,486.(800/2-381,470/2)+\{(552,601+552,601).$
$(800/2-59,5)\}+\{(208,974+135,037).(800/2-173)\}+\{(122,971+$
$49,035).(800/2-286,50)\}] \;.\; 10^{-3}$

$= 2288.72216$ kNm

$\phi\ Mnb = 0.65 \;.\; 2288.722$

$= 1487.669$ kNm

$$eb = \frac{Mnb}{Pnb} = \frac{2288.72216}{7813.137} = 0.2929 \text{ m} = 292.933 \text{ mm}$$

- Kondisi Seimbang dengan 1,25 fy

$fy = 1.25 \times 400 = 500.00$ MPa

$$cb = \frac{600.\ d}{600 + fy} = \frac{600 \times 839.0}{600 + 500.00} = 457.636 \text{ mm}$$

$ab = cb \;.\; \beta$

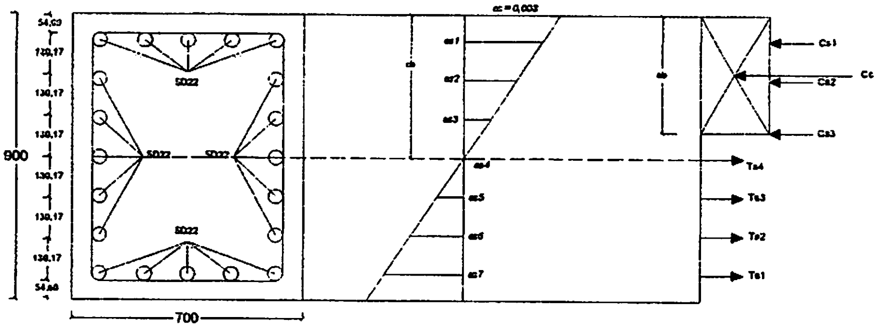
$= 457.636 \;.\; 0.85$

$= 388.991$ mm

Cc = 0,85 . f'c . ab . b

= 0.85 . 30 . 388.991 . 700 . 10^{-3}

= 6943.48773 kN



Gambar 4.14 Diagram tegangan dan regangan kolom kondisi seimbang *1,25 fy*

$$\varepsilon y = \frac{fy}{Es} = \frac{500.00}{200000} = 0.00250$$

$$\varepsilon s_1 = \frac{457.636 - 61}{457.636} \times 0.003$$

= 0.00260 > εy ; maka fs = fy = 500.00 MPa

Cs_1 = 1416.925 . 500.00 . 10^{-3} = 708.463 kN

$$\varepsilon s_2 = \frac{457.636 - 191.17}{457.636} \times 0.003$$

= 0.00175 < εy ;

maka fs = 0.00175 . 200000 = 349.364 MPa

Cs_2 = 566.770 . 349.364 . 10^{-3} = 198.009 kN

$$\varepsilon s_3 = \frac{457.636 - 321.33}{457.636} \times 0.003$$

= 0.00089 < εy ;

maka fs = 0.00089 . 200000 = 178.705 MPa

Cs_3 = 566.770 . 178.705 . 10^{-3} = 101.285 kN

$$\varepsilon s_4 = \frac{457.636 - 452}{451.500} \times 0.003$$

= 0.00004 < εy ;

maka fs = 0.00004 . 200000 = 8.155 MPa

Cs_4 = 566.770 . 8.155 . 10^{-3} = 4.622 kN

$$\varepsilon s_5 = \frac{581.67 - 457.636}{457.636} \times 0.003$$

= 0.00081 < εy ;

maka fs = 0.00081 . 200000 = 162.614 MPa

Ts_3 = 566.770 . 162.614 . 10^{-3} = 92.165 kN

$$\varepsilon s_6 = \frac{711.83 - 457.636}{457.636} \times 0.003$$

= 0.00167 < εy ;

maka fs = 0.00167 . 200000 = 333.274 MPa

Ts_2 = 566.770 . 333.274 . 10^{-3} = 188.890 kN

$$\varepsilon s_7 = \frac{825.33 - 457.636}{457.636} \times 0.003$$

= 0.00241 = εy ; maka fs = fy = 500.00 MPa

Ts_1 = 1416.925 . 500.00 . 10^{-3} = 708.463 kN

Pnb = Cc + Cs_1 + Cs_2 + Cs_3 + Cs_4 - Ts_1 - Ts_2 - Ts_3

= 6943.48773 + 708.463 + 198.009 + 101.285 + 4.622 - 708.463 - 188.890 - 92.165

= 6966.349 kN

ϕ Pnb = 0.65 . 6966.349

= 4528.127 kN

Mnb = Cc(h/2 - ab/2)+{(Cs_1+Ts_1).(h/2 - 59,5)}+{(Cs_2+Ts_2). h/2 - 173}+{(Cs_3+Ts_3).(h/2 - 286,50)}

= [5213,215.(800/2-347,269/2)+{(690,751+690,751). (800/2-59,5)}+{(196,064+181,828).(800/2-173)}+{(101,591+ 87,355).(800/2-286,50)}] . 10^{-3}

= 2366.33817 kNm

ϕ Mnb = 0.65 . 2366.338

= 1538.120 kNm

$$eb = \frac{Mnb}{Pnb} = \frac{2366.33817}{6966.349} = 0.3397 \text{ m} = 339.681 \text{ mm}$$

- Kondisi Patah Desak (c > cb)

Dipakai nilai c = 650 mm > cb = 501 mm

a = c . β

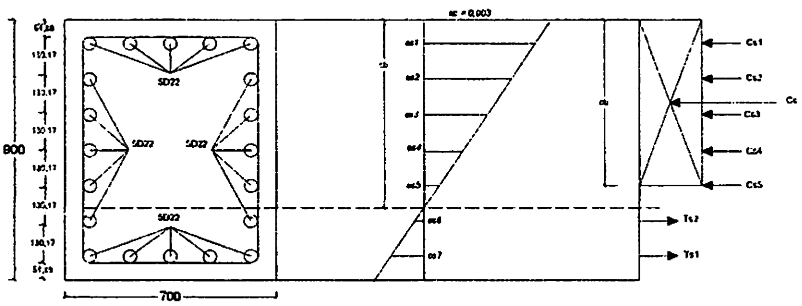
= 650.000 . 0.85

= 552.50 mm

Cc = 0,85 . f'c . a . b

= 0.85 . 30 . 552.50 . 700 . 10^{-3}

= 9862.125 kN



Gambar 4.15 Diagram tegangan dan regangan kolom kondisi patah desak

$$\varepsilon y = \frac{fy}{Es} = \frac{400}{200000} = 0.00200$$

$$\varepsilon s_1 = \frac{650 - 61}{650.000} \times 0.003$$

= 0.00272 > εy ; maka fs = fy = 400 MPa

Cs_1 = 1416.925 . 400 . 10^{-3} = 566.770 kN

$$\varepsilon s_2 = \frac{650 - 191.17}{650} \times 0.003$$

= 0.00212 > εy ; maka fs = fy = 400 MPa

$Cs_2 = 566.770 \ . \ 400 \ . \ 10^{-3} = 226.708$ kN

$$\varepsilon s_3 = \frac{650 - 321.33}{650} \times 0.003$$

$= 0.00152 < \varepsilon y$;

maka $fs = 0.00152 \ . \ 200000 = 303.38$ MPa

$Cs_3 = 566.770 \ . \ 303.38 \ . \ 10^{-3} = 171.949$ kN

$$\varepsilon s_4 = \frac{650 - 452}{650} \times 0.003$$

$= 0.00092 < \varepsilon y$;

maka $fs = 0.00092 \ . \ 200000 = 183$ MPa

$Cs_4 = 566.770 \ . \ 183 \ . \ 10^{-3} = 103.850$ kN

$$\varepsilon s_5 = \frac{650 - 581.67}{650} \times 0.003$$

$= 0.00032 < \varepsilon y$;

maka $fs = 0.00032 \ . \ 200000 = 63.08$ MPa

$Cs_5 = 566.770 \ . \ 63.08 \ . \ 10^{-3} = 35.750$ kN

$$\varepsilon s_6 = \frac{711.83 - 650}{650} \times 0.003$$

$= 0.00029 < \varepsilon y$;

maka $fs = 0.00029 \ . \ 200000 = 57.077$ MPa

$Ts_2 = 566.770 \ . \ 57.077 \ . \ 10^{-3} = 32.349$ kN

$$\varepsilon s_7 = \frac{825.33 - 650}{650} \times 0.003$$

$= 0.00081 < \varepsilon y$;

maka $fs = 0.00081 \ . \ 200000 = 161.846$ MPa

$Ts_1 = 1416.925 \ . \ 161.846 \ . \ 10^{-3} = 229.324$ kN

$Pn = Cc + Cs_1 + Cs_2 + Cs_3 + Cs_4 + Cs_5 - Ts_1$
Ts_2

$= 9862.125 + 566.770 + 226.708 + 171.949 + 103.850 -$

$35.750 - 229.324 - 32.349$

$= 10633.979$ kN +

ϕ Pn = 0.65 . 10633.979

= 6912.086054 kN

Mn = $Cc(h/2 - ab/2)+\{(Cs_1+Ts_1).(h/2 - 59,5)\}+\{(Cs_2+Ts_2).$

$h/2 - 173)\} + \{(Cs3+Cs5).(h/2 - 286,50)\}$

= [7803.(800/2-510/2)+{(552,601+199,078).(800/2-59,5)}+

{(221,040+15,303).(800/2-173)}+{(177,682+49,026).

(h/2 - 286,50)}]. 10^{-3}

= 2066.994 kNm

ϕ Mn = 0.65 . 2066.994

= 1343.55 kNm

$$eb = \frac{Mn}{Pn} = \frac{2066.99411}{10633.979} = 0.1944 \text{ m} = 194.376 \text{ mm}$$

- Kondisi Patah Tarik (c < cb)

Dipakai nilai c = 250 mm < cb = 501 mm

a = c . β

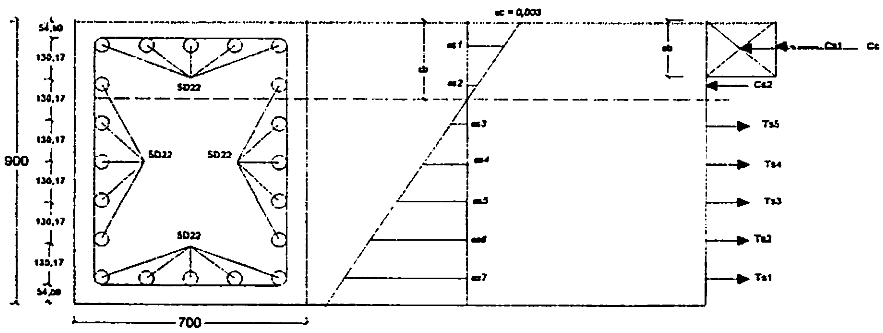
= 250 . 0.85

= 213 mm

Cc = 0,85 . f'c . a . b

= 0.85 . 30 . 213 . 700 . 10^{-3}

= 3793.125 kN



Gambar 4.16 Diagram tegangan dan regangan kolom kondisi patah tarik

$$\varepsilon y = \frac{fy}{Es} = \frac{400}{200000} = 0.00200$$

$$\varepsilon s_1 = \frac{250 - 61}{250} \times 0.003$$

$= 0.00227 > \varepsilon y$; maka fs = fy = 400 MPa

$Cs_1 = 1416.925 \;.\; 400 \;.\; 10^{-3} = 566.770$ kN

$$\varepsilon s_2 = \frac{250 - 191.17}{250} \times 0.003$$

$= 0.00071 < \varepsilon y$;

maka fs = 0.00071 . 200000 = 141.200 MPa

$Cs_2 = 566.770 \;.\; 141.200 \;.\; 10^{-3} = 80.028$ kN

$$\varepsilon s_3 = \frac{321 - 250}{250} \times 0.003$$

$= 0.00086 < \varepsilon y$;

maka fs = 0.00086 . 200000 = 171.200 MPa

$Ts_5 = 566.770 \;.\; 171.200 \;.\; 10^{-3} = 97.031$ kN

$$\varepsilon s_4 = \frac{452 - 250}{250.000} \times 0.003$$

$= 0.00242 > \varepsilon y$; maka fs = fy = 400 MPa

$Ts_4 = 566.770 \;.\; 400 \;.\; 10^{-3} = 226.708$ kN

$$\varepsilon s_5 = \frac{581.67 - 250}{250} \times 0.003$$

$= 0.00398 > \varepsilon y$; maka fs = fy = 400 MPa

$Ts_3 = 566.770 \;.\; 400 \;.\; 10^{-3} = 226.708$ kN

$$\varepsilon s_6 = \frac{711.83 - 250}{250} \times 0.003$$

$= 0.00554 > \varepsilon y$; maka fs = fy = 400 MPa

$Ts_2 = 566.770 \;.\; 400 \;.\; 10^{-3} = 226.708$ kN

$Cc = 0{,}85 \cdot fc' \cdot a \cdot b$

$= 0.85 \times 30 \times 92.512 \times 700$

$= 1651.331$ kN

$Cs_1 = fs' \cdot As'$

$$= \frac{(c - d')}{c} \times 600 \cdot As'$$

$$= \frac{108.837 - 61.0}{108.837} \times 600 \times 3799.400 \times 10^{-3}$$

$= 1001.97$ MPa

$Ts_1 = Asl \times fy$

$= 3799.400 \times 400 \times 10^{-3}$

$= 1519.76$ kN

$Ts_2 = Asl \times fy$

$= 566.770 \times 400 \times 10^{-3}$

$= 226.708$ kN

$Ts_3 = Asl \times fy$

$= 566.770 \times 400 \times 10^{-3}$

$= 226.708$ kN

$Ts_4 = Asl \times fy$

$= 566.770 \times 400 \times 10^{-3}$

$= 226.708$ kN

$Ts_5 = Asl \times fy$

$= 566.770 \times 400 \times 10^{-3}$

$= 226.708$ kN

$Ts_6 = Asl \times fy$

$= 566.770 \times 400 \times 10^{-3}$

$= 226.708$ kN

$$\varepsilon s_7 = \frac{825.33 - 250}{250} \times 0.003$$

$= 0.00690 > \varepsilon y$; maka $fs = fy = 400$ MPa

$Ts_1 = 1416.925 \;.\; 400 \;.\; 10^{-3} = 566.770$ kN

$Pn = Cc + Cs_1 + Cs_2 - Ts_1 - Ts_2 - Ts_3 - Ts_4 - Ts_5$

$= 3793.125 + 566.770 + 80.028 - 566.770 - 226.708 - 226.708 - 226.708 - 97.031$

$= 3095.998$ kN

$\phi\, Pn = 0.65 \;.\; 3095.998$

$= 2012.398635$ kN

$Mn = Cc(h/2 - ab/2)+\{(Cs_1+Ts_1).(h/2 - 59,5)\}+\{(Cs_2+Ts_2).h/2 - 173)\} + \{(Ts3+Ts5).(h/2 - 286,50)\}$

$= [2601.(800/2-510/2)+\{(552,601+552,601).(800/2-59,5)\}+\{(45,908+221,040).(800/2-173)\}+\{(221,040+147,077).(h/2 - 286,50)\}].\ 10^{-3}$

$= 1852.237$ kNm

$\phi\, Mn = 0.65 \;.\; 1852.237$

$= 1203.95$ kNm

$$eb = \frac{Mn}{Pn} = \frac{1852.23737}{3095.998} = 0.5983 \text{ m} = 598.268 \text{ mm}$$

- Kondisi Lentur Murni

Dicoba dipasang tulangan sebagai berikut :

Tulangan tarik As = 7 D 22 = 2659.580 mm^2

Tulangan tekan As' = 13 D 22 = 4939.220 mm^2

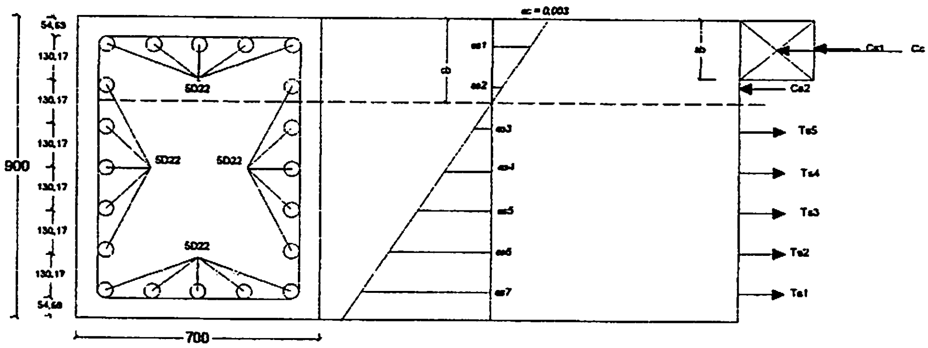
As' 1 = 5 D 22 = 1899.700 mm^2

As' 2 = 6 D 22 = 2279.640 mm^2

y1 = 40 + 10 + 1/2 22 = 61 mm

y2 = 61 + 113.5 = 175 mm

$$y = d' = \frac{1899.7 \times 61 + 2279.64 \times 175}{4939.220} = 104.000 \text{ mm}$$



Gambar 4.17 Diagram tegangan dan regangan kolom kondisi 1 lentur murni

Dimisalkan garis netral (c) > y2 maka perhitungan garis netral harus dicari menggunakan persamaan :

0,85 . f'c . a . b + As' . fs' = As . fy

Substitusi nilai : $fs' = \frac{(c - d')}{c} \times 600$

$$(0,85 . f'c . a . b) + As' \frac{(c - d')}{c} \times 600 = As . fy$$

$$(0,85 . f'c . a . b) . c + As' (c - d') \times 600 = As . fy . c$$

Substitusi nilai : $a = \beta 1.c$

$$(0,85 . f'c . \beta 1.c . b) . c + As' (c - d') 600 = As . fy . c$$

$$(0,85.f'c.\beta 1.b)\, c^2 + 600As'.c - 600As'.d' = As . fy . c$$

$$(0,85.f'c.\beta 1.b)\, c^2 + 600As'.c - 600As'.d' - As. fy . c = 0$$

$$(0,85.f'c.\beta 1.b)\, c^2 + (600As' - As . fy).c - 600As'.d' = 0$$

$(0,85.30.0,85.600)c^2 + (600.3684,005-1983,695.390).c -$

$600.3684,005.102,731 = 0$

$$15173\ c^2 + 1899700.000\ c - 308207328.0 = 0$$

c = 87.98138 mm

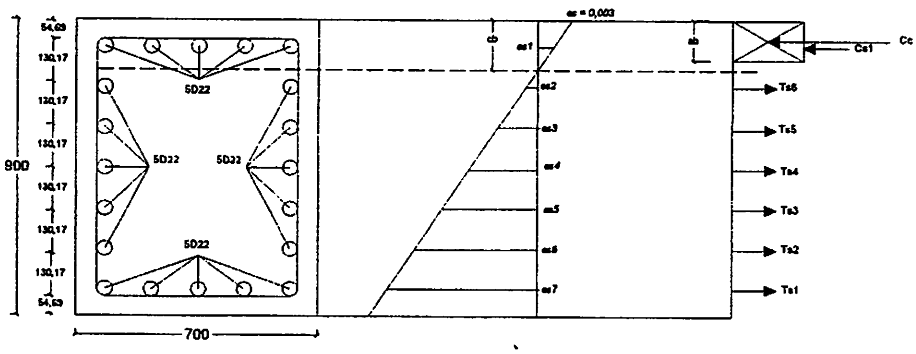
Karena nilai c < y2 maka dihitung nilai c sebenarnya berdasarkan persamaan yang kedua.

