

**SRKIPSI**

**ANALISA PERENCANAAN DINDING GESER DENGAN BUKAAN  
PADA PEMBANGUNAN IJEN PADJADJARAN SUITES HOTEL  
RESORT**



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MALANG**

**2014**

SECRET

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DEPARTMENT OF HIGHER EDUCATION  
JANUARY 1962

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SECRET

# LEMBAR PERSETUJUAN

**ANALISA DINDING GESER DENGAN BUKAAN PADA GEDUNG IJEN  
PADJAJARAN SUITES HOTEL RESORT**

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**2014**

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SKRIPSI**

**ANALISA DINDING GESER DENGAN BUKAAN PADA  
PEMBANGUNAN IJEN PADJADJARAN SUITES HOTEL  
RESORT**

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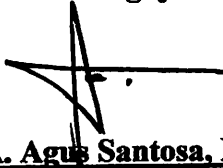
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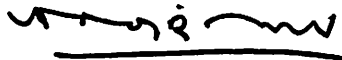
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## ABTARAKSI

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### **ANALISA DINDING GESER DENGAN BUKAAN PADA GEDUNG IDJEN PADJADJARAN SUITES HOTEL RESORT. Agus Faisal, 10.21.053.**

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Semakin banyak pembangunan gedung-gedung tinggi yang direncanakan tahan terhadap gempa, untuk struktur tahan gempa biasa di gunakan elemen dinding geser. Dinding geser dipasang untuk menambah kekakuan struktur dan menyerap gaya geser yang besar seiring dengan semakin tingginya struktur. Dinding geser juga berfungsi sebagai pengganti kolom dari segi pemanfaatan ruang. Dinding geser juga berperilaku sebagai balok lentur kantilever, dinding geser adalah elemen lentur dan tekan aksial.

Oleh karena itu, dinding geser selain menahan geser dan lentur juga menahan tekan aksial. Pada penelitian Tugas Akhir ini adalah dinding geser dengan bukaan atau yang biasa disebut opening shearwall, dinding geser yang ditinjau dari gedung 15 lantai yang berfungsi sebagai hotel. Penelitian difokuskan untuk menganalisa tulangan transversal dan longitudinal. Analisa statika pada model gedung menggunakan program bantu STAAD PRO 2004.

dengan mutu beton  $f_c' 30$  Mpa dan muru baja tulangan  $f_y 300$  Mpa serta Dari hasil gaya-gaya dalam yang di dapat dari program bantu di rencanakan tulangan transversal dan longitudinal untuk dinding geser, sehingga didapatkan diameter tulangan yang berbeda antara segmen dinding geser yang tidak berlubang dengan segmen dinding geser yang ada lubangnya.

**Kata Kunci : Tahan Gempa, dinding geser, tulangan**

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Alhamdulillah hirobbil alamin, puji syukur kehadiran Allah SWT. Solawat serta salam semoga selalu tercurahkan kepada Nabi Muhammad saw. Hanya atas berkat, rahmat dan hidayah-Nya sehingga penulis mampu menyelesaikan skripsi yang berjudul “**Analisis analisa perencanaan dinding geser dengan bukaan pada pembangunan ijen padjadjaran suites hotel resort**” dengan baik.

Atas terselesaikannya penulisan skripsi ini, penulis mengucapkan terima kasih kepada :

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Malang, Agustus 2014

**Penulis**



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**Kata Pengantar**

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# **BAB I**

## **PENDAHULUAN**

### **1.1.Latar Belakang**

Pertambahan penduduk yang semakin meningkat, mengakibatkan bertambah pula kebutuhan fisik antara lain untuk gedung perkantoran dan perumahan. Hal ini terutama sangat dirasakan dikota-kota besar. Salah satu jawaban untuk menyelesaikan hal ini adalah dengan pembangunan gedung kearah vertikal atau gedung bertingkat.

Perencanaan struktur terbagi atas dua komponen besar berupa perencanaan struktur atas dan perencanaan struktur bawah. Perencanaan struktur atas harus dapat memenuhi kriteria yang telah ditentukan baik dalam segi keamanan maupun dalam segi ekonomi. Terlebih dengan tuntutan tingkat keamanan gedung dalam menghadapi bahaya gempa. Mengingat Indonesia merupakan daerah pertemuan beberapa patahan dunia yang senantiasa terancam oleh bahaya gempa.

Perencanaan ini menggunakan dinding geser (shear wall) untuk mengoptimalkan dampak buruk bahaya gempa sehingga dapat menyelamatkan khususnya keselamatan nyawa manusia yang menggunakan gedung tersebutataupun orang yang berada disekeliling gedung tersebut.

Dinding geser ('shear wall') didefinisikan sebagai komponen struktur vertikal yang relatif sangat kaku. Fungsi dinding geser berubah menjadi dinding penahan beban ('bearing wall'), jika dinding geser menerima beban tegak lurus dinding geser.

Dinding geser sering digunakan pada gedung-gedung bertingkat tinggi untuk menahan gaya geser, gaya lateral akibat gempa. Dinding geser bersifat

kaku, maka terkadang menempatkan atau merencanakan sebuah ruang pada bangunan harus mengikuti perletakan dinding geser. Penempatan-penempatan pintu dan jendela juga dipertimbangkan agar tidak memngganggu perletakan dinding geser, karena lubang pada dinding geser yang diakibatkan pintu dan jendela mempengaruhi kekuatan dinding geser.

Oleh karena itu, pada penulisan tugas akhir ini penulis mencoba menganalisa perhitungan dinding geser dengan bukaan atau lubang, sehingga nantinya dapat direncanakan gedung dengan dinding geser dengan lubang atau bukaan yang kuat menahan gaya geser dan gaya lateral, dengan judul “ANALISA DINDING GESER DENGAN BUKAAN PADA PADA PEMBANGUNAN IJEN PADJADJARAN SUITES HOTEL RESORT”.

## **1.2. Identifikasi Masalah**

Pada pembangunan gedung bertingkat yang menggunakan dinding geser dengan lubang pada beberapa bagiannya, perlu ditinjau kekuatan dinding geser tersebut, karena lubang pada dinding geser akan mempengaruhi kekuatannya.

## **1.3. Perumusan Masalah**

Masalah yang akan dibahas pada penulisan tugas akhir ini :

1. Berapa tulangan yang dibutuhkan pada penulangan longitudinal dinding geser ?
2. Berapa tulangan yang dibutuhkan pada penulangan geser dinding geser ?

## **1.4. Tujuan Penulisan**

Maksud dilakukan analisa ini adalah sebagai berikut :

1. Mampu merencanakan penulangan longitudinal pada dinding geser.
2. Mampu merencanakan tulangan geser pada dinding geser.

## 1.5. Batasan Masalah

Pembahasan pada perencanaan dinding geser ini lebih dikhususkan pada dinding geser dengan bukaan. Berdasarkan masalah yang telah diuraikan di atas, maka untuk menghindari penyimpangan pembahasan perlu dibuat pembatasan masalah. Batasan-batasan yang dipakai dalam penulisan tugas akhir ini adalah :

1. Obyek analisa adalah pembangunan ijen padjadjaran suites hotel resort.
2. Analisa perencanaan penulangan longitudinal.
3. Analisa perencanaan penulangan geser.
4. Perencanaan menggunakan SK SNI 03-2847-2002.
5. Perhitungan analisa stuktur dengan menggunakan program STAAD Pro 2004.



## **BAB II TINJAUAN PUSTAKA**

### **2.1. Dinding Geser**

Untuk bangunan tinggi, diperlukan kekakuan yang cukup untuk menahan gaya-gaya lateral yang disebabkan oleh angin dan gempa. Jika bangunan tinggi tersebut tidak di desain secara benar terhadap gaya-gaya tersebut, dapat timbul tegangan yang sangat tinggi, serta getaran dan goyangan ke samping ketika gaya-gaya tersebut terjadi. Akibatnya tidak hanya menimbulkan kerusakan parah pada bangunan tersebut tetapi juga mengakibatkan ketidak nyamanan pada penghuni.