Dicoba dipasang tulangan sebagai berikut :

Tulangan tarik As	=	10	D	22	=	3799.400	mm^2
Tulangan tekan As'	=	10	D	22	=	3799.400	mm^2

d' = 40 + 10 + 1/2 22 = 61 mm

d = 900 - 61 = 839 mm



Gambar 4.18 Diagram tegangan dan regangan kolom kondisi 2 lentur murni

0,85 . f'c . a . b + As' . fs' = As . fy

Substitusi nilai : $fs' = \dfrac{(c-d')}{c} \times 600$

$(0,85 . f'c . a . b) + As' \dfrac{(c-d')}{c} \times 600 = As . fy$

$(0,85 . f'c . a . b) . c + As' (c - d') \times 600 = As . fy . c$

Substitusi nilai : $a = \beta 1.c$

$(0,85 . f'c . \beta 1.c . b) . c + As' (c - d') 600 = As . fy . c$

$(0,85.f'c.\beta 1.b) c^2 + 600As'.c - 600As'.d' = As . fy . c$

$(0,85.f'c.\beta 1.b) c^2 + 600As'.c - 600As'.d' - As. fy . c = 0$

$(0,85.f'c.\beta 1.b) c^2 + (600As' - As . fy).c - 600As'.d' = 0$

$(0,85.30.0,85.800)c^2 + (600.1416,925-4250,775.390).c -$

$600.1416,925.59,5 = 0$

$15173 \; c^2 - -759880.000 \; c - 139058040.000 = 0$

c = 108.837 mm

a = β. c

= 0.85 x 108.837 = 92.512 mm

$Cc + Cs1 = Ts1 + Ts2 + Ts3 + Ts4 + Ts5 + Ts6$

$1651.331 + 1001.97 = 1519.76 + 226.708 + 226.708 + 226.708$

$+ 226.708 + 226.708$

$2653.300 \text{ kN} = 2653.300 \text{ kN}$

$ZD_D = c - a/2$

$= 108.837 - \frac{92.512}{2}$

$= 62.58134 \text{ mm}$

$ZD_1 = c - y1$

$= 108.837 - 61$

$= 47.837 \text{ mm}$

$ZT_6 = y2 - c$

$= 175 - 108.837$

$= 65.663 \text{ mm}$

$ZT_5 = y3 - c$

$= 321.33 - 108.837$

$= 212.496 \text{ mm}$

$ZT_4 = y4 - c$

$= 452 - 108.837$

$= 342.663 \text{ mm}$

$ZT_3 = y5 - c$

$= 581.67 - 108.837$

$= 472.830 \text{ mm}$

$ZT_2 = y6 - c$

$= 712 - 108.837$

$= 602.996 \text{ mm}$

$ZT_1 = y7 - c$

$= 842.00 - 108.837$

$= 733.163 \text{ mm}$

$$Mn = (Cc.ZD_D) + (Cs_1.ZD_1) + (Ts_1.ZT_1) + (Ts_2.ZT_2) + (Ts_3.ZT_3) + (Ts_4.ZT_4) + (Ts_5.ZT_5) + (Ts_6.ZT_6)$$

$$= \{(1309,871.57,914) + (347,932.41,221) + (552,601.639,779) + (221,040.526,279) + (221,040.412,779) + (221,040.299,279) + (221,040.185,779) + (221,040.72,279)$$

$$= 1650.15 \text{ kNm}$$

$$\phi\ Mn = 0.65 \ . \ 1650.15$$

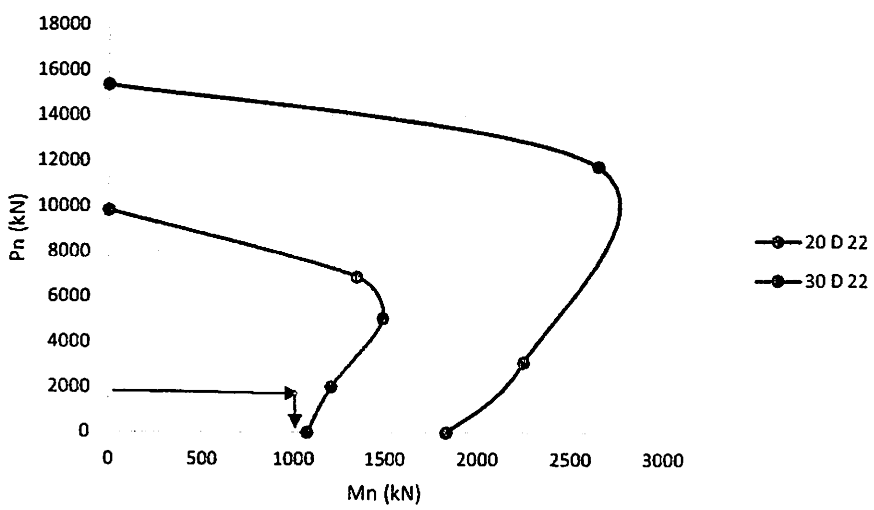
$$= 1072.60 \text{ kNm}$$

KOLOM

Kondisi	20 D 22		30 D 22	
	ϕ Pn (kN)	ϕ Mn (kNm)	ϕ Pn (kN)	ϕ Mn (kNm)
Sentris	9833.59	0	15362.46	0
Patah Desak	6912.09	1343.546	11759.09	2646.706
Balance	5078.539	1487.669	8376.275	3132.135
Patah Tarik	2012.40	1203.954	3123.025	2247.058
Lentur	0	1072.597	0	1834

Didapat dari STAAD Pro

ϕ Pn (kN)	1,766
ϕ Mn (kNm)	1100



Gambar 4.19 Diagram Interaksi Kolom 1146

4.2.2 Perhitungan Penulangan Geser Kolom

Penulangan geser kolom no. 1146 pada portal memanjang line 5.

Diketahui :	h	=	900	mm	f'c	=	30	MPa
	b	=	700	mm	fy_{ulir}	=	400	MPa
	d	=	839.0	mm	fy_{polos}	=	240	MPa

Tinggi bersih ln = 3200 mm

Tulangan sengkang = Ø 10 mm

a. Pengekangan Kolom

Daerah yang berpotensi sendi plastis terletak sepanjang lo (SNI 2847-2013 Pasal 21.6.4.1) dari muka yang ditinjau, dimana panjang lo tidak boleh kurang dari :

- h = 900 mm
- ⅙ ln = ⅙ . 3200 = 533.333 mm
- 450 mm

Jadi daerah yang berpotensi terjadi sendi plastis sejauh 800 mm dari muka kolom.

Persyaratan spasi maksimum pada daerah sendi plastis (SNI 2847-2013 Pasal 21.6.4.3), spasi maksimum tidak boleh melebihi :

- ¼ x dimensi terkecil komponen struktur = ¼ x 700 = 175 mm
- 6 x diameter terkecil komponen struktur = 6 x 22 = 132 mm
- 150 mm

Dipasang tulangan geser 4 Ø 10 mm

$A_s = 4 \times ¼ \times 3,14 \times 10^2$

$= 314.00 \ mm^2$

Jadi As = 314 $mm^2 \geq A_{sh}$

$h_c = 700 - 40 - 40 - 10 = 610 \ mm$

$A_{ch} = (700 - 2 \times 40)^2 = 384400 \ mm^2$

A_{sh} minimum harus memenuhi persyaratan sesuai SNI 2847-2013 Pasal 21.6.4.4.(b) dan diambil nilai yang terbesar dari hasil rumus berikut ini :

$$A_{sh} = 0.3\left(\frac{s.h_c.f'_c}{f_{yh}}\right)\left(\left(\frac{Ag}{A_{ch}}\right) - 1\right)$$

$$314.00 = 0.3\left(\frac{s \times 610 \times 30}{240}\right)\left(\left(\frac{630000}{384400}\right) - 1\right)$$

$$314.00 = 0.3 \times 76.3\ s \times 0.639$$

$$314.00 = 14.61524\ s$$

$$s = 21.484 \text{ mm}$$

atau

$$A_{sh} = 0.09\left(\frac{s.h_c.f'_c}{f_{yh}}\right)$$

$$314.00 = 0.09\left(\frac{s \times 610 \times 30}{240}\right)$$

$$314.00 = 0.09 \times 76.3\ s$$

$$314.00 = 6.8625\ s$$

$$s = 45.756 \text{ mm}$$

Dipakai s = 130 mm

Jadi dipasang tulangan geser 4 Ø 10 - 130 mm.

a. Perhitungan Tulangan Transversal Kolom Akibat Ve

Diketahui : h = 900 mm f'c = 30 MPa

b = 700 mm fy_{ulir} = 400 MPa

d = 839.0 mm fy_{polos} = 240 MPa

Tinggi bersih hn = 3200 mm

Tulangan sengkang = Ø 10 mm

Nu, k = 1766000 N

Perhitungan Momen Probabilitas (Mpr)

Mpr = Mnb = 2366338165.259 Nmm

Karena tulangan longitudinal sepanjang kolom sama, maka Mpr_3 dan Mpr_4

= 2366338165.259 Nmm, sehingga :

$$Ve_{kolom} = \frac{Mpr_3 + Mpr_4}{hn}$$

$$= \frac{2366338165.259 \quad + \quad 2366338165.259}{3200}$$

$$= \quad 1478961.353 \quad N$$

$$Ve_{balok} = \frac{MPr_1 + MPr_2}{hn}$$

$$= \frac{1013945419.139 \quad + \quad 537604701.141}{8850}$$

$$= \quad 175316.398 \quad N \; < \; Ve_{kolom} \; = \; 1478961.353 \quad N$$

Vc = apabila memenuhi ketentuan pada SNI 2847-2013 Pasal 21.5.4.2

sebagai berikut :

Gaya aksial terfaktor < Ag.f'c/20

$$1766000 \quad N \; < \; \frac{900 \quad x \quad 700 \quad x \quad 30}{20}$$

$$1766000 \quad N \; > \; 945000 \quad N$$

Maka dipakai Vc sesuai dengan SNI 2847-2013 Pasal 11.2.1.2 :

$$Vc \; = \; 0.17 \left(1 \; + \; \frac{Nu}{14.Ag} \right) \lambda \; x \; \sqrt{fc'} \; x \, bw \, x \, d$$

$$= \; 0.17 \left(1 \; + \; \frac{1766000}{14 \quad x \quad 630000} \right) x \; 1 \; x \; \sqrt{30} \quad x \quad 700 \quad x \quad 839$$

$$= \quad 656346.017 \quad N$$

- Tulangan geser di dalam daerah sendi plastis

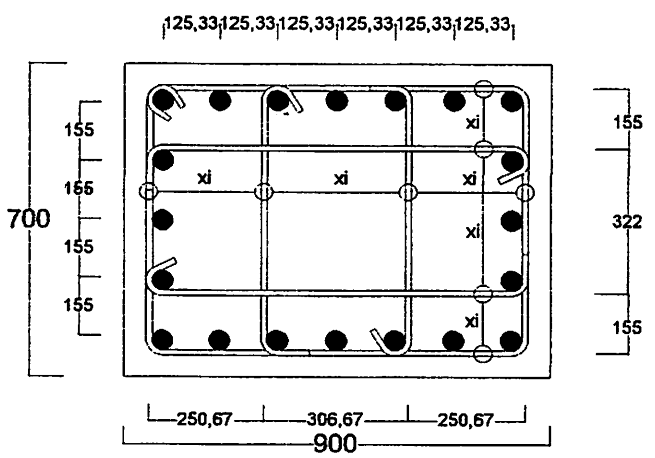
  Daerah yang berpotensi sendi plastis terletak sepanjang lo (SNI 2847-2013 Pasal 21.6.4.1) dari muka yang ditinjau, dimana panjang lo tidak boleh kurang dari :

  - h = 900 mm
  - ⅙ ln = ⅙ . 3200 = 533.333 mm
  - 450 mm

  Jadi daerah yang berpotensi terjadi sendi plastis sejauh 800 mm dari muka kolom

Persyaratan spasi maksimum pada daerah sendi plastis (SNI 2847-2013 Pasal 21.6.4.3), spasi maksimum tidak boleh melebihi :

- ¼ x dimensi terkecil komponen struktur = ¼ x 700 = 175 mm
- 6 x diameter terkecil komponen struktur = 6 x 22 = 132 mm



Gambar 4.20. tulangan transfersal pada kolom

- $S_o = 100 + \dfrac{350 - h_x}{3}$, dimana h_x = xi = 322 mm

$= 100 + \dfrac{350 - 322}{3} = 109$ mm

Dipasang tulangan geser 4 Ø 10 mm

$A_s = 4 \times ¼ \times 3{,}14 \times 10^2$

$= 314.00 \text{ mm}^2$

Jadi As = 314 mm^2 ≥ A_{sh}

h_c = 700 - 40 - 40 - 10 = 610 mm

$A_{ch} = (700 - 2 \times 40)^2 = 384400 \text{ mm}^2$

A_{sh} minimum harus memenuhi persyaratan sesuai SNI 2847-2013 Pasal 21.6.4.4.(b) dan diambil nilai yang terbesar dari hasil rumus berikut ini :

$$A_{sh} = 0.3\left(\frac{s.h_c.f'_c}{f_{yh}}\right)\left(\left(\frac{Ag}{A_{ch}}\right) - 1\right)$$

$$314.00 = 0.3\left(\frac{s \times 610 \times 30}{240}\right)\left(\left(\frac{630000}{384400}\right) - 1\right)$$

$$314.00 = 0.3 \times 76.3\ s \times 0.639$$

$314.00 = 14.61524\ s$

$s = 21.484$ mm

atau

$$A_{sh} = 0.09\left(\frac{s.h_c.f'_c}{f_{yh}}\right)$$

$$314.00 = 0.09\left(\frac{s \times 610 \times 30}{240}\right)$$

$314.00 = 0.09 \times 76.3\ s$

$314.00 = 6.8625\ s$

$s = 45.756$ mm

Dipakai s = 100 mm

$$Vs = \frac{As\,.\,fy\,.d}{s} = \frac{314 \times 240 \times 839.0}{100}$$

$$= 632270.400 \text{ N}$$

Jadi dipasang tulangan geser 4 Ø 10 - 100 mm

Kontrol kuat geser nominal menurut SNI 2847-2013 Pasal 11.4.7.9

$Vs \leq 0.66\ \sqrt{fc'}\ \ bw\,.\,d$

$Vs \leq 0.66\ \sqrt{30} \times 700 \times 839.0$

632270.400 N < 2123071.223 N .... OK

Maka :

$\phi\,(V_s + V_c) = 0.75\ [\ 632270.400 + 656346.017\ ]$

$= 966462.313$ N > Vu = 175316.398 N .... OK

Jadi untuk penulangan geser di daerah yang berpotensi terjadi sendi plastis sejauh lo = 900 mm dipasang tulangan geser 4 Ø 10 - 100

- Tulangan geser di luar daerah sendi plastis

Persyaratan spasi maksimum untuk daerah luar sendi plastis menurut SNI 2847-2013 Pasal 21.6.4.5, spasi maksimum tidak boleh melebihi :

- 6 x diameter tulangan utama = 6 x 22 = 132 mm
- 150 mm

Dipakai sengkang 4 Ø 10 dengan spasi 130 mm

$$V_s = \frac{As.fy.d}{s} = \frac{314.00 \times 240 \times 839.0}{130}$$

$$= 486361.846 \text{ N}$$

Kontrol kuat geser nominal menurut SNI 2847-2013 Pasal 11.4.7.9

$Vs \leq 0.66 \sqrt{fc'} \; bw.d$

$Vs \leq 0.66 \sqrt{30} \times 700 \times 839.0$

486361.846 N < 2123071.223 N .... OK

Maka :

$\phi (V_s + V_c) = 0.75 \left(486361.846 + 656346.017 \right)$

= 857030.897 N > Vu = 175316.398 N .... OK

Jadi untuk penulangan geser di luar sendi plastis dipasang tulangan geser

4 Ø 10 - 130 mm

4.3 Sambungan Lewatan Tulangan Vertikal Kolom

Sesuai SNI 2847-2013 Pasal 12.2.3 panjang sambungan lewatan harus dihitung sesuai dengan rumus sebagai berikut :

$$l_d = \left(\frac{f_y}{1{,}1\lambda \sqrt{fc'}} \cdot \frac{\Psi_t \, \Psi_o \, \Psi_s}{\left(\frac{c_b + K_{tr}}{d_b} \right)} \right) d_b$$

dimana : $\Psi t = 1$ $\Psi o = 1$ $\Psi s = 0.8$ $\lambda = 1$

c = selimut beton + Ø sengkang + ½ D kolom

= 40 + 10 + (½ . 22)

= 61.0 mm

$$c = \frac{700 - 2 \left(40 + 10 \right) - 22}{2 \times 4}$$

= 72.25 mm

diambil c = 72.25 mm yang menentukan

$K_{tr} = 0$

$$\left(\frac{c_b + K_{tr}}{d_b}\right) = \frac{72.25 \quad + \quad 0}{22} = 3.284$$

$$\text{Sehingga}: \; l_d = \frac{400}{1.1 \quad x \quad 1\sqrt{30}} \cdot \frac{1 \;\; x \;\; 1 \;\; x \;\; 0.8}{3.284} \cdot 22$$

$$= 355.799 \text{ mm}$$

Sesuai Pasal 21.6.3.3, sambungan lewatan harus diletakan ditengah panjang kolom dan harus dihitung sebagai sambungan tarik.

Mengingat sambungan lewatan ini termasuk kelas B, maka panjangnya harus = 1,3 l_d = 1.3 x 355.799 = 462.538 mm ≈ 400 mm.

spasi sengkang pada daerah sambungan lewatan, tidak boleh melebehi yang terkecil dari syarat-syarat pada SNI-03-2847-2013 pasal 21.5.2.3 yaitu :

- $d/4 = \dfrac{839.0}{4} = 209.75$ mm
- 100 mm

Maka spasi tulangan geser pada sambungan lewatan digunakan sebesar 100 mm

4.4 Kontrol Desain Kapasitas

Kontrol desain kapasitas untuk joint 1146

a. Momen pada kolom

Me atas = Me bawah = Mnc = 2288722162.401 Nmm

b. Momen pada balok

Mpr^{-} = 1013945419.139 Nmm

Mpr^{+} = 537604701.141 Nmm

$$\sum Mnc \geq 1.2 \sum Mnb$$

$$\sum Mnc = \frac{2288722162.401 \quad + \quad 2288722162.401}{0.65}$$

$$= 7042222038.157 \text{ Nmm}$$

$$1.2 \sum Mnb = \frac{1.2 \quad x \quad 1013945419.139 \quad + \quad 537604701.141}{0.9}$$

$$= 2068733493.706 \quad Nmm$$

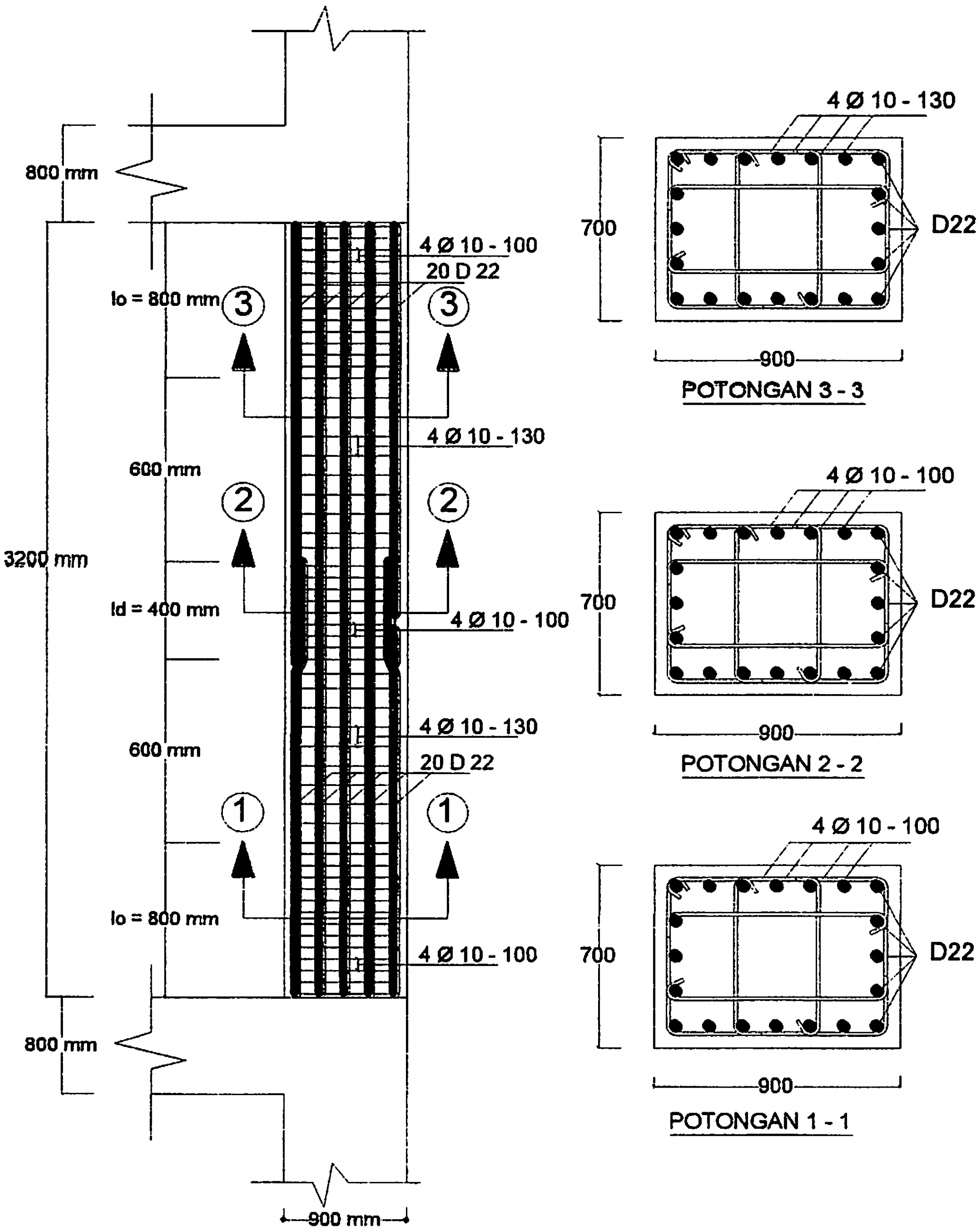
Maka :

$$\sum Mnc \geq 1.2 \sum Mnb$$

7042222038.157 Nmm > 2068733493.706 Nmm .... OK

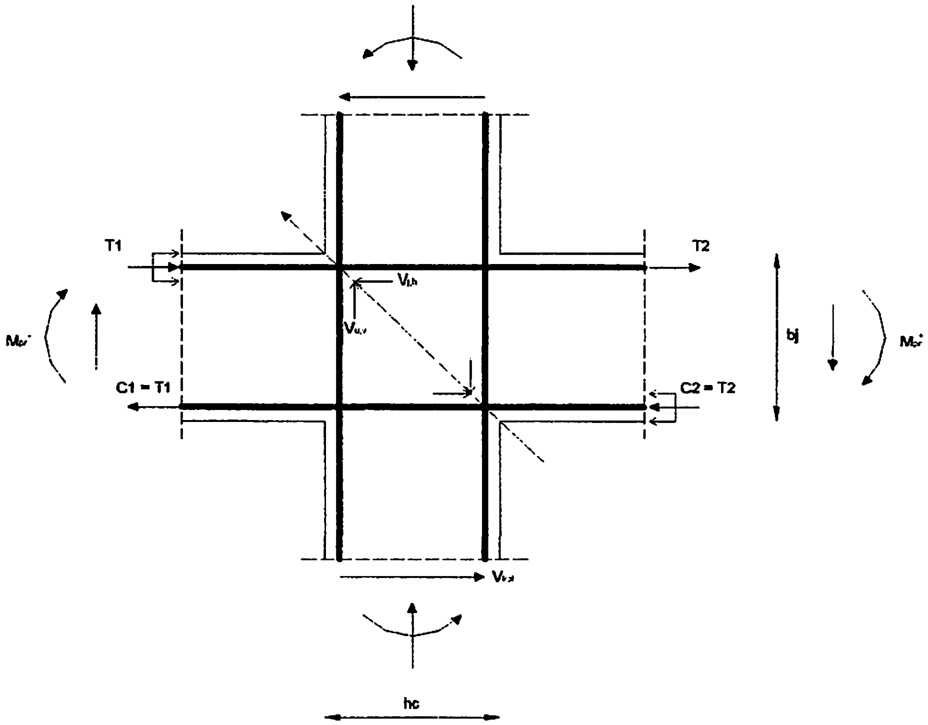
Dari hasil perencanaan balok dan kolom dapat disimpulkan bahwa :

Persyaratan "Strong Column Weak Beam" telah terpenuhi ....OK



Gambar 4.20 Detail Penulangan Longitudinal dan Transversal Kolom 1146

4.5 Perhitungan Pertemuan Balok-Kolom



Gambar 4.21 Analisa geser dari hubungan balok kolom *(Joint 1146)*

Data perencanaan :

f'c	=	30 MPa
fy	=	400 MPa
Mpr^-, b	=	968739364.580 Nmm
Mpr^+, b	=	0.000 Nmm
hn, a	=	3200 mm
hn, b	=	3200 mm

Tulangan yang terpasang pada balok :

balok kiri = 6 D 22

balok kanan = 0 D 22

Pemeriksaan kuat geser nominal pada joint :

Gaya geser yang terjadi

$As_1 = 6 \ . \ ¼ \ . \ 3.14 \ . \ 22^2 = 2279.64 \ mm^2$

$T = As \ . \ 1{,}25 \ . \ fy$

$T_1 = 2279.64 \ . \ 1.25 \ . \ 400 = 1139820.000 \ N$

$$Mu = \frac{Mpr, b\ kiri + Mpr, b\ kanan}{2}$$

$$= \frac{968739364.580 \quad + \quad 0.000}{2}$$

$$= 484369682.290 \quad Nmm$$

$$Vh = \frac{2 \times Mu}{h_n / 2}$$

$$= \frac{2 \times 484369682.290}{3200 / 2}$$

$$= 605462.103 \quad N$$

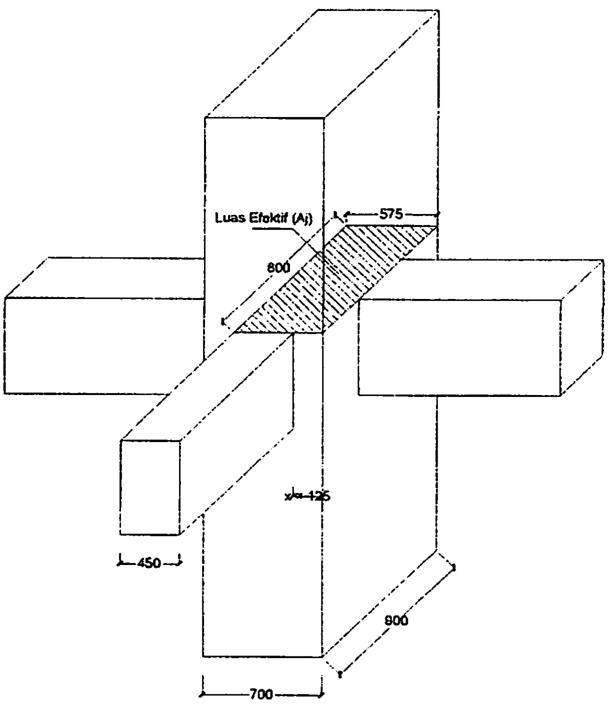
$$Vjh = T_1 - Vh$$

$$= 1139820.000 - 605462.103$$

$$= 534357.897 \quad N$$

Kuat geser nominal untuk HBK yang terkekang tiga sisinya maka berlaku :

$$V_{jh} < \phi \times 1.25 \times \sqrt{fc'} \times Aj$$



Gambar 4.22 Luas efektif *(Aj)* untuk HBK

Maka :

$V_{jh} \quad < \phi \quad x \quad 1.25 \quad x \quad \sqrt{fc'} \quad x \quad Aj$

$534357.897 \quad < \quad 0.75 \quad x \quad 1.25 \quad x \quad \sqrt{30} \quad x \quad 575 \quad x \quad 900$

534357.897 N < 2657310.22 N .... OK

- Penulangan geser horisontal

Nu = 1766000 N

$$\frac{Nu}{Ag} = \frac{1766000}{900 \quad x \quad 700}$$

$= \quad 3.504 \quad N/mm^2 \quad > 0,1 \,.\, f'c \quad = \quad 0.1 \quad x \quad 30 \quad = \quad 3.0 \quad N/mm^2$

Jadi $V_{c,h}$ dihitung menurut persamaan

$$V_{c,h} = \frac{2}{3}\sqrt{\left(\frac{Nu,k}{Ag} - 0,1 \times f'c\right)} \times bj \times hc$$

$$= \frac{2}{3}\sqrt{\left(\frac{Nu\,,\,k}{630000} - 0,1 \times 30\right)} \times 700 \times 900$$

= 298161.030 N

$V_{s,h} \quad + V_{c,h} \quad = V_{j,h}$

$V_{s,h} \quad = \quad V_{j,h} \quad - \quad V_{c,h}$

= 534357.897 - 298161.030

= 236196.867 N

$$A_{j,h} = \frac{V_{s,h}}{fy}$$

$$= \frac{236196.867}{400}$$

$= 590.4921671 \; mm^2$

Coba dipasang 4 lapis tulangan sengkang :

Maka As ada = 4 . 2279.64

$= \quad 9118.560 \quad mm^2 \quad > A_{j,h} \quad = \quad 590.492167 \quad mm^2$ ... Aman

- Penulangan geser vertikal

$$V_{j,v} = \frac{hc}{bj} \; V_{j,h}$$

$$= \frac{900}{700} \times \quad 534357.897$$

$$= \quad 687031.582 \quad N$$

$$V_{c,v} = \frac{As' . \, V_{j,h}}{As} \times \left(0.6 \quad + \frac{Nu, \, k}{Ag \, . \, f'c} \right)$$

$$= \frac{1519.76 \quad \times \quad 687031.582}{2279.64} \times \left(0.6 \quad + \frac{1766000}{630000 \quad \times \quad 30} \right)$$

$$= \quad 317609.732 \quad N$$

$$V_{s,v} = V_{j,v} \; - \; V_{c,v}$$

$$= \quad 687031.582 \quad - \quad 317609.732$$

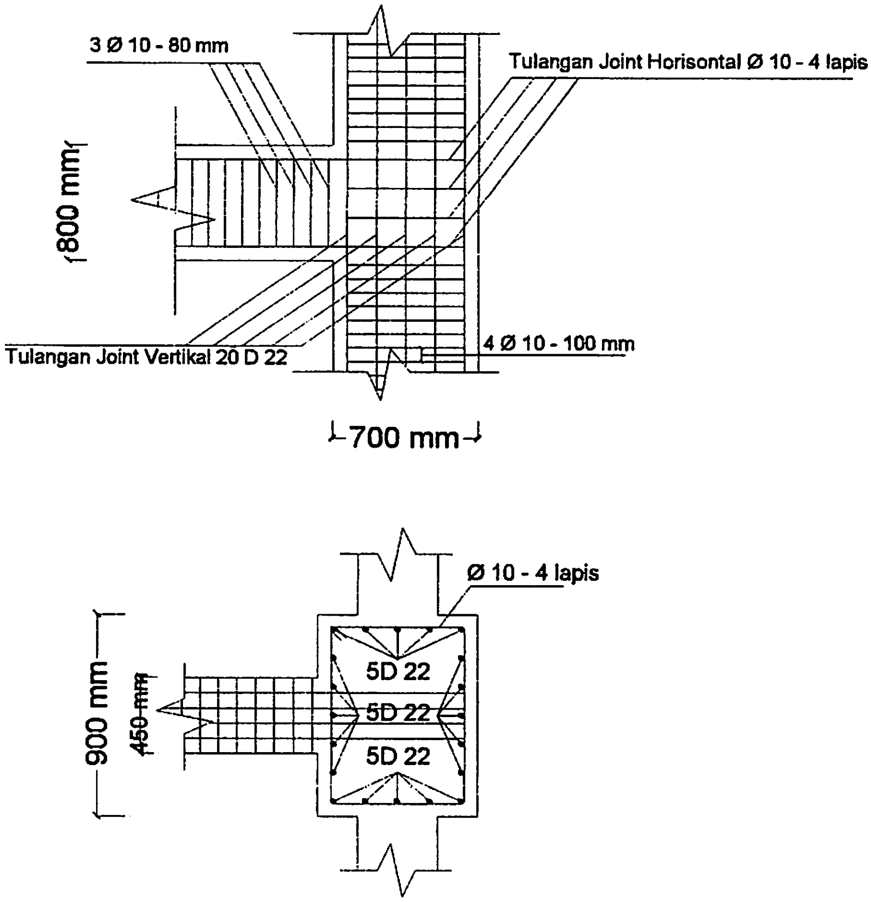
$$= \quad 369421.850 \quad N$$

$$A_{j,v} = \frac{V_{s,v}}{fy}$$

$$= \frac{369421.850}{400}$$

$$= \quad 923.55 \quad mm^2$$

Tulangan kolom yang terpasang 20 D 22, dimana luas tulangan (As ada = 7598.80 mm^2) > 923.555 mm^2. Maka tidak diperlukan lagi tulangan geser vertikal karena sudah ditahan oleh tulangan kolom yang terpasang.



Gambar 4.23 Penulangan Hubungan Balok Kolom *(Joint 1146)*

4.6 Perhitungan Pendetailan Tulangan

Perhitungan pendetailan joint 1146

- Pendetailan Tulangan Tumpuan Tarik (atas)
  - Untuk pemberhentian tulangan tumpu tarik ke dalam balok adalah sejauh

    ¼ Ln = ¼ . 8850 = 2213 mm dari muka kolom.

    Ditambah dengan penjangkaran yang diperlukan untuk penjangkaran sejauh

    12 db = 14 x 22 = 308 mm

    $$\frac{1}{16}\ \text{ln} = \frac{1}{16} \times 8850 = 553.125 \text{ mm}$$

    d = 739.00 mm

    Dipakai perpanjangan 735.00 mm

Total panjang yang diperlukan = 2213 + 739.00 = 2951.50 mm

Modifikasi yang digunakan :

† Batang tulangan baja paling atas dengan elevasi antara tulangan tersebut dengan lapisan beton terbawah tidak kurang dari 300 mm.

800 - 40 - 10 - (0,5 x 22) = 739.00 mm > 300 mm

† Ld yang dibutuhkan adalah :

$$Ldb = \frac{0{,}02.As.fy}{\sqrt{fc'}} = \frac{0{,}02.(\frac{1}{4}.\pi.22^2).390}{\sqrt{30}} = 554.938 \text{ mm}$$

Ldb = 0.07 . 22 . 400 = 616.000 mm

Dipakai Ldb = 616.000 mm

Dipakai faktor 1.4

Maka Ld = 616.000 x 1.4

= 862.400

Ld = 862.400 + 739.00

= 1601.400 mm < 2951.50 mm

Jadi dipakai panjang penyaluran Ld = 2951.50 mm

- Penjangkaran masuk ke dalam kolom
  - Pendetailan tulangan tumpuan tekan balok (SNI 2847-2013 Pasal 12.3.2)

    Untuk tulangan tumpuan tekan, panjang penyaluran yang masuk ke dalam kolom adalah :

$$Ldb = \frac{db.fy}{4\sqrt{fc'}} = \frac{22 \times 400}{4\sqrt{30}} = 401.663 \text{ mm}$$

Panjang Ldb tidak boleh kurang dari :

Ldb = 0.04 . 22 . 400 = 352.000 mm

Ldb = 200 mm

Dipakai Ldb = 401.663 mm ≈ 400 mm

- Pendetailan tulangan tumpuan tarik balok (SNI 2847-2013 Pasal 12.5.2)

$$Lhb = \frac{100.db}{\sqrt{fc'}} = \frac{100 \quad x \quad 22}{\sqrt{30}} = 401.663 \quad mm$$

Tidak kurang dari :

8 db = 8 x 22 = 176 mm

Ldh = 401.663 mm > 8 db = 176 mm

Dipakai Ldh = 401.663 mm ≈ 400 mm

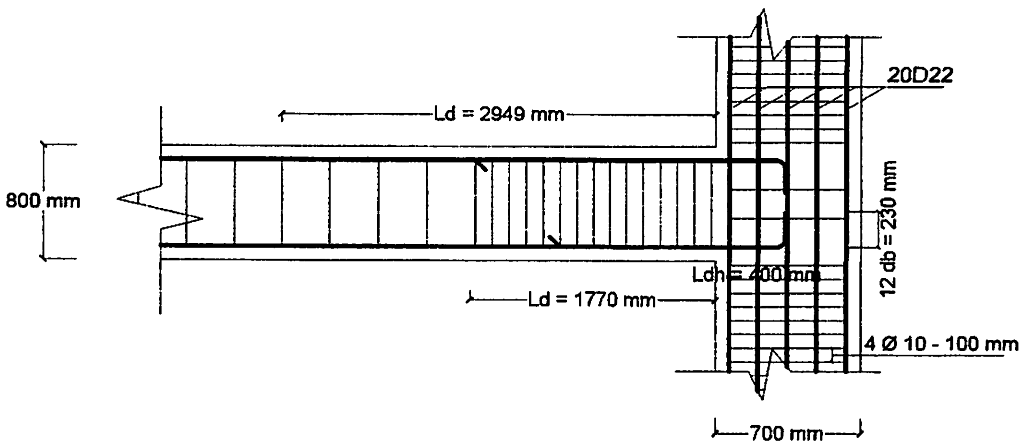
Dipilih pembengkokan 90° dengan panjang pembengkokan 12 db

= 12 x 22 = 264 mm ≈ 230 mm

- Pemutusan tulangan tumpuan tekan

Untuk pemberhentian tulangan tumpuan tekan adalah sejauh

⅕ ln = ⅕ x 8850 = 1770 mm dari muka kolom.