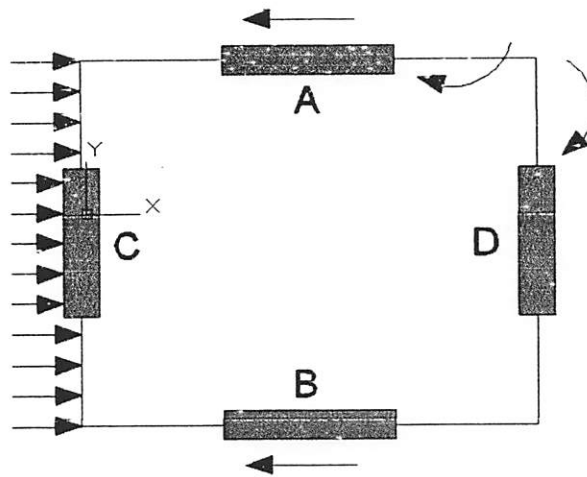
Ketika dinding beton bertulang dengan kekakuan bidang datar yang sangat besar ditempatkan pada lokasi-lokasi tertentu yang cocok dan strategis, dinding tersebut dapat digunakan secara ekonomis untuk menyediakan tahanan beban horizontal yang diperlukan. Dinding-dinding seperti ini disebut dinding geser dan pada dasarnya merupakan balok kantilever vertical yang tinggi dan memberikan stabilitas lateral kepada struktur dengan menahan geser dan momen tekuk pada bidang datar yang disebabkan gaya-gaya lateral.

Karena kekuatan dinding geser hampir selalu ditentukan oleh ketahanan lenturnya, kita sering kali salah menyebutkan namanya. Meskipun demikian, sebenarnya pada beberapa kejadian, dinding geser mungkin memerlukan beberapa tulangan geser untuk mencegah kegagalan tarik tarik diagonal.

Praktek yang umum adalah mengasumsikan gaya lateral bekerja pada semua tingkatan lantai. Kekakuan pelat lantai secara horizontal cukup besar bila

dibandingkan dengan kekakuan dinding dan kolom. Maka diasumsikan bahwa tiap-tiap lantai bergeser pada bidang horizontalnya sebagai sebuah struktur kaku.

Gambar 2.1 menunjukkan sebuah rencana bangunan yang menerima gaya horizontal. Gaya-gaya tersebut diterapkan kepada pelat lantai dan atap bangunan dan pelat-pelat itu bekerja sebagai balok besar di sisi-sisi dinding serta menyalurkan beban ke dinding geser A dan B. Jika gaya lateral datang dari arah lain, gaya-gaya tersebut akan ditahan oleh dinding geser C dan D.



Gambar 2.1

Dinding geser biasanya digunakan untuk bangunan dengan pelat lantai dasar. Sebenarnya, kombinasi pelat dan dinding adalah jenis konstruksi paling umum saat ini untuk digunakan pada bangunan apartemen yang tinggi dan bangunan residensial lainnya.

Dinding geser membentang pada keseluruhan jarak vertikal antar lantai. Jika dinding ditempatkan secara hati-hati dan simetris dalam perencanaannya, dinding geser sangat efisien dalam menahan beban vertikal maupun lateral dan



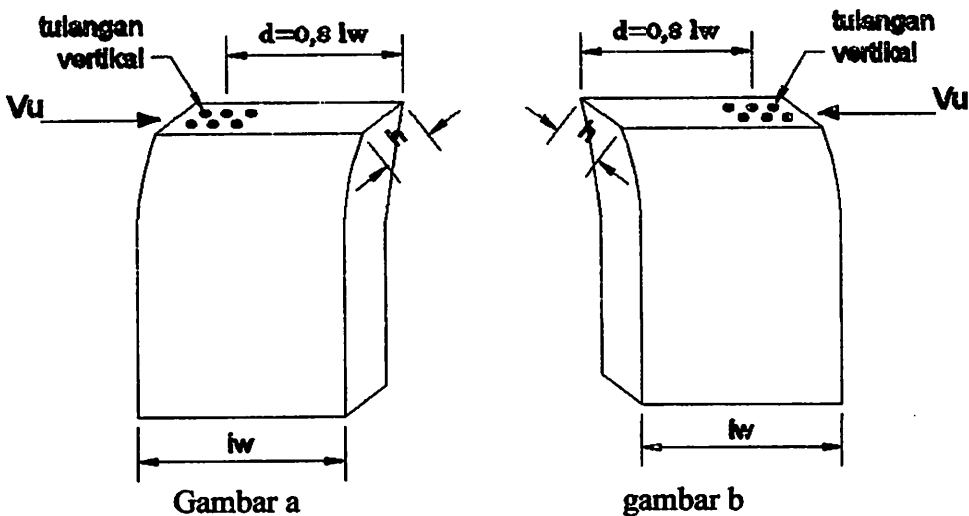
tidak mengganggu persyaratan arsitektur. Bangunan beton bertulang sampai 70 lantai harus dilengkapi dengan dinding geser. Pada arah horizontal, dinding geser penuh dapat digunakan dan dipasang memanjang pada keseluruhan panjang panel dan bagian utama struktur lainnya. Jika gaya yang terjadi lebih kecil, dinding geser hanya perlu dipasang pada sebagian panjang bagian utama struktur saja.

Dinding geser dapat digunakan untuk menahan gaya lateral saja maupun sebagai dinding pendukung. Selanjutnya, dinding geser dapat digunakan untuk ruang lift, tangga, dan mungkin toilet.

Jika diperlukan konstruksi tahan gempa, harus diingat bahwa bagian struktur yang relatif kaku akan menarik gaya yang jauh lebih besar daripada bagian yang fleksibel. Struktur dengan dinding geser beton bertulang akan cukup kaku sehingga dapat menyerap gaya gempa yang besar. Jika dinding geser rapuh dan runtuh, sisa dinding geser tersebut daktil, dan tingkat daktilitas yang baik akan tercapai bila dinding geser ditulangi dengan baik, dinding geser akan sangat efektif dalam menahan gaya gempa.

Bangunan beton bertulang yang tinggi sering didesain dengan dinding geser untuk menahan gaya gempa dan bangunan seperti ini telah bekerja cukup baik pada gempa yang terjadi akhir-akhir ini. Selama terjadinya gempa, dinding geser yang didesain dengan baik dapat dipastikan akan meminimalkan jumlah kerusakan pada portal struktur. Dinding geser juga meminimalkan kerusakan bagian non structural bangunan seperti jendela, pintu, partisi langit-langit, dan seterusnya.

Gambar 2.2 memperlihatkan dinding geser yang menerima gaya lateral  $V_u$ . Dinding tersebut sebenarnya adalah sebuah balok kantilever dengan lebar  $l_w$  dan tebal keseluruhan  $h$ . Pada Gambar bagian (a) dinding tertekuk dari kiri ke kanan akibat  $V_u$  dan akibatnya tulangan Tarik diperlukan di sebelah kiri atau pada sisi tarik.



Gambar 2.2. gambar dinding geser yang menerima gaya lateral

Jika diterpakan dari sisi kanan seperti diperlihatkan pada gambar bagian (b), tulangan tarik akan diperlukan pada sisi kanan dinding. Maka dapat kita lihat bahwa dinding geser memerlukan tulangan tarik kedua sisinya karena  $V_u$  bias datang dari kedua arah tersebut. Untuk perhitungan lentur, tinggi balok yang diperlukan dari sisi tekan dinding ke titik berat tulangan tarik adalah sekitar  $0,8 \times$  panjang dinding  $l_w$ .

### 2.1.1. Dinding Geser Berdasarkan Geometrinya

Dinding geser adalah struktur vertikal yang digunakan pada bangunan tingkat tinggi. Fungsi utama dari dinding geser adalah menahan beban lateral seperti gaya

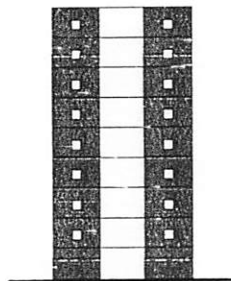
gempa dan angin. Berdasarkan geometrinya dinding geser dapat diklasifikasikan dalam beberapa jenis yaitu :

### 1. Dinding Geser dengan Bukaan ( Openning Shearwall )

Pada banyak keadaan, dinding geser tidak mungkin digunakan tanpa beberapa bukaan di dalamnya untuk jendela, pintu, dan saluran-saluran mekanikal dan elektrik. Meskipun demikian, kita dapat menempatkan bukaan-bukaan pada tempat di mana bukaan-bukaan tersebut tidak banyak mempengaruhi kekakuan atau tegangan pada dinding. Jika bukaan-bukaan tersebut kecil, pengaruh keseluruhannya sangat kecil tetapi tidak demikian halnya bila bukaan-bukaan yang berukuran besar.

Biasannya bukaan-bukaan tersebut ( jendela, pintu, dan sebagainya ) ditempatkan pada baris vertikal dan simetris pada dinding sepanjang ketinggian struktur. Penampang dinding pada sisi bukaan ini diikat menjadi satu, baik oleh balok yang terdapat pada dinding, pelat lantai, atau kombinasi keduanya. Seperti yang dapat anda lihat, analisis struktur untuk situasi seperti ini sangat rumit dan bisanya dilakukan dengan persamaan empiris.

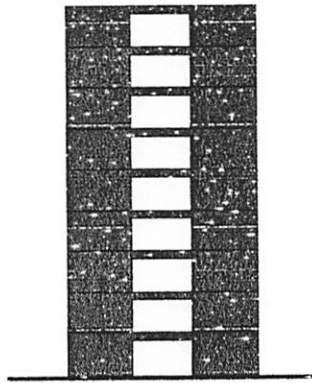
Bukaan sedikit mengganggu pada geser dukung struktur. Perlawanan lentur struktur penopang bagian dasar kritis secara drastis dikurangi dengan perubahan tiba-tiba dari bagian dinding ke kolom.



Gambar 2.3 Dinding geser dengan bukaan

## 2. Dinding geser berangkai (coupled shearwall).

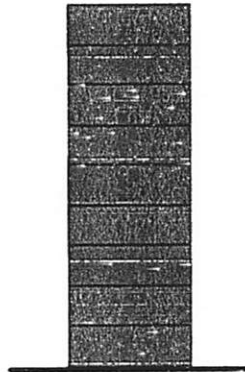
Dinding geser berangkai terdiri dari dua atau lebih dinding kantilever yang mempunyai kemampuan untuk membentuk suatu mekanisme peletakan lentur alasnya. Antara dinding geser kantilever tersebut saling dirangkaikan oleh balok-balok perangkai yang mempunyai kekuatan cukup sehingga mampu memindahkan gaya dari satu dinding ke dinding yang lain (gambar 2.7).



2.4. Dinding geser berangkai

## 3. Dinding geser kantilever (free standing shearwall).

Adalah suatu dinding geser tanpa lubang-lubang yang membawa pengaruh penting terhadap perilaku dari struktur gedung yang bersangkutan. Dinding geser kantilever ada dua macam, yaitu dinding geser kantilever daktail dan dinding geser katilever dengan daktilitas terbatas



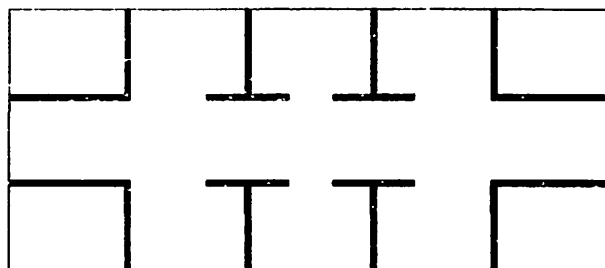
2.5. Dinding geser kantilever

### 2.1.2. Dinding Geser Berdasarkan Letak dan fungsinya

Berdasarkan letak dan fungsinya dinding geser dapat diklasifikasikan dalam beberapa jenis yaitu :

#### 1. Bearing Walls

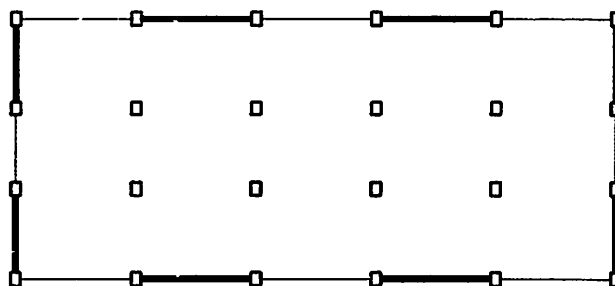
adalah dinding geser yang juga mendukung sebagian besar beban gravitasi. Tembok-tembok ini juga menggunakan dinding partisi antarapartemen yang berdekatan.



2.6. Bearing Walls

#### 2. Frame Walls

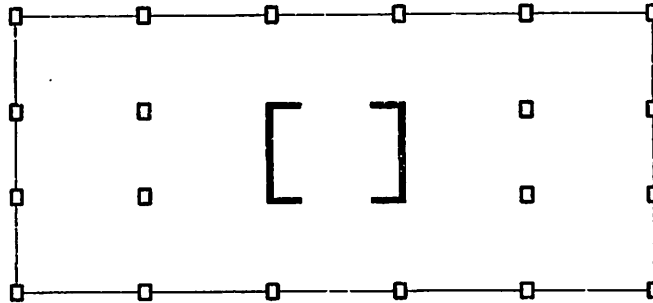
adalah dinding geser yang menahan beban lateral, dimana beban gravitasi berasal dari frame beton bertulang. Tembok-tembok ini dibangun diantara baris kolom.



2.7. Frame Walls

### 3. Core Walls

adalah dinding geser yang terletak di dalam wilayah inti pusat dalam gedung, yang biasanya diisi tangga atau poros lift. Dinding yang terletak di kawasan inti pusat memiliki fungsi ganda dan dianggap menjadi pilihan ekonomis.



2.8. Core Walls

### 2.2. Struktur Bangunan Tahan Gempa

Untuk perencanaan pada pembangunan Ijen Padjadjaran Suites Hotel Resort menggunakan analisis beban gempa pada gedung beraturan

#### 2.2.1. Analisis bangunan tahan gempa pada Gedung Beraturan

##### a. Beban geser dasar nominal statik ekivalen ( V )

Beban geser dasar statik ekivalen (V) ditentukan berdasarkan ketentuan pasal 6.1.2 SPKGUSBG-2002, yaitu :

$$V = \frac{C_1 \cdot i}{R} \cdot W_t$$

Dimana :

V = Beban (gaya) geser dasar nominal statik ekivalen akibat pengaruh gempa rencana yang bekerja di tingkat dasar struktur gedung beraturan, kN.

C<sub>1</sub> = Nilai faktor respons gempa yang diperoleh dari spektrum respons gempa rencana untuk waktu getar alami fundamental dari

struktur gedung.

I = faktor keutamaan gedung

R = Faktor reduksi gempa

Wt = Berat total gedung, termasuk beban hidup yang sesuai, kN.

Cara ini adalah merupakan analisa beban gempa static ekuivalen yang biasanya digunakan pada gedung-gedung yang strukturnya beraturan dan tinggi bangunan  $\leq 40$  m. Dimana pembangian beban geser dasar ( V ) akibat gempa sepanjang tinggi gedung dibagikan menjadi beban-beban horizontal terpusat yang bekerja pada masing-masing tingkat lantai.

**b. Beban gempa nominal statik ekuivalen pada lantai ( $F_1$ )**

Beban gempa nominal statik ekuivalen ( $F_1$ ) ditentukan berdasarkan ketentuan pasal 6.1.3 SPKGUSBG-2002, yaitu :

$$F_1 = \frac{W_1 \cdot Z_1}{\sum_{i=1}^n (W_i \cdot Z_i)} \cdot V$$

Dimana :

$F_i$  = Beban gempa nominal statik ekuivalen yang menangkap pada pusat massa pada taraf lantai tingkat ke-I struktur atas gedung, kN.