Gambar 4.24 Pendetailan Tulangan Joint 1146

No	Joint	Vu	Ø (Vs + Vc)	Pada sendi plastis
1	1143	162266.5	2220998.38	6 Ø 12 -100
2	1141	173752.6	1128324.67	4 Ø 12 -100

Tabel. 4.2.2 Tulangan Geser Kolom Pada Sendi Plastis

No	Joint	Vu	Ø (Vs + Vc)	Pada luar sendi plastis
1	1143	162266.4	1992711.34	6 Ø 10 -130
2	1141	173752.6	1283297.81	4 Ø 10 -130

Tabel 4.2.3 Tulangan Geser Kolom Pada luar Sendi Plastis

No	Joint	Vu (d)	Ø Vn	Pada sendi plastis
1	2359	221.006	266.74	4 Ø 10 - 100

Tabel 4.2.4 Tulangan Geser Balok Pada Sendi Plastis

No	Joint	Vu (2h)	Ø Vn	Pada luar sendi plastis
1	2359	182.593	266.74	3 Ø 10 - 150

Tabel 4.2.5 Tulangan Geser Balok Pada luar Sendi Plastis

BAB V

PENUTUP

5.1. Kesimpulan

Setelah dilakukan beberapa tahap dalam studi perencanaan portal gedung hotel Aria Centra Surabaya didapatkan hasil perhitungan dimensi penampang dan tulangan balok, kolom, dan hubungan balok kolom., dan dalam perhitungan ini diambil sampel pada portal line 5. Kemudian dari perencanaan tersebut didadapat hasil sebagai berikut:

❖ Hasil perhitungan pada balok lantai 15 join 1146 dan 1143 (pada tumpuan), join 21807 (pada lapangan), yaitu:

- Dimensi Balok : 45/80
- Tulangan Tumpuan Kiri : 6 D 22 (atas), 4 D 22 (bawah)
- Tulangan Lapangan : 6 D 22 (atas), 4 D 22 (bawah)
- Tulangan Tumpuan Kanan : 6 D 22 (atas), 4 D 22 (bawah)
- Tulangan Geser :

  Joint Kiri = joint kanan

  Daerah sendi plastis : 2 (kaki) Ø 10 - 30

  Daerah Luar Sendi Plastis : 3 (kaki) Ø 10 - 70

❖ Kolom pada portal ini direncanakan dengan menggunakan dimensi 70/90 dengan jumlah tulangan pada kolom nomor 1146 didapat tulangan lentur 20 D 22, dengan spesifikasi tulangan geser:

Daerah sendi plastis : 4 kaki Ø 14 - 90

Daerah luar sendi plastis : 4 kaki Ø 14 - 100

- Untuk hasil perhitungan tulangan pada joint lainnya dan hasil gambar terlampir pada lampiran.

5.2. Saran

Untuk perencanaan gedung tahan gempa selanjutnya, diharapkan perencana memahami dan menggunakan aturan-aturan perencanaan gedung tahan gempa. Kemudian dalam perencanaan juga harus memahami aturan-aturan yang tertera pada Standar Nasional Indonesia yang digunakan secara umum diIndonesia. Selain itu, ketelitian dalam melakukan perencanaan juga harus diperhatikan, jadi dalam melakukan perencanaan harus disertai kehati-hatian.

DAFTAR PUSTAKA

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"ASSALAMUALAIKUM WR. WB."

"OM SWASTYASTU"

"SALAM SEJAHTERA BUAT KITA SEMUA"

"SEMOGA SELALU TERJAGA DAN LINGUNGAN NYA AAMIN"

- Buat bapak dan ibu orang tua tercinta yang selalu sayang kepada saya, terima kasih dan doa yang selalu saya curahkan tiada henti karena kerja keras beliau yang selalu support dalam lisan, doa, dan materi. Semoga ilmu pelajaran hidup ini bisa bermanfaat yang selalu buat beliau bangga dunia dan akhirat. Sehat selalu Bapak Ibu ku. .Aamiin Mujibas Sailiin. Aamiin Ya-Robb..
- Buat istriku ervina tercinta terima kasih sudah banyak menyuport aku dalam banyak hal, I Love U my Honny, dan buat bapak ibu mertua juga, terima kasih banyak.
- Buat adik ku siget terima kasih banyak sudah memberi dukungan moral maupun lisan, maaf masih belom bisa membanggakan kamu I Love U dik.
- Buat para ulama, Pak Nasrudin, cak Ulum, ayah dari tante, terima kasih banyak atas jasa bimbingan agamanya, yang selalu memberi amalan hidup, pelajaran hidup, terima kasih.
- Buat sodara ku hijau hitam HMI Madani, terlalu banyak kenanga dan aku slalu merindukan itu, terma kasih atas ilmu yang tercurahkan. Ilmu engkau tiada habis dan mati selama hidup di dun dan ahirat. "YAKUSA" Selalu Yakin dan Usaha Akan Sampai. ..Aamiinnn..
- Buat teman seperjuangan ku angkatan 2010, terima kasih sudah membantuku dalam bentuk akademisi dan organisasi. .terutama buat saodara ku tercinta sannur, dekri, sany, onteh gogon, dan dimas odong-odongg. ...
- Terima kasih banyak juga buat mas. Ripki, wahyu kucing, dwi ratno, dan kawar, yang selalu membagun ilmu organisasi hidup untuk bermasyarakat..
- Buat mak.ning dan minah terima kasih sudah menampung tempat parkir dan tempat ngopi.

SATU WARNA

"BIRU"

SATU RASA

"SENASIP SEPENANGGUNGAN"

SATU SUARA

"SATU SIPIL - SIPIL SATU"

YAKUSA

YAKIN USAHA SAMPAI

HMI MADANI

EKO BUDI SUSANTO

LAMPIRAN

STAAD PRO

- INPUT STAAD PRO 2004
- OUTPUT STAAD PRO 2004
- GAMBAR 3D
- TABEL BEAM DAN FORCE

Job Information

	Engineer	Checked	Approved
Name:	Eko Budi S		
Date:	06-Jul-15		

Structure Type	SPACE FRAME

Number of Nodes	16501	Highest Node	16746
Number of Elements	9120	Highest Beam	25022
Number of Plates	15360	Highest Plate	24974

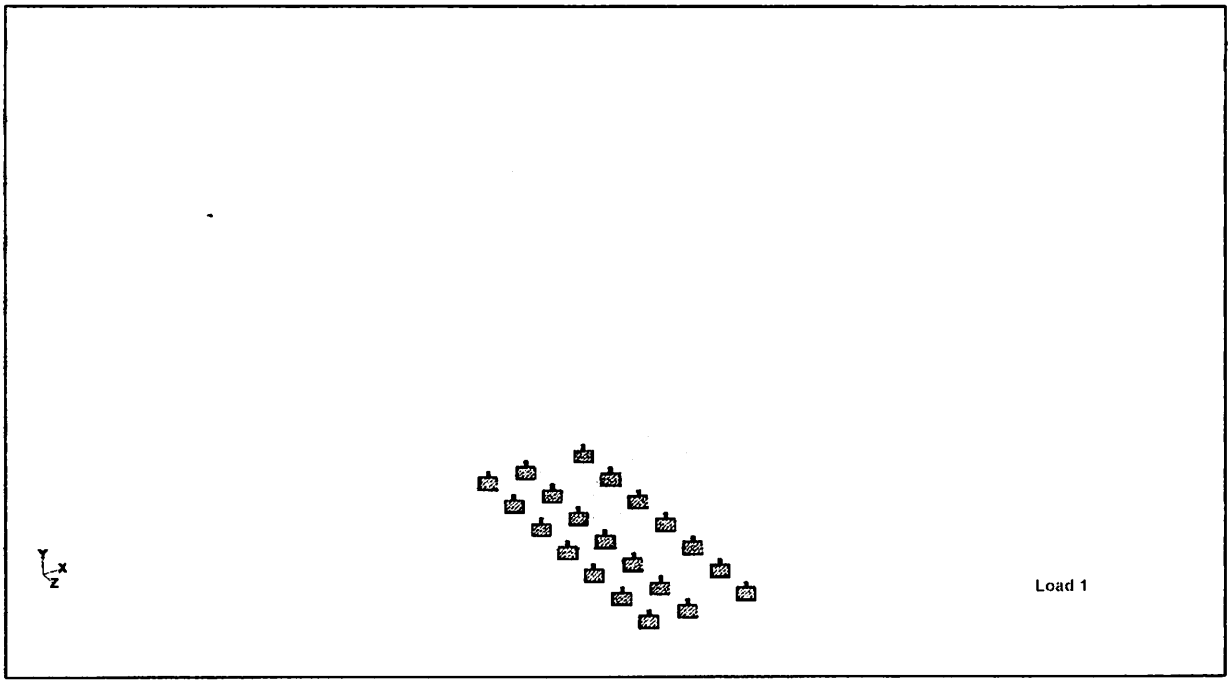
Number of Basic Load Cases	4
Number of Combination Load Cases	18

Included in this printout are data for:

All	The Whole Structure

Included in this printout are results for load cases:

Type	L/C	Name
Primary	1	BEBAM MATI
Primary	2	BEBAN HIDUP
Primary	3	BEBAN GEMPA ARAH X
Primary	4	BEBAN GEMPA ARAH Z
Combination	5	KOMBINASI 2
Combination	6	KOMBINASI 1
Combination	7	KOMBINASI 3
Combination	8	KOMBINASI 4
Combination	9	KOMBINASI 5
Combination	10	KOMBINASI 6
Combination	11	KOMBINASI 7
Combination	12	KOMBINASI 8
Combination	13	KOMBINASI 9
Combination	14	KOMBINASI 10
Combination	15	KOMBINASI 11
Combination	16	KOMBINASI 12
Combination	17	KOMBINASI 13
Combination	18	KOMBINASI 14
Combination	19	KOMBINASI 15
Combination	20	KOMBINASI 16
Combination	21	KOMBINASI 17
Combination	22	KOMBINASI 18



portal

Node Displacement Summary

	Node	L/C	X (mm)	Y (mm)	Z (mm)	Resultant (mm)	rX (rad)	rY (rad)	rZ (rad)
Max X	1285	7:KOMBINASI	**15.213**	-19.256	-0.000	24.541	0.000	-0.000	-0.000
Min X	5780	16:KOMBINAS	**-3.517**	-8.704	0.000	9.388	0.000	-0.000	-0.001
Max Y	12438	3:BEBAN GEM	0.858	**0.260**	0.000	0.896	0.000	0.000	0.000
Min Y	16469	5:KOMBINASI	11.475	**-35.139**	-0.071	36.965	-0.000	-0.000	0.000
Max Z	1321	7:KOMBINASI	15.001	-12.615	**0.236**	19.602	0.001	0.000	-0.000
Min Z	1249	8:KOMBINASI	5.369	-13.218	**-0.236**	14.269	-0.001	-0.000	-0.001
Max rX	16336	5:KOMBINASI	11.444	-24.376	0.111	26.929	**0.003**	0.000	0.000
Min rX	16510	5:KOMBINASI	11.444	-24.376	-0.111	26.929	**-0.003**	-0.000	0.000
Max rY	16552	7:KOMBINASI	14.983	-23.619	0.133	27.971	0.001	**0.000**	0.001
Min rY	16665	8:KOMBINASI	5.223	-24.224	-0.133	24.781	-0.001	**-0.000**	0.001
Max rZ	16529	5:KOMBINASI	11.436	-24.016	0.125	26.600	0.002	-0.000	**0.002**
Min rZ	16184	5:KOMBINASI	11.497	-28.757	0.098	30.970	0.002	0.000	**-0.002**
Max Rst	16469	5:KOMBINASI	11.475	-35.139	-0.071	**36.965**	-0.000	-0.000	0.000

Beam End Displacement Summary

Displacements shown in italic indicate the presence of an offset

	Beam	Node	L/C	X (mm)	Y (mm)	Z (mm)	Resultant (mm)
Max X	2574	1285	7:KOMBINASI	**15.213**	-19.255	0.000	24.539
Min X	9223	5780	16:KOMBINAS	**-3.517**	-8.705	0.000	9.388
Max Y	18820	12438	3:BEBAN GEM	0.858	**0.260**	0.000	0.897
Min Y	24503	16379	5:KOMBINASI	11.475	**-35.136**	0.071	36.962
Max Z	2583	1321	7:KOMBINASI	15.001	-12.613	**0.236**	19.600
Min Z	2565	1249	8:KOMBINASI	5.369	-13.221	**-0.236**	14.271
Max Rst	24503	16379	5:KOMBINASI	11.475	-35.136	0.071	**36.962**

Beam End Force Summary

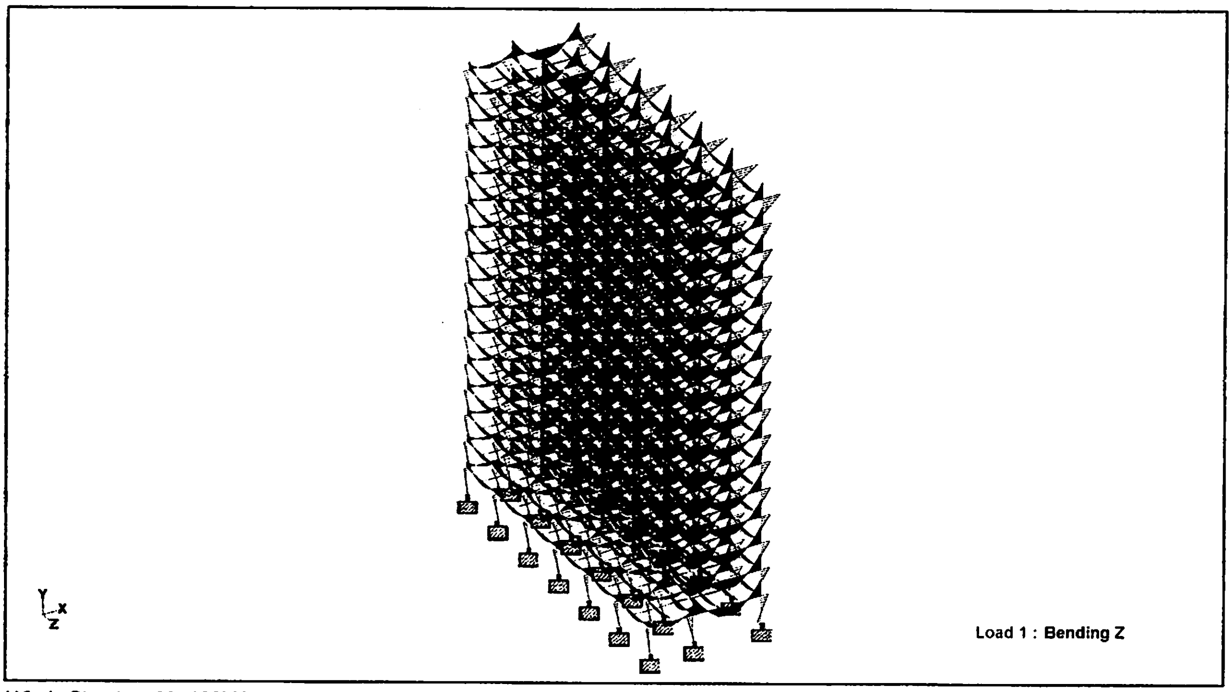
The signs of the forces at end B of each beam have been reversed. For example: this means that the Min Fx entry gives the largest tension value for an beam.

				Axial	Shear		Torsion	Bending	
	Beam	Node	L/C	Fx (kg)	Fy (kg)	Fz (kg)	Mx (kNm)	My (kNm)	Mz (kNm)
Max Fx	164	27	5:KOMBINASI	**1.62E 6**	-7.71E 3	-118.186	-0.020	2.310	-62.394
Min Fx	24975	1976	7:KOMBINASI	**-13.3E 3**	25.4E 3	-0.000	0.000	-0.000	52.726
Max Fy	240	93	5:KOMBINASI	-3.3E 3	**37.2E 3**	3.824	-1.778	-0.047	740.176
Min Fy	23425	1224	5:KOMBINASI	-7.46E 3	**-39E 3**	-32.144	-1.042	-0.159	744.044
Max Fz	2584	1245	5:KOMBINASI	66.5E 3	-10.5E 3	**19.3E 3**	-0.789	-288.582	-143.838
Min Fz	2566	1173	5:KOMBINASI	66.5E 3	-10.5E 3	**-19.3E 3**	0.789	288.582	-143.838
Max Mx	23341	15569	5:KOMBINASI	-5.21E 3	-22.4E 3	500.838	**105.102**	-0.780	283.934
Min Mx	23601	15747	5:KOMBINASI	-5.21E 3	-22.4E 3	-500.838	**-105.102**	0.780	283.934
Max My	2584	1323	5:KOMBINASI	55.2E 3	-10.5E 3	19.3E 3	-0.789	**470.436**	266.468
Min My	2566	1251	5:KOMBINASI	55.2E 3	-10.5E 3	-19.3E 3	0.789	**-470.436**	266.468
Max Mz	23425	1224	5:KOMBINASI	-7.46E 3	-39E 3	-32.144	-1.042	-0.159	**744.044**
Min Mz	2573	1278	5:KOMBINASI	60.6E 3	30.2E 3	101.638	0.184	1.230	**-720.249**

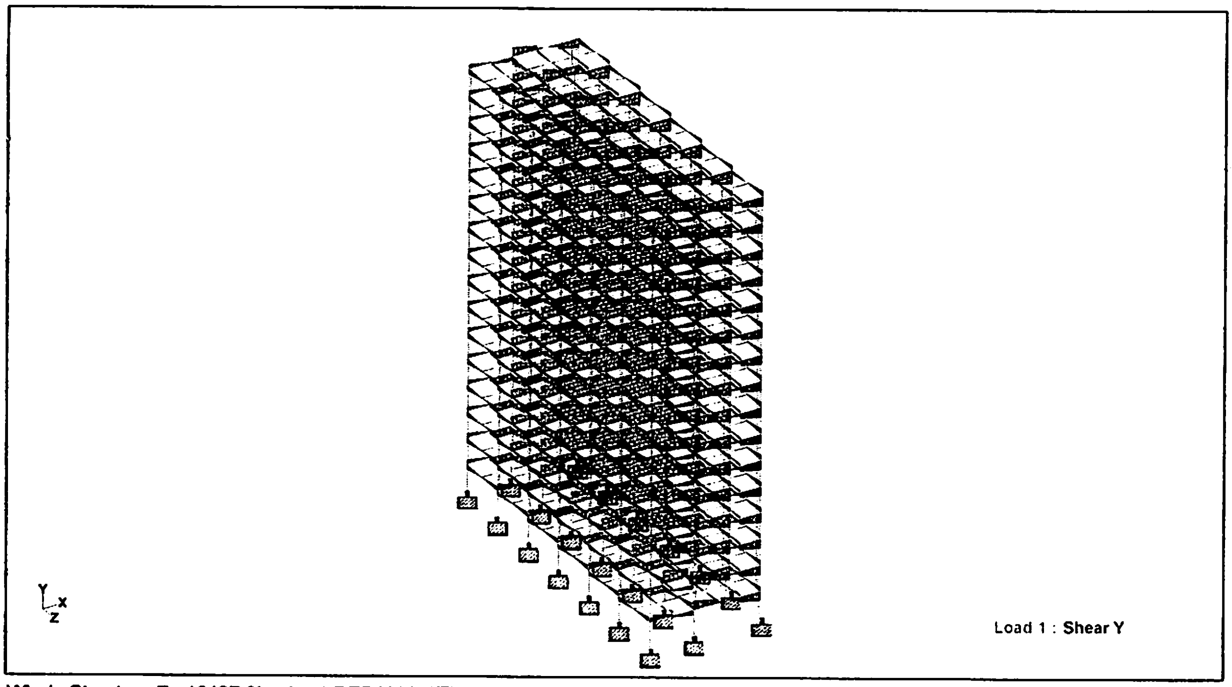
Beam Force Detail Summary

Sign convention as diagrams:- positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	Shear		Torsion	Bending	
	Beam	L/C	d (m)	Fx (kg)	Fy (kg)	Fz (kg)	Mx (kNm)	My (kNm)	Mz (kNm)
Max Fx	164	5:KOMBINASI	0.000	**1.62E 6**	-7.71E 3	-118.186	-0.020	2.310	-62.394
Min Fx	24975	7:KOMBINASI	0.092	**-13.3E 3**	25.4E 3	-0.000	0.000	-0.000	52.726
Max Fy	240	5:KOMBINASI	0.000	-3.3E 3	**37.2E 3**	3.824	-1.778	-0.047	740.176
Min Fy	23425	5:KOMBINASI	0.792	-7.46E 3	**-39E 3**	-32.144	-1.042	-0.159	744.044
Max Fz	2584	5:KOMBINASI	0.000	66.5E 3	-10.5E 3	**19.3E 3**	-0.789	-288.582	-143.838
Min Fz	2566	5:KOMBINASI	0.000	66.5E 3	-10.5E 3	**-19.3E 3**	0.789	288.582	-143.838
Max Mx	23341	5:KOMBINASI	0.000	-5.21E 3	-22.4E 3	500.838	**105.102**	-0.780	283.934
Min Mx	23601	5:KOMBINASI	0.000	-5.21E 3	-22.4E 3	-500.838	**-105.102**	0.780	283.934
Max My	2584	5:KOMBINASI	4.000	55.2E 3	-10.5E 3	19.3E 3	-0.789	**470.436**	266.468