$W_i$  = Berat lantai tingkat ke-i struktur atas suatu gedung, termasuk beban hidup yang sesuai, kN.

$Z_i$  = Ketinggian lantai tingkat ke-I gedung terhadap taraf penjepitan lateral, m.

N = Nomor lantai paling atas.

**c. Koefesien gempa dasar - C**

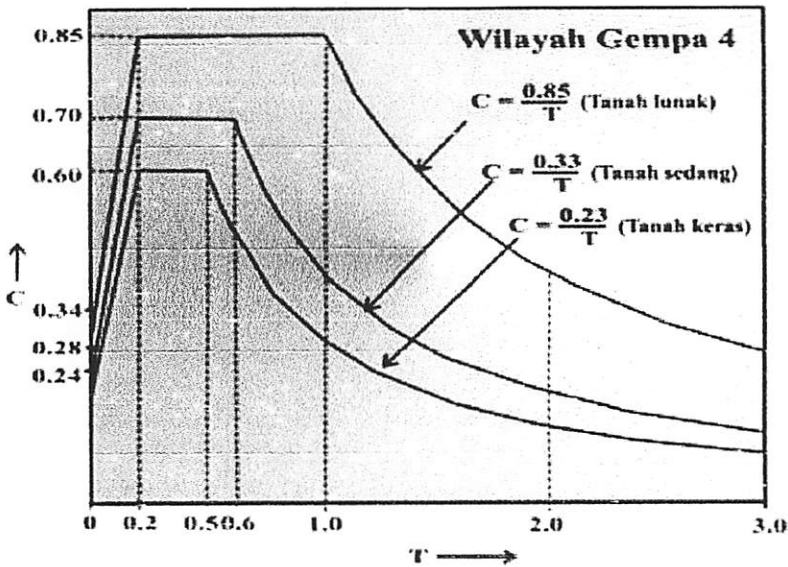
Koefesien gempa dasar ditentukan dari PPKGURG 1987, dengan memakai waktu getar alami struktur gedung. Dua jenis tanah bawah harus

dibedakan dalam memilih nilai C, yaitu tanah keras dan tanah lunak. Untuk menentukan pedoman ini suatu struktur gedung harus dianggap berdiri diatas tanah bawah yang lunak.

### 2.2.2. Wilayah Gempa dan Respons Spektrum

Indonesia ditetapkan terbagi dalam 6 Wilayah Gempa, dimana Wilayah gempa 1 adalah wilayah dengan kegempaan paling rendah dan wilayah gempa 6 dengan kegempaan yang paling tinggi. Pembagian wilayah gempa ini didasarkan atas percepatan puncak batuan dasar akibat pengaruh gempa rencana dengan periode ulang 500 tahun.

Pada lokasi yang dipakai dalam penulisan skripsi ini berada di kota Malang sehingga termasuk dalam wilayah gempa 4 dengan respons spectrum gempa rencana seperti pada gambar 2.9.



gambar 2.9. respons spectrum gempa rencana

### 2.2.3. Rencana Pembebanan

Berdasarkan Peraturan Pembebanan Indonesia Untuk Gedung 1983 (DPU,1983), beban yang harus diperhitungkan untuk suatu struktur adalah beban



mati, beban hidup, beban angin, beban gempa dan kombinasi dari beban-beban tersebut.

Pengertian dari setiap beban tersebut adalah sebagai berikut ini.

1. **Beban-mati** adalah berat dari semua bagian struktur gedung yang bersifat tetap, termasuk segala unsur tambahan, mesin-mesin, serta peralatan tetap yang merupakan bagian tak terpisahkan dari gedung itu.
2. **Beban-hidup** adalah semua beban yang terjadi akibat penghunian atau penggunaan suatu gedung, dan ke dalamnya termasuk beban-beban padalantai yang berasal dari barang-barang yang dapat berpindah, mesin-mesin serta peralatan yang tidak merupakan bagian yang tak terpisahkan dari gedung dan dapat diganti selama masa dari gedung itu, sehingga mengakibatkan perubahan dalam pembebanan lantai dan atap gedung tersebut.
3. **Beban-gempa** adalah semua beban statik ekuivalen yang bekerja pada gedung yang menirukan pengaruh dari gerakan tanah akibat gempa tersebut.
4. **Beban angin** adalah semua beban yang bekerja pada gedung atau bagian gedung yang disebabkan oleh selisih dalam tekan udara

Berat sendiri bahan bangunan dan komponen struktur gedung menurut Peraturan Pembebanan Indonesia untuk Rumah dan Gedung (DPU, 1983) yang digunakan adalah :

- |                                     |                          |
|-------------------------------------|--------------------------|
| a. beton Bertulang                  | : 24 kN/m <sup>3</sup>   |
| b. adukan dari semen (per cm tebal) | : 0,21 kN/m <sup>3</sup> |

- c. penutup lantai (tanpa adukan, per cm tebal) : 0,24 kN/m<sup>3</sup>
- d. plafon dan penggantung : 0,18 kN/m<sup>3</sup>
- e. dinding pasangan bata merah ½ batu : 2,5 kN/m<sup>3</sup>

#### 2.2.4. Analisis Statik Ekuivalen

Analisis perancangan struktur bangunan terhadap pengaruh beban gempa secara statis, pada prinsipnya adalah menggantikan gaya-gaya horizontal yang bekerjapada struktur akibat pergerakan tanah dengan gaya-gaya statis yang ekuivalen, dengantujuan penyederhanaan dan kemudahan di dalam perhitungan. Metode ini disebut Metode Gaya Lateral Ekuivalen (Equivalent Lateral Force Method). Pada metode ini diasumsikan bahwa gaya horizontal akibat gempa yang bekerja pada suatu elemenstruktur, besarnya ditentukan berdasarkan hasil perkalian antara suatu konstanta beratatau massa dari elemen struktur tersebut.

### 2.3. Perencanaan Dinding Geser dengan bukaan.

#### 2.3.1. Dessain Dinding Geser Terhadap Beban Lentur dan Beban Aksial

Rasio penulangan  $p_v$  dan  $p_n$  untuk dinding strutural tidak boleh kurang dari 0,0025 pada arah sumbu-sumbu longitudinal dan transversal. Apabila tebal dinding lebih besar atau sama dengan 200 mm dan atau apabila nilai gaya geser terfaktor yang bekerja pada suatu bidang dinding geser melampaui nilai :  $\frac{1}{6} \cdot A_{cv} \cdot \sqrt{f'c}$  , maka pada dinding tersebut paling sedikit harus di pasang tulangan dalam 2 lapis, dimana dinding harus memiliki tulangan geser tersebar yang memberikan perlawanan dalam dua arah yang saling tegak lurus dalam bidang.

Beberapa pembatasan untuk penulangan lentur vertikal dinding geser menurut *Paulay dan Priestley*, yaitu :

- a. Besarnya  $\rho_v$  pada seluruh bagian dinding geser tidak boleh kurang dari  $0,7/f_y$  ( dalam MPa ) dan tidak lebih dari  $16/f_y$  ( MPa ).
- b. Jarak antar tulangan vertikal tidak boleh lebih dari 200 mm dan pada daerah lain ( yaitu daerah elastis ), 450 mm atau tiga kali tebal dinding.
- c. Diameter tulangan yang digunakan tidak boleh melebihi  $1/8$  dari tebal dinding geser.

Jika pembatasannya tulangan lentur dibatasi sesuai dengan momen yang terjadi, maka sendi plastis dapat terbentuk di semua bagian di sepanjang tinggi dinding geser dengan tingkat kemungkinan yang sama. Hal ini tidak diinginkan dari segi perencanaan karena daerah sendi plastis memerlukan detail tulangan khusus. Jika sendi plastis mempunyai kemungkinan yang sama untuk terjadi pada setiap bagian sepanjang tinggi dinding geser, maka pendetailan khusus untuk sendi plastis harus dilakukan di sepanjang tinggi dinding. Tentu saja hal ini sangatlah tidak ekonomis. Selain itu, kuat dinding geser akan berkurang pada daerah dimana pelelehan tulangan lentur terjadi. Hal ini akan mengharuskan penambahan tulangan geser pada setiap tingkat. Akan lebih rasional memastikan bahwa sendi plastis hanya bisa terjadi pada lokasi yang telah ditentukan sebelumnya, secara logika yaitu di dasar dinding geser, dengan cara menetapkan kuat lentur melebihi kekuatan lentur maksimum yang dibutuhkan.

Diagram bidang momen menunjukkan momen dari hasil aplikasi gaya statis lateral dengan kekuatan ideal terjadi pada dasar. Gambar tersebut menunjukkan kekuatan lentur minimum ideal yang harus ditetapkan dimana kekuatan ideal terjadi pada dasar dinding geser.

Sesuai dengan gambar 2.3.1, daerah perubahan kekuatan diasumsikan terjadi pada jarak yang sama dengan lebar dinding geser  $l_w$ . Dimana daerah dengan ketinggian sebesar  $l_w$  akan menerima momen lentur yang sama dengan momen pada dasar dinding geser. Daerah setinggi  $l_w$  tersebut merupakan daerah sendi plastis.

Untuk keperluan penyambungan, tulangan dari tingkat sebelumnya harus diteruskan agar menjamin perilaku serta kekuatan dari struktur. Panjang tulangan yang diteruskan tersebut panjangnya tidak kurang dari panjang penyaluran  $l_d$ . Besarnya  $l_d$  dapat dihitung dengan rumus :

$$l_d = m_{db} \cdot l_{db}$$

dimana :

$$l_{db} = \frac{1,38 \cdot A_b \cdot f_y}{c \cdot \sqrt{f'c}}$$

dengan :

$A_b$  = Luas penampang tulangan ( $\text{mm}^2$ )

$c$  = 3 x diameter tulangan ( mm )

$m_{db}$  = faktor modifikasi 1,3.

### 2.3.2. Desain Dinding Geser Terhadap Beban Beban Geser

Elemen dinding (Wall) dikatakan sebagai dinding geser (shear wall) karena kemampuannya untuk memikul beban geser akibat beban lateral lebih diandalkan/ditekankan bila dibandingkan dengan kemampuannya menahan beban yang lain, walaupun tidak menutup kemungkinan untuk dapat ikut serta memikul

Beberapa pembatasan untuk penulangan dinding geser menurut *Paulay dan Priestley* adalah :

- a. Besarnya rasio penulangan horizontal ( $\rho_h$ ) minimal 0,0025 atau  $\rho_h \geq 0,0025$ .
- b. Jarak antar tulangan horizontal tidak boleh melebihi dua setengah kali tebal dinding atau 450 mm.
- c. Diameter tulangan yang digunakan tidak boleh lebih dari  $\frac{1}{8}$  tebal dinding geser.

Keruntuhan akibat geser sedapat mungkin dihindarkan. Karena itu, kekuatan dinding geser terhadap geser harus dibuat melampaui besarnya gaya geser maksimum yang mungkin terjadi.

Pada waktu berlangsungnya gempa, pada dinding geser akan terjadi gaya geser yang lebih besar dibandingkan dengan perkiraan semula dengan analisa statik. Untuk mendapatkan kapasitas yang ideal pada setiap ketinggian dinding, maka gaya geser rencana harus diperbesar dengan memasukkan faktor  $\phi$  dan faktor pembesaran dinamis ( $\omega$ ). Faktor  $\phi$  dimaksudkan agar tidak terjadi keruntuhan geser terlebih dahulu sebelum terjadi keruntuhan/pelelehan lentur pada struktur.

Menurut SK-SNI 03-2847-2002 pasal 23.6.3, kuat geser rencana bagi dinding geser pada penampang dasar sehubungan dengan adanya pembesaran dinamis, harus dihitung menurut persamaan berikut :

$$V_{u.d.maks} = \omega_d \cdot 0,7 \cdot \frac{M_{kap.d}}{M_{E.d.maks}} \cdot V_{d.maks}$$

Dimana :

- $M_{kap.d}$  = Momen kapasitas dinding geser pada penampang dasar yang dihitung berdasarkan luas baja tulangan yang terpasang dan tegangan tarik baja tulangan

- $M_{E.d.maks}$  = Momen lentur maksimum dinding geser akibat beban gempa tak berfaktor pada penampang.
- $M_{E.d.maks}$  = Gaya geser maksimum dinding geser akibat beban gempa tak berfaktor pada penampang.
- $\omega_d$  = Koefisien pembesaran dinamis yang memperhitungkan pengaruh dari terjadinya sendi plastis pada struktur secara keseluruhan.

Menurut SK-SNI 03-2847-2002 pasal 23.6.4 butir 1, kuat geser nominal

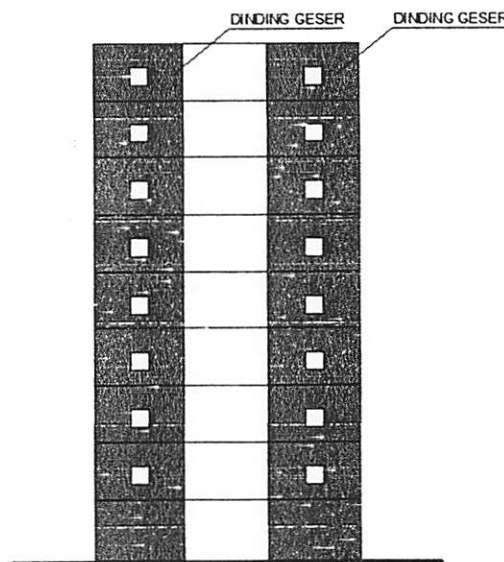
$V_n$  dinding struktural tidak diperkenankan lebih daripada :

$$V_n = A_{CV} ( \alpha_C \sqrt{f'_c} + p_n f_y )$$

Dimana koefisien :

- $\alpha_C = 1/4$  untuk  $( h_w/l_w ) \leq 1,5$
- $\alpha_C = 1/6$  untuk  $( h_w/l_w ) \leq 2$

Tahanan geser nominal segmen-segmen dinding horizontal tidak boleh diambil melebihi  $( 5/6 ) A_{CP} \sqrt{f'_c}$  , dimana  $A_{CP}$  adalah luas penampang segmen dinding horizontal atau balok perangkai, seperti pada gambar 2.10.

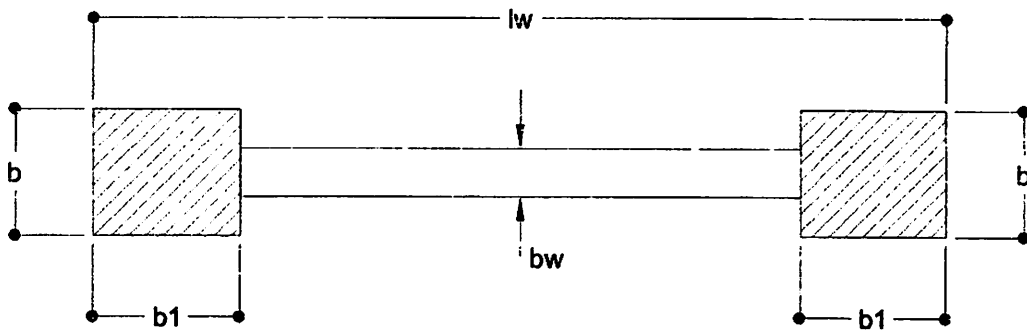


Gambar 2.10. Dinding geser dengan bukaan

Kontrol Penulangan, Ukuran dimensi dan jarak antar tulangan agar dinding tersebut dapat memenuhi persyaratan yang ada. Rasio penulangan dinding geser adalah sebesar :

$$\rho_1 = \sum A_b / b_{sv}$$

Dimana  $A_b$  adalah luas tulangan dan  $b_{sv}$  adalah jarak antar tulangan, tidak boleh kurang dari  $0,7/f_y$  ( Mpa ) dan tidak boleh lebih dari  $1,6/f_y$  ( Mpa ). Sedangkan untuk pembatasan dimensi dinding adalah sebagai berikut :