Whole Structure Mz 100kNm:1m 1 BEBAM MATI



Whole Structure Fy 10197.2kg:1m 1 BEBAM MATI

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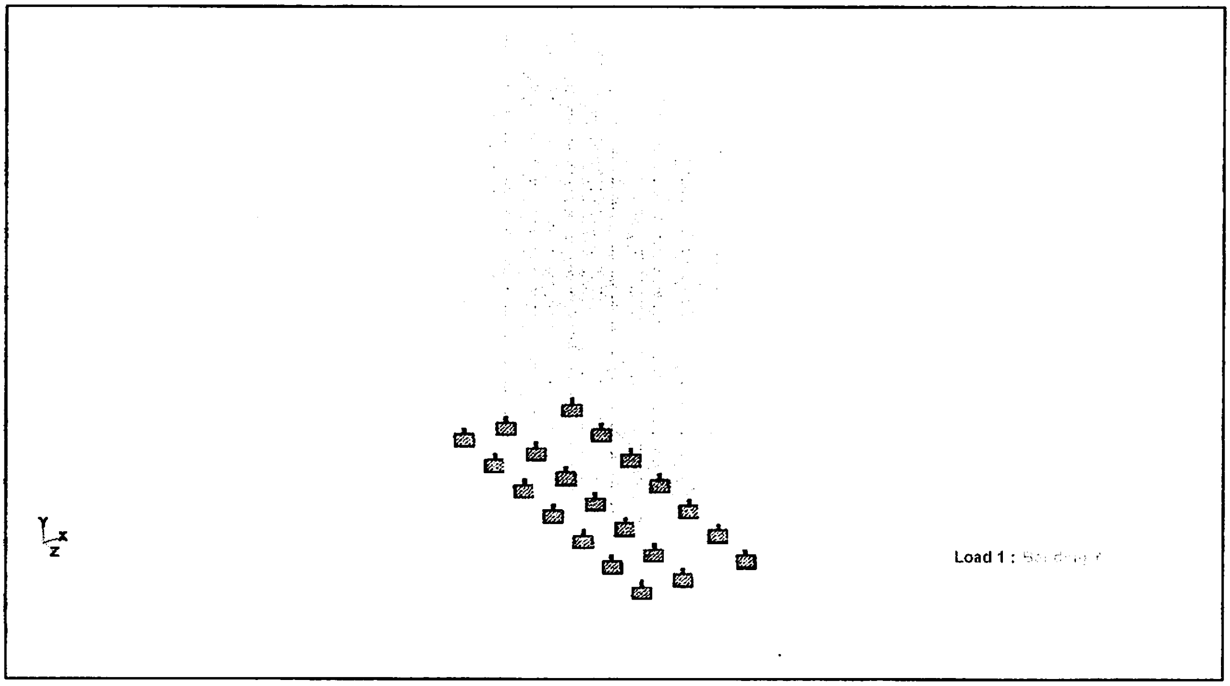
Client	File akang eko.std		Date/Time 29-Oct-2018 01:03	



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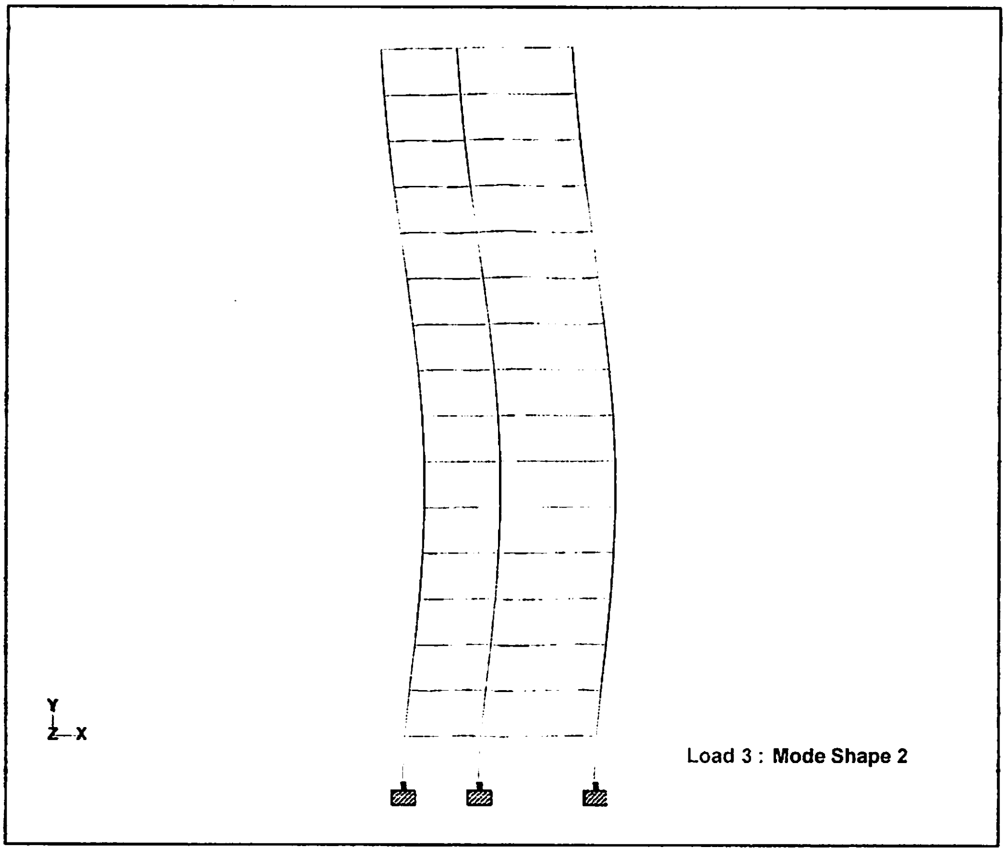
	Ref		
	By Eko Budi S	Date 06-Jul-15	Chd
Client	File akang eko.std	Date/Time 29-Oct-2018 01:03	



Whole Structure

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Job No	Sheet No 1		Rev
Part			
Ref			
By Eko Budi S	Date 06-Jul-15	Chd	
File pusat masa ATAP.std	Date/Time 06-Jul-2015 14:23		

Job Title Skripsi

Client

Job Information

	Engineer	Checked	Approved
Name:	Eko Budi S		
Date:	06-Jul-15		

Structure Type	SPACE FRAME

Number of Nodes	1050	Highest Node	1185
Number of Elements	569	Highest Beam	1662
Number of Plates	960	Highest Plate	1664

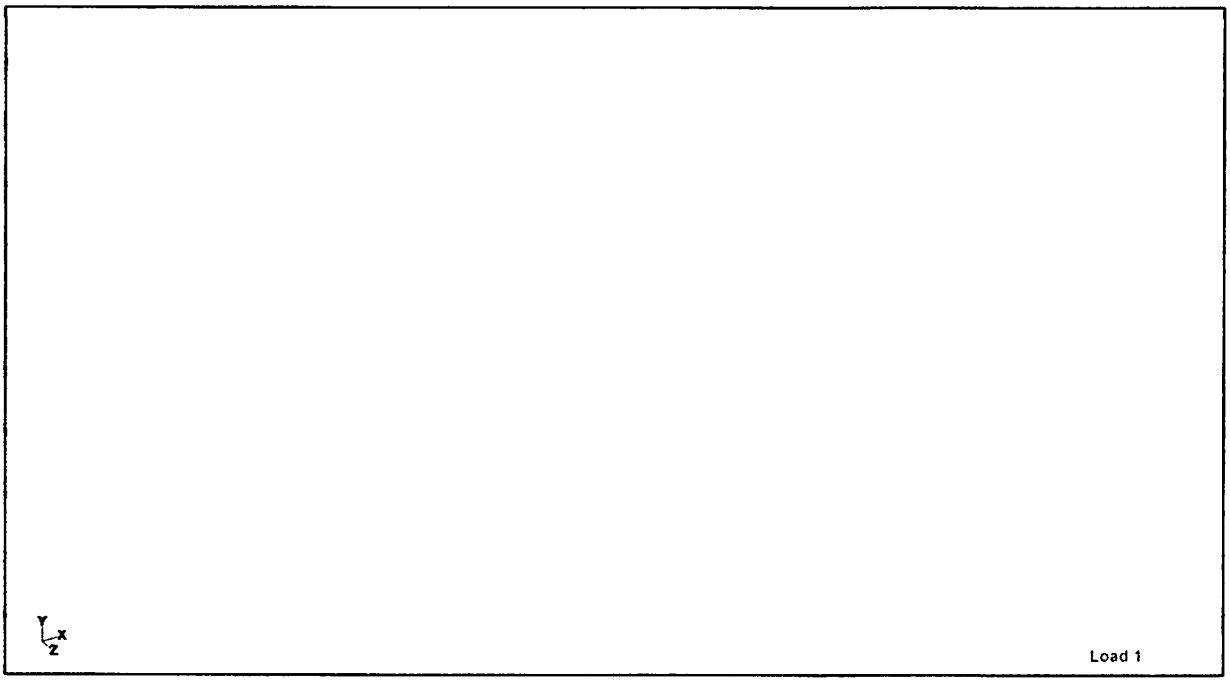
Number of Basic Load Cases	1
Number of Combination Load Cases	0

Included in this printout are data for:

All	The Whole Structure

Included in this printout are results for load cases:

Type	L/C	Name
Primary	1	BEBAN MATI



Whole Structure

Node Displacement Summary

	Node	L/C	X (mm)	Y (mm)	Z (mm)	Resultant (mm)	rX (rad)	rY (rad)	rZ (rad)
Max X	1	1:BEBAN MATI	**129.331**	-201E 3	-632.209	201E 3	-4.971	0.000	-2.620
Min X	157	1:BEBAN MATI	**-5.11E 3**	-201E 3	9.31E 3	201E 3	-4.971	0.000	-2.620
Max Y	229	1:BEBAN MATI	0.000	**0.000**	0.000	0.000	0.000	0.000	0.000
Min Y	6	1:BEBAN MATI	129.331	**-244E 3**	-632.209	244E 3	-4.978	0.000	-2.615
Max Z	162	1:BEBAN MATI	-5.1E 3	-244E 3	**9.32E 3**	245E 3	-4.978	0.000	-2.615
Min Z	1	1:BEBAN MATI	129.331	-201E 3	**-632.209**	201E 3	-4.971	0.000	-2.620
Max rX	229	1:BEBAN MATI	0.000	0.000	0.000	0.000	**0.000**	0.000	0.000
Min rX	1009	1:BEBAN MATI	129.331	-239E 3	-632.209	239E 3	**-4.978**	0.000	-2.614
Max rY	451	1:BEBAN MATI	129.331	-166.095	-632.209	666.335	-1.099	**0.000**	-0.270
Min rY	229	1:BEBAN MATI	0.000	0.000	0.000	0.000	0.000	**0.000**	0.000
Max rZ	229	1:BEBAN MATI	0.000	0.000	0.000	0.000	0.000	0.000	**0.000**
Min rZ	1	1:BEBAN MATI	129.331	-201E 3	-632.209	201E 3	-4.971	0.000	**-2.620**
Max Rst	162	1:BEBAN MATI	-5.1E 3	-244E 3	9.32E 3	**245E 3**	-4.978	0.000	-2.615

Beam End Displacement Summary

Displacements shown in italic indicate the presence of an offset

	Beam	Node	L/C	X (mm)	Y (mm)	Z (mm)	Resultant (mm)
Max X	3	3	1:BEBAN MATI	**129.331**	-218E 3	-632.209	218E 3
Min X	216	157	1:BEBAN MATI	**-5.11E 3**	-201E 3	9.31E 3	201E 3
Max Y	288	229	1:BEBAN MATI	0.000	**0.000**	0.000	0.000
Min Y	11	6	1:BEBAN MATI	129.330	**-244E 3**	-632.209	244E 3
Max Z	221	162	1:BEBAN MATI	-5.1E 3	-244E 3	**9.32E 3**	245E 3
Min Z	45	25	1:BEBAN MATI	129.331	-122E 3	**-632.209**	122E 3
Max Rst	221	162	1:BEBAN MATI	-5.1E 3	-244E 3	9.32E 3	**245E 3**

Beam End Force Summary

The signs of the forces at end B of each beam have been reversed. For example: this means that the Min Fx entry gives the largest tension value for an beam.

				Axial	Shear		Torsion	Bending	
	Beam	Node	L/C	Fx (kg)	Fy (kg)	Fz (kg)	Mx (kNm)	My (kNm)	Mz (kNm)
Max Fx	288	229	1:BEBAN MATI	**567E 3**	0.000	-0.000	-0.000	-134E 3	-46.7E 3
Min Fx	218	3	1:BEBAN MATI	**-4.71E 3**	-0.000	-0.000	-0.000	0.000	-0.000
Max Fy	588	450	1:BEBAN MATI	-0.000	**434E 3**	-0.000	-15.4E 3	0.000	20.6E 3
Min Fy	584	73	1:BEBAN MATI	-0.000	**-1.12E 6**	0.000	-22.2E 3	0.000	93.9E 3
Max Fz	133	73	1:BEBAN MATI	-0.000	-578E 3	**0.000**	38.3E 3	-0.000	23.4E 3
Min Fz	288	73	1:BEBAN MATI	565E 3	0.000	**-0.000**	-0.000	-134E 3	-46.7E 3
Max Mx	133	73	1:BEBAN MATI	-0.000	-578E 3	0.000	**38.3E 3**	-0.000	23.4E 3
Min Mx	584	447	1:BEBAN MATI	-0.000	-1.12E 6	0.000	**-22.2E 3**	-0.000	83E 3
Max My	233	18	1:BEBAN MATI	-3.03E 3	-0.000	-0.000	-0.000	**0.000**	0.000
Min My	288	73	1:BEBAN MATI	565E 3	0.000	-0.000	-0.000	**-134E 3**	-46.7E 3

Beam End Force Summary Cont...

				Axial	Shear		Torsion	Bending	
	Beam	Node	L/C	Fx (kg)	Fy (kg)	Fz (kg)	Mx (kNm)	My (kNm)	Mz (kNm)
Max Mz	584	73	1:BEBAN MATI	-0.000	-1.12E 6	0.000	-22.2E 3	0.000	93.9E 3
Min Mz	288	73	1:BEBAN MATI	565E 3	0.000	-0.000	-0.000	-134E 3	-46.7E 3

Beam Force Detail Summary

Sign convention as diagrams:- positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	Shear		Torsion	Bending	
	Beam	L/C	d (m)	Fx (kg)	Fy (kg)	Fz (kg)	Mx (kNm)	My (kNm)	Mz (kNm)
Max Fx	288	1:BEBAN MATI	2.000	567E 3	0.000	-0.000	-0.000	-134E 3	-46.7E 3
Min Fx	218	1:BEBAN MATI	0.000	-4.71E 3	-0.000	-0.000	-0.000	0.000	-0.000
Max Fy	588	1:BEBAN MATI	0.000	-0.000	434E 3	-0.000	-15.4E 3	0.000	20.6E 3
Min Fy	584	1:BEBAN MATI	1.000	-0.000	-1.12E 6	0.000	-22.2E 3	0.000	93.9E 3
Max Fz	133	1:BEBAN MATI	0.000	-0.000	-578E 3	0.000	38.3E 3	-0.000	23.4E 3
Min Fz	288	1:BEBAN MATI	0.000	565E 3	0.000	-0.000	-0.000	-134E 3	-46.7E 3
Max Mx	133	1:BEBAN MATI	0.000	-0.000	-578E 3	0.000	38.3E 3	-0.000	23.4E 3
Min Mx	584	1:BEBAN MATI	0.000	-0.000	-1.12E 6	0.000	-22.2E 3	-0.000	83E 3
Max My	233	1:BEBAN MATI	0.000	-3.03E 3	-0.000	-0.000	-0.000	0.000	0.000
Min My	288	1:BEBAN MATI	0.000	565E 3	0.000	-0.000	-0.000	-134E 3	-46.7E 3
Max Mz	584	1:BEBAN MATI	1.000	-0.000	-1.12E 6	0.000	-22.2E 3	0.000	93.9E 3
Min Mz	288	1:BEBAN MATI	0.000	565E 3	0.000	-0.000	-0.000	-134E 3	-46.7E 3

Reaction Summary

			Horizontal	Vertical	Horizontal	Moment		
	Node	L/C	FX (kg)	FY (kg)	FZ (kg)	MX (kNm)	MY (kNm)	MZ (kNm)
Max FX	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Min FX	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Max FY	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Min FY	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Max FZ	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Min FZ	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Max MX	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Min MX	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Max MY	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Min MY	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Max MZ	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3
Min MZ	229	1:BEBAN MATI	-0.000	567E 3	0.000	134E 3	-0.000	46.7E 3

Job Information

	Engineer	Checked	Approved
Name:	Eko Budi S		
Date:	06-Jul-15		

Structure Type	SPACE FRAME

Number of Nodes	1071	Highest Node	1185
Number of Elements	590	Highest Beam	1662
Number of Plates	960	Highest Plate	1664