Gambar 2.11. Pembatasan dimensi dinding geser

$$b \geq bw \qquad b_1 \geq \frac{bc.lw}{10.b}$$

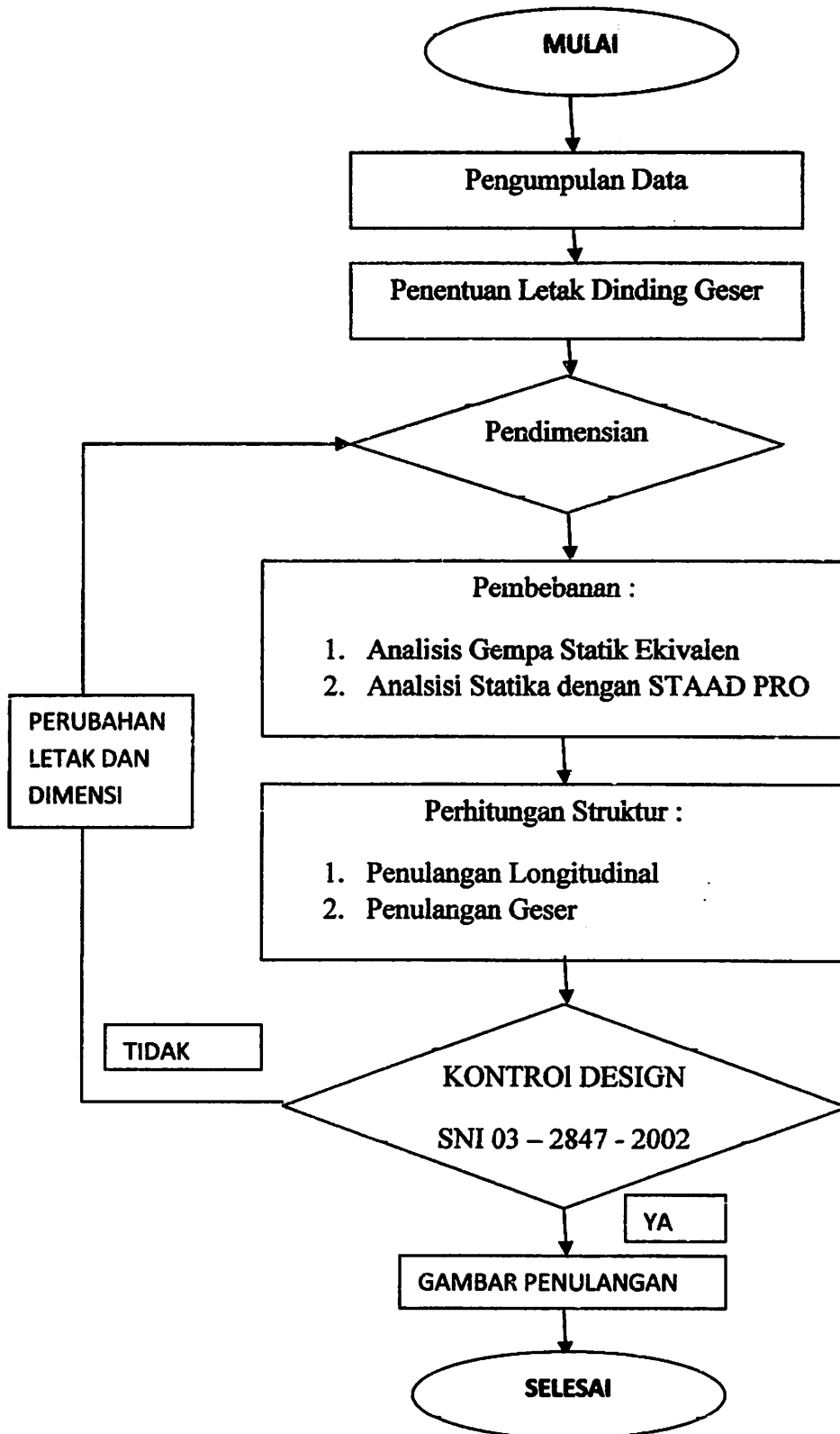
$$b \geq bc \qquad b_1 \geq \frac{bc^2}{b}$$

$$b \geq h_i/16 \qquad b_1 \geq h_i/16$$

dimana :  $bc = 0,0171 . lw . \sqrt{\mu_\phi}$

$\mu_\phi =$  rasio daktilitas kurva = 5

## 2.4. Bagan Alir





## **BAB III**

### **PERHITUNGAN PEMBEBANAN DINDING GESER**

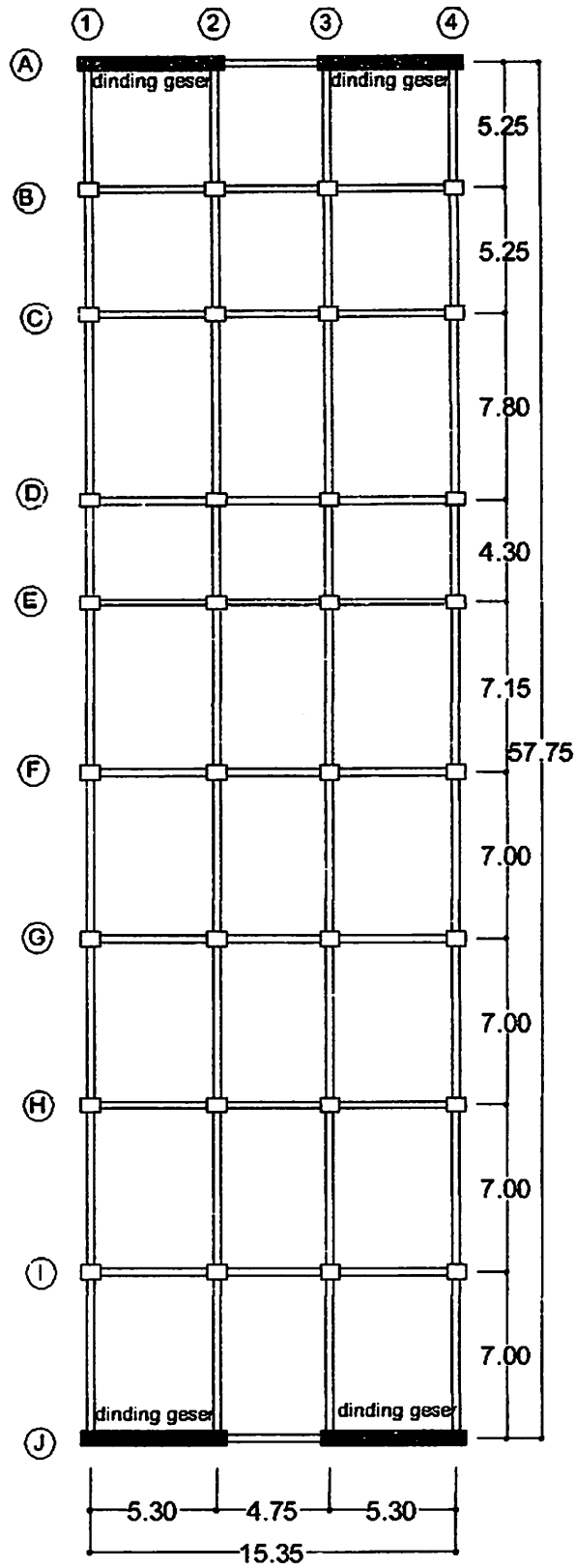
#### **3.1 Data Bangunan**

Data umum Pembangunan Ijen Padjadjaran Suites Hotel Resort and Convention Hall adalah sebagai berikut :