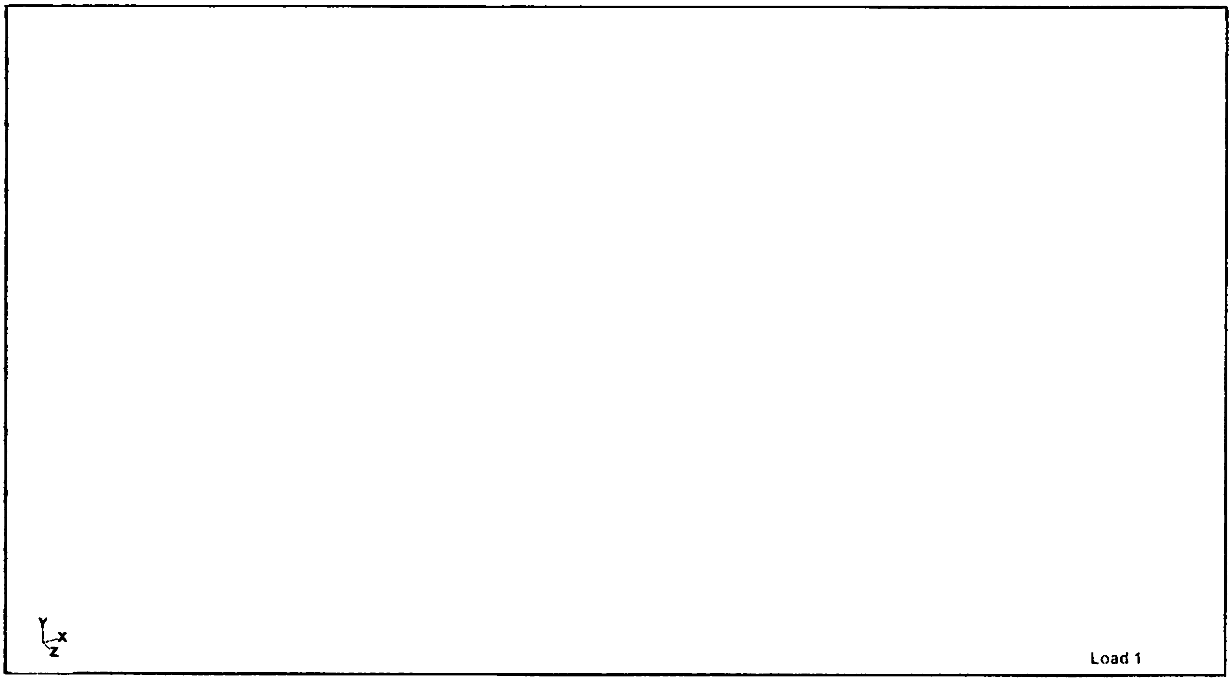
Number of Basic Load Cases	1
Number of Combination Load Cases	0

Included in this printout are data for:

All	The Whole Structure

Included in this printout are results for load cases:

Type	L/C	Name
Primary	1	BEBAN MATI



Whole Structure

Node Displacement Summary

	Node	L/C	X (mm)	Y (mm)	Z (mm)	Resultant (mm)	rX (rad)	rY (rad)	rZ (rad)
Max X	79	1:BEBAN MATI	**6.01E 3**	-227E 3	-12E 3	227E 3	-5.620	-0.000	-2.932
Min X	157	1:BEBAN MATI	**-5.72E 3**	-227E 3	10.5E 3	227E 3	-5.620	-0.000	-2.932
Max Y	229	1:BEBAN MATI	0.000	**0.000**	0.000	0.000	0.000	0.000	0.000
Min Y	6	1:BEBAN MATI	144.748	**-276E 3**	-713.253	276E 3	-5.625	-0.000	-2.927
Max Z	162	1:BEBAN MATI	-5.71E 3	-276E 3	**10.5E 3**	276E 3	-5.625	-0.000	-2.927
Min Z	84	1:BEBAN MATI	6E 3	-276E 3	**-12E 3**	276E 3	-5.625	-0.000	-2.927
Max rX	229	1:BEBAN MATI	0.000	0.000	0.000	0.000	**0.000**	0.000	0.000
Min rX	1009	1:BEBAN MATI	144.748	-270E 3	-713.253	270E 3	**-5.626**	-0.000	-2.927
Max rY	229	1:BEBAN MATI	0.000	0.000	0.000	0.000	0.000	**0.000**	0.000
Min rY	79	1:BEBAN MATI	6.01E 3	-227E 3	-12E 3	227E 3	-5.620	**-0.000**	-2.932
Max rZ	229	1:BEBAN MATI	0.000	0.000	0.000	0.000	0.000	0.000	**0.000**
Min rZ	1	1:BEBAN MATI	144.748	-227E 3	-713.253	227E 3	-5.620	-0.000	**-2.932**
Max Rst	84	1:BEBAN MATI	6E 3	-276E 3	-12E 3	**276E 3**	-5.625	-0.000	-2.927

Beam End Displacement Summary

Displacements shown in italic indicate the presence of an offset

	Beam	Node	L/C	X (mm)	Y (mm)	Z (mm)	Resultant (mm)
Max X	138	79	1:BEBAN MATI	**6.01E 3**	-227E 3	-12E 3	227E 3
Min X	216	157	1:BEBAN MATI	**-5.72E 3**	-227E 3	10.5E 3	227E 3
Max Y	288	229	1:BEBAN MATI	0.000	**0.000**	0.000	0.000
Min Y	11	6	1:BEBAN MATI	144.748	**-276E 3**	-713.253	276E 3
Max Z	221	162	1:BEBAN MATI	-5.71E 3	-276E 3	**10.5E 3**	276E 3
Min Z	143	84	1:BEBAN MATI	6E 3	-276E 3	**-12E 3**	276E 3
Max Rst	143	84	1:BEBAN MATI	6E 3	-276E 3	-12E 3	**276E 3**

Beam End Force Summary

The signs of the forces at end B of each beam have been reversed. For example: this means that the Min Fx entry gives the largest tension value for an beam.

				Axial	Shear		Torsion	Bending	
	Beam	Node	L/C	Fx (kg)	Fy (kg)	Fz (kg)	Mx (kNm)	My (kNm)	Mz (kNm)
Max Fx	288	229	1:BEBAN MATI	**640E 3**	0.000	-0.000	0.000	-151E 3	-52.3E 3
Min Fx	218	3	1:BEBAN MATI	**-4.71E 3**	-0.000	-0.000	-0.000	0.000	0.000
Max Fy	588	450	1:BEBAN MATI	0.000	**491E 3**	0.000	-17.3E 3	-0.000	23.3E 3
Min Fy	584	73	1:BEBAN MATI	-0.000	**-1.26E 6**	-0.000	-24.9E 3	-0.000	106E 3
Max Fz	233	18	1:BEBAN MATI	-3.03E 3	0.000	**0.000**	0.000	-0.000	0.000
Min Fz	133	73	1:BEBAN MATI	0.000	-656E 3	**-0.000**	43.1E 3	0.000	26.1E 3
Max Mx	133	73	1:BEBAN MATI	0.000	-656E 3	-0.000	**43.1E 3**	0.000	26.1E 3
Min Mx	584	447	1:BEBAN MATI	-0.000	-1.26E 6	-0.000	**-24.9E 3**	0.000	93.6E 3
Max My	133	73	1:BEBAN MATI	0.000	-656E 3	-0.000	43.1E 3	**0.000**	26.1E 3
Min My	288	73	1:BEBAN MATI	638E 3	0.000	-0.000	0.000	**-151E 3**	-52.3E 3

Beam End Force Summary Cont...

				Axial	Shear		Torsion	Bending	
	Beam	Node	L/C	Fx (kg)	Fy (kg)	Fz (kg)	Mx (kNm)	My (kNm)	Mz (kNm)
Max Mz	584	73	1:BEBAN MATI	-0.000	-1.26E 6	-0.000	-24.9E 3	-0.000	**106E 3**
Min Mz	288	73	1:BEBAN MATI	638E 3	0.000	-0.000	0.000	-151E 3	**-52.3E 3**

Beam Force Detail Summary

Sign convention as diagrams:- positive above line, negative below line except Fx where positive is compression. Distance d is given from beam end A.

				Axial	Shear		Torsion	Bending	
	Beam	L/C	d (m)	Fx (kg)	Fy (kg)	Fz (kg)	Mx (kNm)	My (kNm)	Mz (kNm)
Max Fx	288	1:BEBAN MATI	2.000	**640E 3**	0.000	-0.000	0.000	-151E 3	-52.3E 3
Min Fx	218	1:BEBAN MATI	0.000	**-4.71E 3**	-0.000	-0.000	-0.000	0.000	0.000
Max Fy	588	1:BEBAN MATI	0.000	0.000	**491E 3**	0.000	-17.3E 3	-0.000	23.3E 3
Min Fy	584	1:BEBAN MATI	1.000	-0.000	**-1.26E 6**	-0.000	-24.9E 3	-0.000	106E 3
Max Fz	233	1:BEBAN MATI	0.000	-3.03E 3	0.000	**0.000**	0.000	-0.000	0.000
Min Fz	133	1:BEBAN MATI	0.000	0.000	-656E 3	**-0.000**	43.1E 3	0.000	26.1E 3
Max Mx	133	1:BEBAN MATI	0.000	0.000	-656E 3	-0.000	**43.1E 3**	0.000	26.1E 3
Min Mx	584	1:BEBAN MATI	0.000	-0.000	-1.26E 6	-0.000	**-24.9E 3**	0.000	93.6E 3
Max My	133	1:BEBAN MATI	0.000	0.000	-656E 3	-0.000	43.1E 3	**0.000**	26.1E 3
Min My	288	1:BEBAN MATI	0.000	638E 3	0.000	-0.000	0.000	**-151E 3**	-52.3E 3
Max Mz	584	1:BEBAN MATI	1.000	-0.000	-1.26E 6	-0.000	-24.9E 3	-0.000	**106E 3**
Min Mz	288	1:BEBAN MATI	0.000	638E 3	0.000	-0.000	0.000	-151E 3	**-52.3E 3**

Reaction Summary

			Horizontal	Vertical	Horizontal	Moment		
	Node	L/C	FX (kg)	FY (kg)	FZ (kg)	MX (kNm)	MY (kNm)	MZ (kNm)
Max FX	229	1:BEBAN MATI	**-0.000**	640E 3	0.000	151E 3	0.000	52.3E 3
Min FX	229	1:BEBAN MATI	**-0.000**	640E 3	0.000	151E 3	0.000	52.3E 3
Max FY	229	1:BEBAN MATI	-0.000	**640E 3**	0.000	151E 3	0.000	52.3E 3
Min FY	229	1:BEBAN MATI	-0.000	**640E 3**	0.000	151E 3	0.000	52.3E 3
Max FZ	229	1:BEBAN MATI	-0.000	640E 3	**0.000**	151E 3	0.000	52.3E 3
Min FZ	229	1:BEBAN MATI	-0.000	640E 3	**0.000**	151E 3	0.000	52.3E 3
Max MX	229	1:BEBAN MATI	-0.000	640E 3	0.000	**151E 3**	0.000	52.3E 3
Min MX	229	1:BEBAN MATI	-0.000	640E 3	0.000	**151E 3**	0.000	52.3E 3
Max MY	229	1:BEBAN MATI	-0.000	640E 3	0.000	151E 3	**0.000**	52.3E 3
Min MY	229	1:BEBAN MATI	-0.000	640E 3	0.000	151E 3	**0.000**	52.3E 3
Max MZ	229	1:BEBAN MATI	-0.000	640E 3	0.000	151E 3	0.000	**52.3E 3**
Min MZ	229	1:BEBAN MATI	-0.000	640E 3	0.000	151E 3	0.000	**52.3E 3**

Job Information

	Engineer	Checked	Approved
Name:			
Date:	06-Jul-15		

Structure Type	SPACE FRAME

Number of Nodes	16501	Highest Node	16746
Number of Elements	9120	Highest Beam	25022
Number of Plates	15360	Highest Plate	24974

Number of Basic Load Cases	4
Number of Combination Load Cases	18

Included in this printout are data for:

All	The Whole Structure

Job Information

	Engineer	Checked	Approved
Name:			
Date:	06-Jul-15		

Structure Type	SPACE FRAME

Number of Nodes	1050	Highest Node	1185
Number of Elements	569	Highest Beam	1662
Number of Plates	960	Highest Plate	1664

Number of Basic Load Cases	1
Number of Combination Load Cases	0

Included in this printout are data for:

All	The Whole Structure

Job Information

	Engineer	Checked	Approved
Name:			
Date:	06-Jul-15		

Structure Type	SPACE FRAME

Number of Nodes	1071	Highest Node	1185
Number of Elements	590	Highest Beam	1662
Number of Plates	960	Highest Plate	1664

Number of Basic Load Cases	1
Number of Combination Load Cases	0

Included in this printout are data for:

All	The Whole Structure

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23253 TO 23256 23258 TO 23261 23263 23265 23267 23269 TO 23273 23275 TO 23278 -
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MEMBER PROPERTY AMERICAN
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528 531 671 674 677 680 683 686 689 829 832 835 838 841 844 847 987 990 993 -
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218 TO 220 240 TO 242 262 TO 264 284 TO 286 306 TO 308 328 TO 330 350 TO 352 -
376 TO 378 398 TO 400 420 TO 422 442 TO 444 464 TO 466 486 TO 488 -
508 TO 510 534 TO 536 556 TO 558 578 TO 580 600 TO 602 622 TO 624 -
644 TO 646 666 TO 668 692 TO 694 714 TO 716 736 TO 738 758 TO 760 -
780 TO 782 802 TO 804 824 TO 826 850 TO 852 872 TO 874 894 TO 896 -
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SUPPORTS
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SELFWEIGHT Y -1
MEMBER LOAD
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278 281 TO 287 289 292 298 300 303 TO 309 311 314 320 322 325 TO 331 333 -
336 342 344 347 TO 352 374 TO 379 381 384 390 392 395 TO 401 403 406 412 -
414 417 TO 423 425 428 434 436 439 TO 445 447 450 456 458 461 TO 467 469 -
472 478 480 483 TO 489 491 494 500 502 505 TO 510 532 TO 537 539 542 548 -
550 553 TO 559 561 564 570 572 575 TO 581 583 586 592 594 597 TO 603 605 -
608 614 616 619 TO 625 627 630 636 638 641 TO 647 649 652 658 660 -
663 TO 668 690 TO 695 697 700 706 708 711 TO 717 719 722 728 730 733 TO 739 -
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1182 1185 TO 1190 UNI GY -875
1191 1193 1196 1202 1204 1207 TO 1213 1215 1218 1224 1226 1229 TO 1235 1237 -
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614. 23653 23655 23656 23658 TO 23660 23662 23664 TO 23666 23668 23670 TO 23672 -
615. 23674 23676 23678 23680 23681 23683 TO 23685 23687 23689 TO 23691 23693 -
616. 23695 TO 23697 23699 23701 23703 23705 23706 23708 TO 23710 23712 -
617. 23714 TO 23716 23718 23720 TO 23722 23724 23726 23728 23730 23731 -
618. 23733 TO 23735 23737 23739 TO 23741 23743 23745 TO 23747 23749 23751 23753 -
619. 23755 23756 23758 TO 23760 PR GY -250
620. 23762 23764 TO 23766 23768 23770 TO 23772 23774 23776 23778 23780 23781 23783 -
621. 23784 TO 23785 23787 23789 TO 23791 23793 23795 TO 23797 23799 23801 23803 -
622. 23805 23806 23808 TO 23810 23812 23814 TO 23816 23818 23820 TO 23822 23824 -
623. 23826 23828 23830 23831 23833 TO 23835 23837 23839 TO 23841 23843 -
624. 23845 TO 23847 23849 23851 23853 23855 23856 23858 TO 23860 23862 -
625. 23864 TO 23866 23868 23870 TO 23872 23874 23876 23878 23880 23881 -
626. 23883 TO 23885 23887 23889 TO 23891 23893 23895 TO 23897 23899 23901 23903 -
627. 23905 23906 23908 23910 23912 23914 TO 23917 23919 TO 23922 23924 23926 -
628. 23928 23930 TO 23934 23936 TO 23939 23941 TO 23944 23946 23948 23950 23952 -
629. 23953 TO 23956 23958 TO 23961 23963 TO 23966 23968 23970 23972 23974 TO 23978 -
630. 23980 TO 23983 23985 TO 23988 23990 23992 23994 23996 TO 24000 -
631. 24002 TO 24005 24007 TO 24010 24012 24014 24016 24018 TO 24022 -
632. 24024 TO 24027 24029 TO 24032 24034 24036 24038 24040 TO 24044 -
633. 24046 TO 24049 24051 TO 24054 24056 24058 24060 24062 TO 24066 -
634. 24068 TO 24071 24073 TO 24076 24078 24080 24082 24084 TO 24088 -
635. 24090 TO 24093 24095 TO 24098 24100 24102 24104 24106 TO 24110 -
636. 24112 TO 24115 24117 TO 24120 24122 24124 24126 24128 TO 24132 -
637. 24134 TO 24137 24139 TO 24142 24144 24146 24148 24150 TO 24154 -
638. 24156 TO 24159 24161 TO 24164 24166 24168 24170 24172 24173 24175 24177 -
639. 24179 24181 TO 24184 24186 TO 24189 24191 24193 24195 24197 TO 24201 24203 -
640. 24204 TO 24206 24208 TO 24211 24213 24215 24217 24219 TO 24223 24225 TO 24228 -
641. 24230 TO 24233 24235 24237 24239 24241 TO 24245 24247 PR GY -250
642. 24248 TO 24250 24252 TO 24255 24257 24259 24261 24263 TO 24267 24269 TO 24272 -
643. 24274 TO 24277 24279 24281 24283 24285 TO 24289 24291 TO 24294 -
644. 24296 TO 24299 24301 24303 24305 24307 TO 24311 24313 TO 24316 -
645. 24318 TO 24321 24323 24325 24327 24329 TO 24333 24335 TO 24338 -
646. 24340 TO 24343 24345 24347 24349 24351 TO 24355 24357 TO 24360 -
647. 24362 TO 24365 24367 24369 24371 24373 TO 24377 24379 TO 24382 -
648. 24384 TO 24387 24389 24391 24393 24395 TO 24399 24401 TO 24404 -
649. 24406 TO 24409 24411 24413 24415 24417 TO 24421 24423 TO 24426 -
650. 24428 TO 24431 24433 24435 24437 24439 24440 24442 24444 24446 -
651. 24448 TO 24451 24453 TO 24456 24458 24460 24462 24464 TO 24468 -
652. 24470 TO 24473 24475 TO 24478 24480 24482 24484 24486 TO 24490 -
653. 24492 TO 24495 24497 TO 24500 24502 24504 24506 24508 TO 24512 -
654. 24514 TO 24517 24519 TO 24522 24524 24526 24528 24530 TO 24534 -
655. 24536 TO 24539 24541 TO 24544 24546 24548 24550 24552 TO 24556 -
656. 24558 TO 24561 24563 TO 24566 24568 24570 24572 24574 TO 24578 -
657. 24580 TO 24583 24585 TO 24588 24590 24592 24594 24596 TO 24600 -
658. 24602 TO 24605 24607 TO 24610 24612 24614 24616 24618 TO 24622 -
659. 24624 TO 24627 24629 TO 24632 24634 24636 24638 24640 TO 24644 -
660. 24646 TO 24649 24651 TO 24654 24656 24658 24660 24662 TO 24666 -
661. 24668 TO 24671 24673 TO 24676 24678 24680 24682 24684 TO 24688 -
662. 24690 TO 24693 24695 TO 24698 24700 24702 24704 24706 24707 24709 24711 -
663. 24713 24715 TO 24718 24720 TO 24723 24725 24727 24729 24731 TO 24735 24737 -
664. 24738 TO 24740 24742 TO 24745 24747 24749 24751 24753 TO 24757 PR GY -250

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24759 TO 24762 24764 TO 24767 24769 24771 24773 24775 TO 24779 24781 TO 24784 -
24786 TO 24789 24791 24793 24795 24797 TO 24801 24803 TO 24806 -
24808 TO 24811 24813 24815 24817 24819 TO 24823 24825 TO 24828 -
24830 TO 24833 24835 24837 24839 24841 TO 24845 24847 TO 24850 -
24852 TO 24855 24857 24859 24861 24863 TO 24867 24869 TO 24872 -
24874 TO 24877 24879 24881 24883 24885 TO 24889 24891 TO 24894 -
24896 TO 24899 24901 24903 24905 24907 TO 24911 24913 TO 24916 -
24918 TO 24921 24923 24925 24927 24929 TO 24933 24935 TO 24938 -
24940 TO 24943 24945 24947 24949 24951 TO 24955 24957 TO 24960 -
24962 TO 24965 24967 24969 24971 24973 24974 PR GY -250
LOAD 3 BEBAN GEMPA ARAH X
SPECTRUM SRSS X 1 Z 1 ACC SCALE 1 DAMP 0.05 LIN MIS
0 0.224; 0.111 0.555; 0.56 0.561; 1 0.315
JOINT LOAD
16699 FX 693599
2927 FX 620858
3878 FX 620858
4829 FX 620858
5780 FX 620858
16716 FX 620858
16719 FX 620858
16722 FX 620858
9584 FX 620858
10535 FX 620858
16731 FX 620858
12437 FX 620858
13388 FX 620858
14339 FX 620858
15290 FX 620858
16241 FX 545818
LOAD 4 BEBAN GEMPA ARAH Z
SPECTRUM SRSS X 1 Z 1 ACC SCALE 1 DAMP 0.05 LIN MIS
0 0.224; 0.111 0.555; 0.56 0.561; 1 0.315
JOINT LOAD
16699 FZ 693599
2927 FZ 620858
3878 FZ 620858
4829 FZ 620858
5780 FZ 620858
16716 FZ 620858
16719 FZ 620858
16722 FZ 620858
9584 FZ 620858
10535 FZ 620858
16731 FZ 620858
12437 FZ 620858
13388 FZ 620858
14339 FZ 620858
15290 FZ 620858
16241 FZ 545818
LOAD COMB 5 KOMBINASI 2
1 1.2 2 1.6
LOAD COMB 6 KOMBINASI 1
1 1.4
LOAD COMB 7 KOMBINASI 3
1 1.2 2 1.0 3 1.0 4 0.3
```

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721. LOAD COMB 8 KOMBINASI 4
722. 1 1.2 2 1.0 3 -1.0 4 -0.3
723. LOAD COMB 9 KOMBINASI 5
724. 1 1.2 2 1.0 3 1.0 4 -0.3
725. LOAD COMB 10 KOMBINASI 6
726. 1 1.2 2 1.0 3 -1.0 4 0.3
727. LOAD COMB 11 KOMBINASI 7
728. 1 1.2 2 1.0 3 0.3 4 1.0
729. LOAD COMB 12 KOMBINASI 8
730. 1 1.2 2 1.0 3 -0.3 4 -1.0
731. LOAD COMB 13 KOMBINASI 9
732. 1 1.2 2 1.0 3 0.3 4 -1.0
733. LOAD COMB 14 KOMBINASI 10
734. 1 1.2 2 1.0 3 -0.3 4 1.0
735. LOAD COMB 15 KOMBINASI 11
736. 1 0.9 3 0.3 4 1.0
737. LOAD COMB 16 KOMBINASI 12
738. 1 0.9 3 -0.3 4 -1.0
739. LOAD COMB 17 KOMBINASI 13
740. 1 0.9 3 0.3 4 -1.0
741. LOAD COMB 18 KOMBINASI 14
742. 1 0.9 3 -0.3 4 1.0
743. LOAD COMB 19 KOMBINASI 15
744. 1 0.9 3 1.0 4 0.3
745. LOAD COMB 20 KOMBINASI 16
746. 1 0.9 3 -1.0 4 -0.3
747. LOAD COMB 21 KOMBINASI 17
748. 1 0.9 3 1.0 4 -0.3
749. LOAD COMB 22 KOMBINASI 18
750. 1 0.9 3 -1.0 4 0.3
751. PERFORM ANALYSIS


        P R O B L E M   S T A T I S T I C S
        -----------------------------------

 NUMBER OF JOINTS/MEMBER+ELEMENTS/SUPPORTS = 16501/ 24480/    21
 ORIGINAL/FINAL BAND-WIDTH= 15288/   400/   2406 DOF
 TOTAL PRIMARY LOAD CASES =     4, TOTAL DEGREES OF FREEDOM =  98880
 SIZE OF STIFFNESS MATRIX =  237906 DOUBLE  KILO-WORDS
 REQRD/AVAIL. DISK SPACE  =  3738.4/ 27781.3 MB,  EXMEM = 1775.9 MB


IBER OF MODES REQUESTED              =      6
IBER OF EXISTING MASSES IN THE MODEL =     16
IBER OF MODES THAT WILL BE USED      =      6
```

CALCULATED FREQUENCIES FOR LOAD CASE 3

MODE	FREQUENCY(CYCLES/SEC)	PERIOD(SEC)	ACCURACY
1	0.372	2.68934	0.000E+00
2	1.172	0.85335	5.243E-16
3	2.115	0.47282	2.092E-15
4	3.136	0.31887	4.527E-13
5	4.209	0.23761	5.478E-10
6	5.259	0.19015	5.300E-08

The following Frequencies are estimates that were calculated. These are for information only and will not be used. Remaining values are either above the cut off mode/freq values or are of low accuracy. To use these frequencies, rerun with a higher cutoff mode (or mode + freq) value.

CALCULATED FREQUENCIES FOR LOAD CASE 3

MODE	FREQUENCY(CYCLES/SEC)	PERIOD(SEC)	ACCURACY
7	6.256	0.15986	3.685E-06
8	7.186	0.13916	3.462E-05

RESPONSE LOAD CASE 3

SRSS MODAL COMBINATION METHOD USED.
MISSING MASS METHOD USED.

DYNAMIC WEIGHT X Y Z	9.931429E+06	0.000000E+00	0.000000E+00	KG
MISSING WEIGHT X Y Z	-3.309104E+05	0.000000E+00	0.000000E+00	KG
MISSING WEIGHT X Y Z	-3.309104E+05	0.000000E+00	0.000000E+00	KG
MODAL WEIGHT X Y Z	9.600519E+06	0.000000E+00	0.000000E+00	KG

MODE	ACCELERATION-G	DAMPING
1	0.00000	0.05000

**WARNING- ZERO SPECTRAL ACCELERATION ENTERED FOR MODE 1

MODE	ACCELERATION-G	DAMPING
2	0.04048	0.05000
3	0.05709	0.05000
4	0.05688	0.05000
5	0.05677	0.05000
6	0.05670	0.05000
ZPA	0.03183	
ZPAFREQ	33.00000	

MASS PARTICIPATION FACTORS IN PERCENT | BASE SHEAR IN KG

)DE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
1	76.58	0.00	0.00	76.578	0.000	0.000	0.00	0.00	0.00
2	10.75	0.00	0.00	87.327	0.000	0.000	43215.58	0.00	0.00
3	4.08	0.00	0.00	91.402	0.000	0.000	23106.15	0.00	0.00
4	2.41	0.00	0.00	93.809	0.000	0.000	13593.27	0.00	0.00
5	1.64	0.00	0.00	95.446	0.000	0.000	9228.99	0.00	0.00
6	1.22	0.00	0.00	96.668	0.000	0.000	6884.22	0.00	0.00
?A	3.33	0.00	100.00	100.000	0.000	100.000	10533.90	0.00	0.00
					TOTAL SRSS	SHEAR	53195.74	0.00	0.00
					TOTAL 10PCT	SHEAR	53195.74	0.00	0.00
					TOTAL ABS	SHEAR	106562.10	0.00	0.00

RESPONSE LOAD CASE 4

SRSS MODAL COMBINATION METHOD USED.
MISSING MASS METHOD USED.

DYNAMIC WEIGHT X Y Z	9.931429E+06	0.000000E+00	0.000000E+00	KG
MISSING WEIGHT X Y Z	-3.309104E+05	0.000000E+00	0.000000E+00	KG
MISSING WEIGHT X Y Z	-3.309104E+05	0.000000E+00	0.000000E+00	KG
MODAL WEIGHT X Y Z	9.600519E+06	0.000000E+00	0.000000E+00	KG

MODE	ACCELERATION-G	DAMPING
1	0.00000	0.05000

**WARNING- ZERO SPECTRAL ACCELERATION ENTERED FOR MODE 1

MODE	ACCELERATION-G	DAMPING
2	0.04048	0.05000
3	0.05709	0.05000
4	0.05688	0.05000
5	0.05677	0.05000
6	0.05670	0.05000
ZPA	0.03183	
ZPAFREQ	33.00000	

MASS PARTICIPATION FACTORS IN PERCENT / BASE SHEAR IN KG

ODE	X	Y	Z	SUMM-X	SUMM-Y	SUMM-Z	X	Y	Z
1	76.58	0.00	0.00	76.578	0.000	0.000	0.00	0.00	0.00
2	10.75	0.00	0.00	87.327	0.000	0.000	43215.58	0.00	0.00
3	4.08	0.00	0.00	91.402	0.000	0.000	23106.15	0.00	0.00
4	2.41	0.00	0.00	93.809	0.000	0.000	13593.27	0.00	0.00
5	1.64	0.00	0.00	95.446	0.000	0.000	9228.99	0.00	0.00
6	1.22	0.00	0.00	96.668	0.000	0.000	6884.22	0.00	0.00
PA	3.33	0.00	100.00	100.000	0.000	100.000	10533.90	0.00	0.00
				TOTAL SRSS		SHEAR	53195.74	0.00	0.00
				TOTAL 10PCT		SHEAR	53195.74	0.00	0.00
				TOTAL ABS		SHEAR	106562.10	0.00	0.00

752. PRINT STORY DRIFT

STORY	HEIGHT (METE)	LOAD	DRIFT(CM) X	Z	ECCENTRICITY (METE)	RATIO
\SE=	0.00					
1	4.00	1	0.0034	0.0000	0.0000	L /118448
		2	0.0013	0.0000	0.0000	L / 0
		3	0.0413	0.0007	0.0000	L / 9686
		4	0.0413	0.0007	0.0000	L / 9686
		5	0.0061	0.0000	0.0000	L / 65914
		6	0.0047	0.0000	0.0000	L / 84606
		7	0.0590	0.0009	0.0000	L / 6780
		8	-0.0484	-0.0009	0.0000	L / 8269
		9	0.0342	0.0005	0.0000	L / 11689
		10	-0.0236	-0.0005	0.0000	L / 16952
		11	0.0590	0.0009	0.0000	L / 6780
		12	-0.0484	-0.0009	0.0000	L / 8269
		13	-0.0236	-0.0005	0.0000	L / 16952
		14	0.0342	0.0005	0.0000	L / 11689
		15	0.0567	0.0009	0.0000	L / 7051
		16	-0.0506	-0.0009	0.0000	L / 7897
		17	-0.0259	-0.0005	0.0000	L / 15462
		18	0.0319	0.0005	0.0000	L / 12520
		19	0.0567	0.0009	0.0000	L / 7051
		20	-0.0506	-0.0009	0.0000	L / 7897
		21	0.0319	0.0005	0.0000	L / 12520
		22	-0.0259	-0.0005	0.0000	L / 15462
2	8.00	1	0.0130	0.0000	0.0000	L / 61563
		2	0.0048	0.0000	0.0000	L /167886
		3	0.1215	0.0011	0.0000	L / 6584
		4	0.1215	0.0011	0.0000	L / 6584
		5	0.0232	0.0000	0.0000	L / 34456
		6	0.0182	0.0000	0.0000	L / 43974
		7	0.1783	0.0015	0.0000	L / 4486
		8	-0.1376	-0.0015	0.0000	L / 5814
		9	0.1054	0.0008	0.0000	L / 7589
		10	-0.0647	-0.0008	0.0000	L / 12365
		11	0.1783	0.0015	0.0000	L / 4486
		12	-0.1376	-0.0015	0.0000	L / 5814
		13	-0.0647	-0.0008	0.0000	L / 12365
		14	0.1054	0.0008	0.0000	L / 7589
		15	0.1697	0.0015	0.0000	L / 4715
		16	-0.1463	-0.0015	0.0000	L / 5469
		17	-0.0734	-0.0008	0.0000	L / 10905
		18	0.0968	0.0008	0.0000	L / 8268
		19	0.1697	0.0015	0.0000	L / 4715
		20	-0.1463	-0.0015	0.0000	L / 5469
		21	0.0968	0.0008	0.0000	L / 8268
		22	-0.0734	-0.0008	0.0000	L / 10905
3	12.00	1	0.0296	0.0000	0.0000	L / 40503

STORY	HEIGHT (METE)	LOAD	DRIFT(CM) X	Z	ECCENTRICITY (METE)	RATIO
\SE=						
		2	0.0106	0.0000	0.0000	L /112843
		3	0.2054	0.0014	0.0000	L / 5843
		4	0.2054	0.0014	0.0000	L / 5843
		5	0.0526	0.0000	0.0000	L / 22828
		6	0.0415	0.0000	0.0000	L / 28931
		7	0.3132	0.0018	0.0000	L / 3832
		8	-0.2208	-0.0018	0.0000	L / 5435
		9	0.1899	0.0010	0.0000	L / 6318
		10	-0.0976	-0.0010	0.0000	L / 12299
		11	0.3132	0.0018	0.0000	L / 3832
		12	-0.2208	-0.0018	0.0000	L / 5435
		13	-0.0976	-0.0010	0.0000	L / 12299
		14	0.1899	0.0010	0.0000	L / 6318
		15	0.2936	0.0018	0.0000	L / 4087
		16	-0.2403	-0.0018	0.0000	L / 4993
		17	-0.1171	-0.0010	0.0000	L / 10248
		18	0.1704	0.0010	0.0000	L / 7041
		19	0.2936	0.0018	0.0000	L / 4087
		20	-0.2403	-0.0018	0.0000	L / 4993
		21	0.1704	0.0010	0.0000	L / 7041
		22	-0.1171	-0.0010	0.0000	L / 10248
4	16.00	1	0.0525	0.0000	0.0000	L / 30448
		2	0.0186	0.0000	0.0000	L / 86205
		3	0.2744	0.0015	0.0000	L / 5831
		4	0.2744	0.0015	0.0000	L / 5831
		5	0.0928	0.0000	0.0000	L / 17250
		6	0.0736	0.0000	0.0000	L / 21748
		7	0.4383	0.0019	0.0000	L / 3650
		8	-0.2751	-0.0019	0.0000	L / 5816
		9	0.2737	0.0010	0.0000	L / 5846
		10	-0.1105	-0.0010	0.0000	L / 14485
		11	0.4383	0.0019	0.0000	L / 3650
		12	-0.2751	-0.0019	0.0000	L / 5816
		13	-0.1105	-0.0010	0.0000	L / 14485
		14	0.2737	0.0010	0.0000	L / 5846
		15	0.4040	0.0019	0.0000	L / 3960
		16	-0.3094	-0.0019	0.0000	L / 5171
		17	-0.1448	-0.0010	0.0000	L / 11051
		18	0.2394	0.0010	0.0000	L / 6684
		19	0.4040	0.0019	0.0000	L / 3960
		20	-0.3094	-0.0019	0.0000	L / 5171
		21	0.2394	0.0010	0.0000	L / 6684
		22	-0.1448	-0.0010	0.0000	L / 11051
5	20.00	1	0.0814	0.0000	0.0000	L / 24569
		2	0.0284	0.0000	0.0000	L / 70426
		3	0.3197	0.0015	0.0000	L / 6255
		4	0.3197	0.0015	0.0000	L / 6255

STORY	HEIGHT (METE)	LOAD	DRIFT(CM) X	Z	ECCENTRICITY (METE)	RATIO
ASE=						
		5	0.1431	0.0000	0.0000	L / 13974
		6	0.1140	0.0000	0.0000	L / 17550
		7	0.5417	0.0019	0.0000	L / 3692
		8	-0.2896	-0.0019	0.0000	L / 6906
		9	0.3499	0.0010	0.0000	L / 5716
		10	-0.0977	-0.0010	0.0000	L / 20462
		11	0.5417	0.0019	0.0000	L / 3692
		12	-0.2896	-0.0019	0.0000	L / 6906
		13	-0.0977	-0.0010	0.0000	L / 20462
		14	0.3499	0.0010	0.0000	L / 5716
		15	0.4889	0.0019	0.0000	L / 4090
		16	-0.3424	-0.0019	0.0000	L / 5841
		17	-0.1506	-0.0010	0.0000	L / 13284
		18	0.2971	0.0010	0.0000	L / 6732
		19	0.4889	0.0019	0.0000	L / 4090
		20	-0.3424	-0.0019	0.0000	L / 5841
		21	0.2971	0.0010	0.0000	L / 6732
		22	-0.1506	-0.0010	0.0000	L / 13284
6	24.00	1	0.1157	0.0000	0.0000	L / 20737
		2	0.0400	0.0000	0.0000	L / 60019
		3	0.3381	0.0014	0.0000	L / 7099
		4	0.3381	0.0014	0.0000	L / 7099
		5	0.2029	0.0000	0.0000	L / 11831
		6	0.1620	0.0000	0.0000	L / 14812
		7	0.6183	0.0019	0.0000	L / 3881
		8	-0.2606	-0.0019	0.0000	L / 9209
		9	0.4155	0.0010	0.0000	L / 5776
		10	-0.0578	-0.0010	0.0000	L / 41540
		11	0.6183	0.0019	0.0000	L / 3881
		12	-0.2606	-0.0019	0.0000	L / 9209
		13	-0.0578	-0.0010	0.0000	L / 41540
		14	0.4155	0.0010	0.0000	L / 5776
		15	0.5436	0.0019	0.0000	L / 4415
		16	-0.3353	-0.0019	0.0000	L / 7157
		17	-0.1325	-0.0010	0.0000	L / 18115
		18	0.3408	0.0010	0.0000	L / 7042
		19	0.5436	0.0019	0.0000	L / 4415
		20	-0.3353	-0.0019	0.0000	L / 7157
		21	0.3408	0.0010	0.0000	L / 7042
		22	-0.1325	-0.0010	0.0000	L / 18115
7	28.00	1	0.1551	0.0000	0.0000	L / 18055
		2	0.0532	0.0000	0.0000	L / 52660
		3	0.3293	0.0014	0.0000	L / 8501
		4	0.3293	0.0014	0.0000	L / 8501
		5	0.2712	0.0000	0.0000	L / 10326
		6	0.2171	0.0000	0.0000	L / 12897
		7	0.6674	0.0018	0.0000	L / 4195