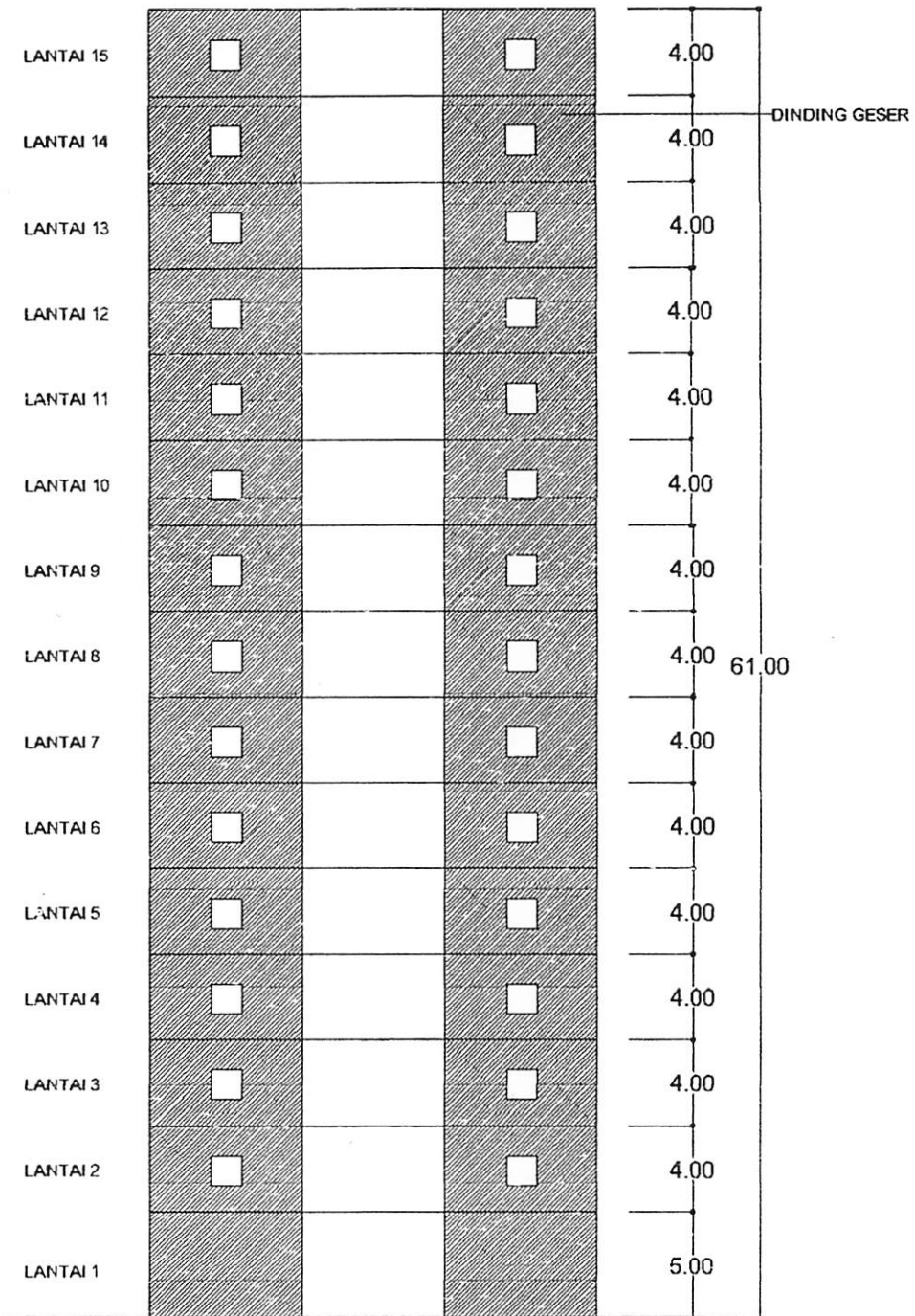
- Nama Gedung : Ijen Padjadjaran Suites Hotel Resort and  
Convention Hall
- Lokasi Bangunan : Ijen Nirwana Residence, Malang – Jawa Timur
- Fungsi : Gedung Hotel
- Daerah gempa : Wilayah Gempa 4
- Luas bangunan : 924 m<sup>2</sup>
- Tinggi bangunan : 52,5 m
- Tinggi tiap bangunan : 3,5 m
- Jumlah lantai : 15 lantai
- Struktur bangunan : Beton Bertulang

#### **3.2 Mutu Bahan Yang Digunakan**

- Mutu beton ( $f_c$ ) : 30 Mpa
- Mutu baja ulir ( $f_y$ ) : 300 MPa
- Mutu baja polos ( $f_y$ ) : 240 Mpa



**Gambar 3.1. Perletakan Dinding Geser**



**Gambar 3.2. Perletakan Dinding Geser Dari Depan**

### 3.3. Pendimensian Kolom, Balok dan Dinding Geser

#### 3.3.1. Dimensi Kolom

Karena yang ditinjau adalah dinding geser, maka untuk dimensi kolom seperti pada gambar rencana Ijen Padjadjaran Suites Hotel Resort dengan ukuran 60/80 cm.

#### 3.3.2. Dimensi Balok

Karena yang ditinjau adalah dinding geser, maka untuk dimensi balok seperti pada rencana Ijen Padjadjaran Suites Hotel Resort yang sudah ada dengan ukuran :

1. Bentang 5,3 m ( B1 ) = 30/50 cm
2. Bentang 4,75 m ( B2 ) = 25/40 cm
3. Bentang 7 m ( B3 ) = 40/60 cm
4. Bentang 4,3 m ( B2 ) = 25/40 cm



### 3.3.3. Dimensi Dinding Geser

Menurut SNI 03-2847-2002 pada penjelasan pasal S13.10.3 banyak percobaan pada dinding geser dengan ketebalan sama sebesar  $l_w / 25$  telah menunjukkan bahwa dapat diperoleh tegangan ultimate lebih dari  $(5/6) \sqrt{f'_c}$ .

Jadi untuk tebal ( $t$ ) Dinding geser berdasarkan lebar dinding :

- $l_w = 560 \text{ cm}$
- $t = l_w / 25$   
 $= 560 / 25$   
 $= 22,4 \text{ cm} \dots\dots\dots$  dipakai  $t = 25 \text{ cm}$

Berdasarkan rumusan hasil T. pauly dan M. J. N. Priestley dalam bukunya yang berjudul "Seismic Design of Reinforced Concrete and Mansory Building", dimensi dinding geser berdasarkan tinggi dinding harus memenuhi persyaratan sebagai berikut :

- $h_1 = 5 \text{ m}$
- $h_2 = 4 \text{ m}$
- $t \geq \frac{h_1}{16}$   
 $\geq \frac{5}{16}$   
 $\geq 0,31 \text{ m} = 31 \text{ cm} \dots\dots\dots$  Dipakai  $35 \text{ cm}$
- $t \geq \frac{h_2}{16}$   
 $\geq \frac{4}{16}$   
 $\geq 0,25 \text{ m} = 25 \text{ cm} \dots\dots\dots$  Dipakai  $25 \text{ cm}$
- Maka untuk tebal dinding geser dipakai  $35 \text{ cm}$