STORY	HEIGHT	LOAD	DRIFT(CM)		ECCENTRICITY	RATIO
	(METE)		X	Z	(METE)	
ASE=						
		8	-0.1889	-0.0018	0.0000	L / 14823
		9	0.4698	0.0009	0.0000	L / 5960
		10	0.0087	-0.0009	0.0000	L /321100
		11	0.6674	0.0018	0.0000	L / 4195
		12	-0.1889	-0.0018	0.0000	L / 14823
		13	0.0087	-0.0009	0.0000	L /321100
		14	0.4698	0.0009	0.0000	L / 5960
		15	0.5677	0.0018	0.0000	L / 4932
		16	-0.2886	-0.0018	0.0000	L / 9702
		17	-0.0910	-0.0009	0.0000	L / 30778
		18	0.3701	0.0009	0.0000	L / 7565
		19	0.5677	0.0018	0.0000	L / 4932
		20	-0.2886	-0.0018	0.0000	L / 9702
		21	0.3701	0.0009	0.0000	L / 7565
		22	-0.0910	-0.0009	0.0000	L / 30778
8	32.00	1	0.1990	0.0000	0.0000	L / 16083
		2	0.0678	0.0000	0.0000	L / 47201
		3	0.2957	0.0013	0.0000	L / 10821
		4	0.2957	0.0013	0.0000	L / 10821
		5	0.3472	0.0000	0.0000	L / 9216
		6	0.2785	0.0000	0.0000	L / 11488
		7	0.6910	0.0017	0.0000	L / 4631
		8	-0.0779	-0.0017	0.0000	L / 41088
		9	0.5136	0.0009	0.0000	L / 6231
		10	0.0995	-0.0009	0.0000	L / 32145
		11	0.6910	0.0017	0.0000	L / 4631
		12	-0.0779	-0.0017	0.0000	L / 41088
		13	0.0995	-0.0009	0.0000	L / 32145
		14	0.5136	0.0009	0.0000	L / 6231
		15	0.5635	0.0017	0.0000	L / 5679
		16	-0.2054	-0.0017	0.0000	L / 15582
		17	-0.0279	-0.0009	0.0000	L /114553
		18	0.3861	0.0009	0.0000	L / 8288
		19	0.5635	0.0017	0.0000	L / 5679
		20	-0.2054	-0.0017	0.0000	L / 15582
		21	0.3861	0.0009	0.0000	L / 8288
		22	-0.0279	-0.0009	0.0000	L /114553
9	36.00	1	0.2469	0.0000	0.0000	L / 14579
		2	0.0837	0.0000	0.0000	L / 43007
		3	0.2408	0.0012	0.0000	L / 14951
		4	0.2408	0.0012	0.0000	L / 14951
		5	0.4302	0.0000	0.0000	L / 8367
		6	0.3457	0.0000	0.0000	L / 10414
		7	0.6930	0.0016	0.0000	L / 5194
		8	0.0670	-0.0016	0.0000	L / 53728
		9	0.5486	0.0009	0.0000	L / 6563
		10	0.2115	-0.0009	0.0000	L / 17024

STORY	HEIGHT (METE)	LOAD	DRIFT(CM) X	DRIFT(CM) Z	ECCENTRICITY (METE)	RATIO
ASE=						
		11	0.6930	0.0016	0.0000	L / 5194
		12	0.0670	-0.0016	0.0000	L / 53728
		13	0.2115	-0.0009	0.0000	L / 17024
		14	0.5486	0.0009	0.0000	L / 6563
		15	0.5352	0.0016	0.0000	L / 6726
		16	-0.0908	-0.0016	0.0000	L / 39657
		17	0.0537	-0.0009	0.0000	L / 67056
		18	0.3908	0.0009	0.0000	L / 9212
		19	0.5352	0.0016	0.0000	L / 6726
		20	-0.0908	-0.0016	0.0000	L / 39657
		21	0.3908	0.0009	0.0000	L / 9212
		22	0.0537	-0.0009	0.0000	L / 67056
10	40.00	1	0.2985	0.0000	0.0000	L / 13401
		2	0.1008	0.0000	0.0000	L / 39699
		3	0.1703	0.0012	0.0000	L / 23489
		4	0.1703	0.0012	0.0000	L / 23489
		5	0.5194	0.0000	0.0000	L / 7701
		6	0.4179	0.0000	0.0000	L / 9572
		7	0.6803	0.0015	0.0000	L / 5879
		8	0.2376	-0.0015	0.0000	L / 16837
		9	0.5781	0.0008	0.0000	L / 6919
		10	0.3397	-0.0008	0.0000	L / 11774
		11	0.6803	0.0015	0.0000	L / 5879
		12	0.2376	-0.0015	0.0000	L / 16837
		13	0.3397	-0.0008	0.0000	L / 11774
		14	0.5781	0.0008	0.0000	L / 6919
		15	0.4900	0.0015	0.0000	L / 8163
		16	0.0473	-0.0015	0.0000	L / 84634
		17	0.1494	-0.0008	0.0000	L / 26767
		18	0.3878	0.0008	0.0000	L / 10313
		19	0.4900	0.0015	0.0000	L / 8163
		20	0.0473	-0.0015	0.0000	L / 84634
		21	0.3878	0.0008	0.0000	L / 10313
		22	0.1494	-0.0008	0.0000	L / 26767
11	44.00	1	0.3532	0.0000	0.0000	L / 12457
		2	0.1188	0.0000	0.0000	L / 37036
		3	0.0977	0.0011	0.0000	L / 45015
		4	0.0977	0.0011	0.0000	L / 45015
		5	0.6139	0.0000	0.0000	L / 7167
		6	0.4945	0.0000	0.0000	L / 8898
		7	0.6697	0.0015	0.0000	L / 6570
		8	0.4156	-0.0015	0.0000	L / 10587
		9	0.6111	0.0008	0.0000	L / 7200
		10	0.4742	-0.0008	0.0000	L / 9278
		11	0.6697	0.0015	0.0000	L / 6570
		12	0.4156	-0.0015	0.0000	L / 10587
		13	0.4742	-0.0008	0.0000	L / 9278

STORY	HEIGHT (METE)	LOAD	DRIFT(CM) X	Z	ECCENTRICITY (METE)	RATIO
ASE=						
		14	0.6111	0.0008	0.0000	L / 7200
		15	0.4450	0.0015	0.0000	L / 9889
		16	0.1908	-0.0015	0.0000	L / 23059
		17	0.2495	-0.0008	0.0000	L / 17638
		18	0.3863	0.0008	0.0000	L / 11390
		19	0.4450	0.0015	0.0000	L / 9889
		20	0.1908	-0.0015	0.0000	L / 23059
		21	0.3863	0.0008	0.0000	L / 11390
		22	0.2495	-0.0008	0.0000	L / 17638
12	48.00	1	0.4107	0.0000	0.0000	L / 11689
		2	0.1377	0.0000	0.0000	L / 34855
		3	0.0814	0.0010	0.0000	L / 58967
		4	0.0814	0.0010	0.0000	L / 58967
		5	0.7131	0.0000	0.0000	L / 6731
		6	0.5749	0.0000	0.0000	L / 8349
		7	0.7363	0.0013	0.0000	L / 6519
		8	0.5247	-0.0013	0.0000	L / 9148
		9	0.6875	0.0007	0.0000	L / 6982
		10	0.5735	-0.0007	0.0000	L / 8369
		11	0.7363	0.0013	0.0000	L / 6519
		12	0.5247	-0.0013	0.0000	L / 9148
		13	0.5735	-0.0007	0.0000	L / 8369
		14	0.6875	0.0007	0.0000	L / 6982
		15	0.4754	0.0013	0.0000	L / 10096
		16	0.2638	-0.0013	0.0000	L / 18198
		17	0.3126	-0.0007	0.0000	L / 15355
		18	0.4266	0.0007	0.0000	L / 11252
		19	0.4754	0.0013	0.0000	L / 10096
		20	0.2638	-0.0013	0.0000	L / 18198
		21	0.4266	0.0007	0.0000	L / 11252
		22	0.3126	-0.0007	0.0000	L / 15355
13	52.00	1	0.4705	0.0000	0.0000	L / 11053
		2	0.1574	0.0000	0.0000	L / 33042
		3	0.1521	0.0010	0.0000	L / 34196
		4	0.1521	0.0010	0.0000	L / 34196
		5	0.8164	0.0000	0.0000	L / 6370
		6	0.6587	0.0000	0.0000	L / 7895
		7	0.9196	0.0013	0.0000	L / 5654
		8	0.5243	-0.0013	0.0000	L / 9919
		9	0.8284	0.0007	0.0000	L / 6277
		10	0.6155	-0.0007	0.0000	L / 8448
		11	0.9196	0.0013	0.0000	L / 5654
		12	0.5243	-0.0013	0.0000	L / 9919
		13	0.6155	-0.0007	0.0000	L / 8448
		14	0.8284	0.0007	0.0000	L / 6277
		15	0.6211	0.0013	0.0000	L / 8372
		16	0.2257	-0.0013	0.0000	L / 23035

STORY	HEIGHT (METE)	LOAD	DRIFT(CM) X	Z	ECCENTRICITY (METE)	RATIO
.SE=						
		17	0.3170	-0.0007	0.0000	L / 16405
		18	0.5299	0.0007	0.0000	L / 9814
		19	0.6211	0.0013	0.0000	L / 8372
		20	0.2257	-0.0013	0.0000	L / 23035
		21	0.5299	0.0007	0.0000	L / 9814
		22	0.3170	-0.0007	0.0000	L / 16405
14	56.00	1	0.5324	0.0000	0.0000	L / 10519
		2	0.1777	0.0000	0.0000	L / 31512
		3	0.2375	0.0010	0.0000	L / 23574
		4	0.2375	0.0010	0.0000	L / 23574
		5	0.9232	0.0000	0.0000	L / 6066
		6	0.7453	0.0000	0.0000	L / 7513
		7	1.1254	0.0013	0.0000	L / 4976
		8	0.5078	-0.0013	0.0000	L / 11029
		9	0.9828	0.0007	0.0000	L / 5698
		10	0.6503	-0.0007	0.0000	L / 8611
		11	1.1254	0.0013	0.0000	L / 4976
		12	0.5078	-0.0013	0.0000	L / 11029
		13	0.6503	-0.0007	0.0000	L / 8611
		14	0.9828	0.0007	0.0000	L / 5698
		15	0.7879	0.0013	0.0000	L / 7107
		16	0.1703	-0.0013	0.0000	L / 32876
		17	0.3129	-0.0007	0.0000	L / 17899
		18	0.6454	0.0007	0.0000	L / 8676
		19	0.7879	0.0013	0.0000	L / 7107
		20	0.1703	-0.0013	0.0000	L / 32876
		21	0.6454	0.0007	0.0000	L / 8676
		22	0.3129	-0.0007	0.0000	L / 17899
15	60.00	1	0.5970	0.0000	0.0000	L / 10050
		2	0.1990	0.0000	0.0000	L / 30158
		3	0.3146	0.0013	0.0000	L / 19070
		4	0.3146	0.0013	0.0000	L / 19070
		5	1.0347	0.0000	0.0000	L / 5798
		6	0.8358	0.0000	0.0000	L / 7178
		7	1.3244	0.0016	0.0000	L / 4530
		8	0.5064	-0.0016	0.0000	L / 11849
		9	1.1356	0.0009	0.0000	L / 5283
		10	0.6951	-0.0009	0.0000	L / 8631
		11	1.3244	0.0016	0.0000	L / 4530
		12	0.5064	-0.0016	0.0000	L / 11849
		13	0.6951	-0.0009	0.0000	L / 8631
		14	1.1356	0.0009	0.0000	L / 5283
		15	0.9463	0.0016	0.0000	L / 6340
		16	0.1283	-0.0016	0.0000	L / 46764
		17	0.3171	-0.0009	0.0000	L / 18923
		18	0.7576	0.0009	0.0000	L / 7920
		19	0.9463	0.0016	0.0000	L / 6340

STORY	HEIGHT (METE)	LOAD	DRIFT(CM) X	Z	ECCENTRICITY (METE)	RATIO
IE=						
		20	0.1283	-0.0016	0.0000	L / 46764
		21	0.7576	0.0009	0.0000	L / 7920
		22	0.3171	-0.0009	0.0000	L / 18923
16	64.00	1	0.6632	0.0000	0.0000	L / 9650
		2	0.2208	0.0000	0.0000	L / 28991
		3	0.3771	0.0017	0.0000	L / 16972
		4	0.3771	0.0017	0.0000	L / 16972
		5	1.1490	0.0000	0.0000	L / 5570
		6	0.9285	0.0000	0.0000	L / 6893
		7	1.5068	0.0022	0.0000	L / 4247
		8	0.5264	-0.0022	0.0000	L / 12159
		9	1.2805	0.0012	0.0000	L / 4998
		10	0.7526	-0.0012	0.0000	L / 8504
		11	1.5068	0.0022	0.0000	L / 4247
		12	0.5264	-0.0022	0.0000	L / 12159
		13	0.7526	-0.0012	0.0000	L / 8504
		14	1.2805	0.0012	0.0000	L / 4998
		15	1.0871	0.0022	0.0000	L / 5887
		16	0.1066	-0.0022	0.0000	L / 60013
		17	0.3329	-0.0012	0.0000	L / 19225
		18	0.8608	0.0012	0.0000	L / 7434
		19	1.0871	0.0022	0.0000	L / 5887
		20	0.1066	-0.0022	0.0000	L / 60013
		21	0.8608	0.0012	0.0000	L / 7434
		22	0.3329	-0.0012	0.0000	L / 19225

53. FINISH

```
*********** END OF THE STAAD.Pro RUN ***********

  **** DATE= OCT 25,2018   TIME= 12: 1: 2 ****

************************************************************
*   For questions on STAAD.Pro,                            *
*   Please contact : Research Engineers Ltd.               *
*   E2/4,Block GP, Sector-V,Salt Lake, KOLKATA - 700 091   *
*   India : TEL:(033)2357-3575      FAX:(033)2357-3467     *
*   email : support@calcutta.reiusa.com                    *
*   US    : Ph-(714) 974-2500, Fax-(714) 921-0683          *
************************************************************
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KELENGKAPAN

- **LEMBAR ASISTENSI**
- **LEMBAR NILAI**
- **LEMBAR REVISI**

INSTITUT TEKNOLOGI NASIONAL
Jl. Bendungan Sigura-gura 2
Jl. Raya Karanglo Km. 2
Malang

UJIAN SKRIPSI
PRODI TEKNIK SIPIL S-1

FORM REVISI / PERBAIKAN

BIDANG ______________________

Nama : BKO B. S

NIM : 1021072

Hari / tanggal : Sabtu / 01 / 08 / 2015.

Perbaikan materi Skripsi meliputi :

> Pd Perenc. Kolom : Ø = ... dimana (dlm perhit Ø = 1;9); 075

> Gbr penulangan blm di Revisi

> Revisi yg lama (Seminar) belum

Perbaikan Skripsi harus diselesaikan **selambatnya 14 hari** terhitung sejak pelaksanaan Ujian dilaksanakan. **Bila melebihi** masa 14 hari, maka **tidak dapat diikutkan Yudisium**.

Tugas Akhir telah diperbaiki dan disetujui :

Malang, ______________ 20	Malang, ______________ 20
Dosen Penguji	Dosen Penguji
(______________)	(______________)

NASIONAL
Jl. Bendungan Sigura-gura 2
Jl. Raya Karanglo Km. 2
Malang

SEMINAR HASIL SKRIPSI
PRODI TEKNIK SIPIL S-1

FORM REVISI / PERBAIKAN

BIDANG ____________________

Nama : Eko Budi Susanto

NIM : 10 21 072

Hari / tanggal : Sabtu / 01 / 08 / 2015.

Perbaikan materi Seminar Hasil Tugas Akhir meliputi :

√ > Tul. geser balok cek lagi √

> Tul. pengikat pd kolom betulkan

> Analisa Tul balok sesuaikan dg Teori

catatan : + Tuliskan di Teori : - Geser

Perbaikan Seminar Hasil Skripsi harus diselesaikan <u>selambatnya 14 hari</u> terhitung sejak pelaksanaan Seminar. **<u>Bila melebihi 14 hari, maka tidak dapat diikutkan Ujian Skripsi.</u>**

Pengumpulan berkas untuk ujian skripsi dengan menyertakan lembar pengesahan dari dosen pembahas dan kaprodi

<u>Skripsi telah diperbaiki dan disetujui :</u>

Malang, ____________ 20	Malang, ____________ 20
Dosen Pembahas	Dosen Pembahas
(____________)	(____________)

INSTITUT TEKNOLOGI NASIONAL
Jl. Bendungan Sigura-gura 2
Jl. Raya Karanglo Km. 2
Malang

UJIAN SKRIPSI
PRODI TEKNIK SIPIL S-1

FORM REVISI / PERBAIKAN

BIDANG Struktur

Nama : Eko Budi Susanto

NIM : 10 21 072

Hari / tanggal : Sabtu, 15 Agustus 2015

Perbaikan materi Skripsi meliputi :

- sumber tabel & gambar, gambar peta respon spectra → warna
- no. tabel & gambar
- cek tanah di surabaya → lunak
- bagan alir → sebutkan SNI yg. digunakan
- ⊕kan teori / kajian pustaka SNI - 2012, 2013
- ⊕kan teori SRPM.   gempa? fotongan?
- hasil² drift → lampiran
- sebutkan kombinasi pd. M⊖, M⊕

erbaikan Skripsi harus diselesaikan **selambatnya 14 hari** terhitung sejak pelaksanaan Ujian ilaksanakan. **Bila melebihi** masa 14 hari, maka **tidak dapat diikutkan Yudisium**.

ugas Akhir telah diperbaiki dan disetujui :

Malang, 24/10 — 2018
Dosen Penguji

( )

Malang, 15 - 08 - 2015
Dosen Penguji

( )

INSTITUT TEKNOLOGI NASIONAL
Jl. Bendungan Sigura-gura 2
Jl. Raya Karanglo Km. 2
Malang

SEMINAR HASIL SKRIPSI II
PRODI TEKNIK SIPIL S-1

FORM REVISI / PERBAIKAN

BIDANG ____________________

Nama : EKO BUDI SUSANTO

NIM : 10.21.072

Hari / tanggal : SENIN / 25 MEI 2015

Perbaikan materi Seminar Hasil Skripsi I meliputi :

- Perbaiki Pembebanan.
- Perbaiki [illegible].

Malang, ____________________ 2014

Dosen Pembahas

(____________________)

INSTITUT TEKNOLOGI NASIONAL
Jl. Bendungan Sigura-gura 2
Jl. Raya Karanglo Km. 2
Malang

Form : 4

NILAI BIMBINGAN SKRIPSI

PROGRAM STUDI TEKNIK SIPIL S-1

›ertanda tangan di bawah ini menerangkan :

Nama : : EKO BUDI SUSANTO

NIM : 10 21 072

nenyelesaikan Skripsi dengan Judul :

STUDI PERENCANAAN PORTAL STRUKTUR GEDUNG DENGAN DASAR SNI 03-1726-2012 DAN SNI 03-2847-2013 PADA HOTEL ARIA CENTRA SURABAYA.

ıggal : ....12 - 01 - 15..... dengan nilai bimbingan : 80 (delapan puluh)

syarat untuk mengikuti ujian Skripsi dan Komprehensif Prodi Teknik Sipil S – 1 di Institut Teknologi l Malang.

Malang, 12 - 01 - 2015

Dosen Pembimbing

(A. Agus Santosa)

GAMBAR

- PORTAL
- DETAIL POTONGAN BALOK - KOLOM
- HUBUNGAN BALOK – KOLOM
- PORTAL MELINTANG