Untuk kontrol lebar dinding geser ( $l_w$ ) =  $l_w < l_{wmaks}$

Diambil type dinding geser dengan  $l_w$  terpanjang

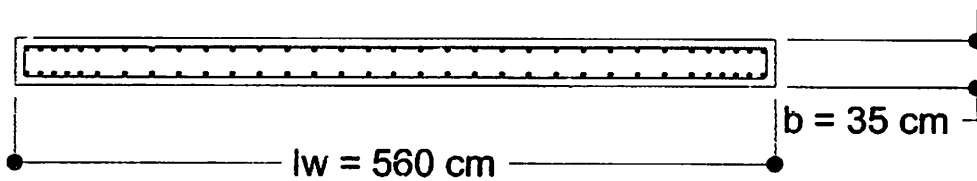
➤  $t = 35 \text{ cm}$

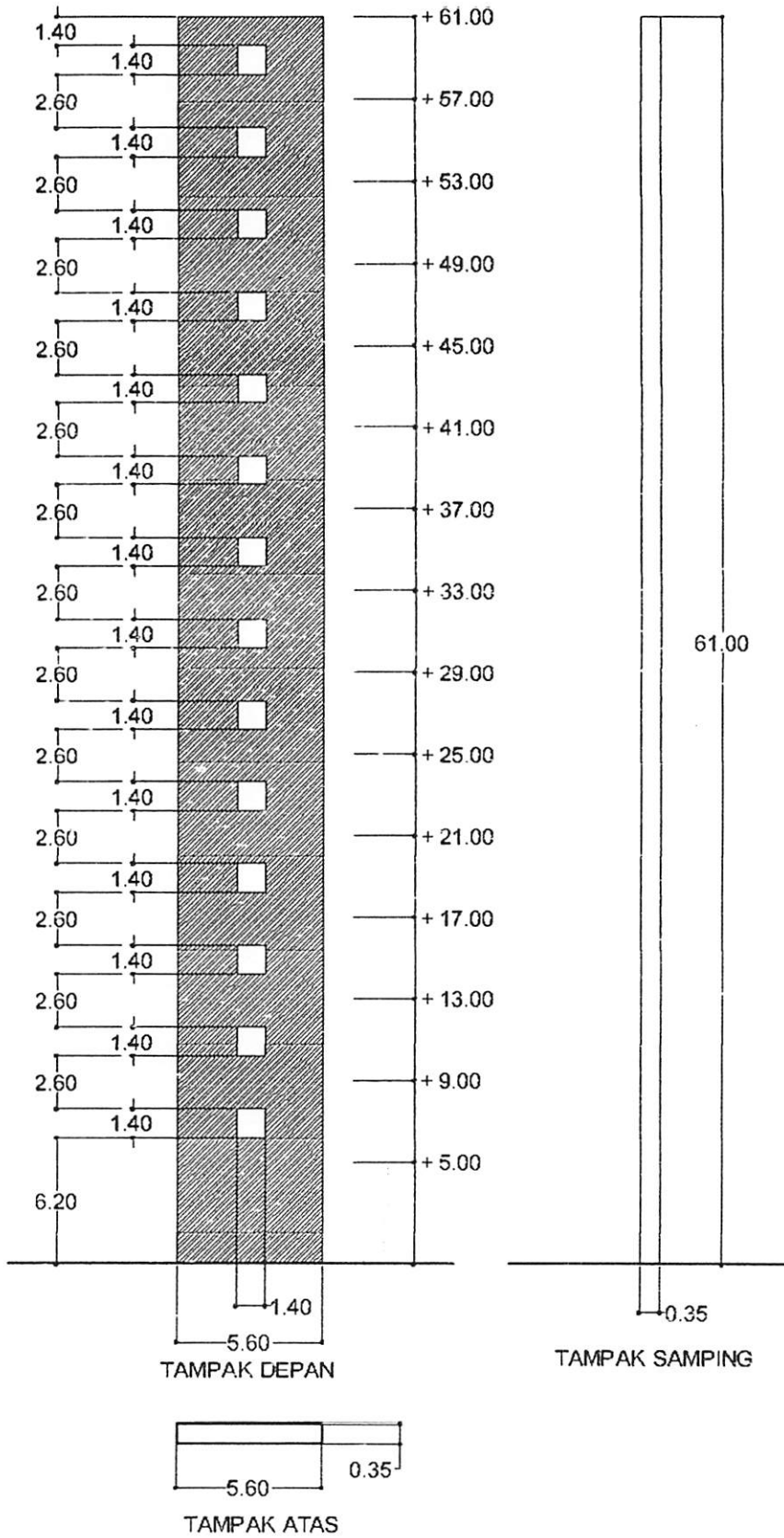
➤  $h_l = 500$

➤  $l_w = 560 \text{ cm}$

➤  $l_{wmaks} = 1,6 \cdot h_l$   
 $= 1,6 \cdot 500$   
 $= 800 \text{ cm}$

➤  $l_w = 560 \text{ cm} < l_{wmaks} = 800 \text{ cm} \dots \text{ (ok)}$





**Gambar 3.3. Gambar rencana dinding Geser**

### 3.4. Pembebanan.

Sesuai dengan PPI'87 tabel 3.1 :

#### a. Beban Hidup

➤ Beban hidup lantai 1-15 = 250 kg/m<sup>2</sup>

➤ Beban hidup atap = 100 kg/m<sup>2</sup>

#### b. Beban mati

➤ Beton Bertulang = 2400 kg/m<sup>3</sup>

➤ Keramik = 25 kg/m<sup>2</sup>

➤ Spesi per 1 cm tebal = 21 kg/m<sup>2</sup>

➤ Langit-langit = 11 kg/m<sup>2</sup>

➤ Pasir = 1600 kg/m<sup>3</sup>



### 3.4.1 Perhitungan Beban Gempa Statik Ekuivalen

#### 1. Perhitungan Berat sendiri

##### - Lantai 15 ( Atap )

##### - Beban Mati ( WDL )

- Berat Plat Atap t = 10 cm	=	57.75	x	15.35	x	0.1	x	2400	x	=	212,751.00	kg	
- Berat Kolom ( 60/80 )	=	2	x	0.6	x	0.8	x	2400	x	32	=	92,160.00	kg
- Berat Balok ( 20/30 )	=	56.1	x	0.2	x	0.3	x	2400	x	=	8,078.40	kg	
- Berat Balok ( 30/40 )	=	148	x	0.3	x	0.4	x	2400	x	=	42,624.00	kg	
- Berat Balok ( 30/50 )	=	171.8	x	0.3	x	0.5	x	2400	x	=	61,848.00	kg	
- dinding geser t = 35 cm	=	2	x	0.35	x	5.3	x	2400	x	4	=	35,616.00	kg
- Berat dinding memanjang	=	2	x	0.15	x	57.75	x	1700	x	4	=	117,810.00	kg
- Berat dinding melintang	=	2	x	0.15	x	132.3	x	1700	x	=	67,473.00	kg	
												638,360.40	kg

##### - Beban Hidup ( WLL )

- Beban hidup atap	=	57.75	x	15.35	x	100	=	88,646.25	kg				
- Beban Air hujan	=	57.75	x	15.35	x	0.05	x	1000	=	44,323.13	kg		
												132,969.38	kg

- **Beban Total** = **771,329.78** kg

##### - Lantai 2 - i4

##### - Beban Mati ( WDL )

- Berat Plat Lantai t = 12	=	57.75	x	15.35	x	0.12	x	2400	x	=	255,301.20	kg	
- Berat Kolom ( 60/80 )	=	4	x	0.6	x	0.8	x	2400	x	32	=	184,320.00	kg
- Berat Balok ( 25/40 )	=	56.1	x	0.25	x	0.4	x	2400	x	=	13,464.00	kg	
- Berat Balok ( 30/50 )	=	148	x	0.3	x	0.5	x	2400	x	=	53,280.00	kg	
- Berat Balok ( 40/60 )	=	171.8	x	0.4	x	0.6	x	2400	x	=	98,956.80	kg	
- Berat keramik	=	57.75	x	15.35	x	25	=	22,161.56	kg				
- Berat Spesi	=	57.75	x	15.35	x	21	=	18,615.71	kg				
- dinding geser t = 35 cm	=	4	x	0.35	x	5.3	x	2400	x	4	=	71,232.00	kg
- dinding memanjang	=	4	x	0.15	x	57.75	x	1700	x	4	=	235,620.00	kg
- dinding melintang	=	4	x	0.15	x	132.3	x	1700	x	=	134,946.00	kg	
												1,087,897.28	kg

##### - Beban Hidup ( WLL )

- Beban hidup Lantai	=	57.75	x	15.35	x	250	=	221,615.63	kg				
												221,615.63	kg

##### Beban

- **Total** = **1,309,512.90** kg

**Lantai 1**

**- Beban Mati ( WDL )**

- Berat Plat Lantai t = 12 cm	=	57.75	x	15.35	x	0.12	x	2400	x	=	255,301.20	kg	
- Berat Kolom ( 60/80 )	=	4.5	x	0.6	x	0.8	x	2400	x	32	=	207,360.00	kg
- Berat Balok ( 25/40 )	=	56.1	x	0.25	x	0.4	x	2400	x	=	13,464.00	kg	
- Berat Balok ( 30/50 )	=	148	x	0.3	x	0.5	x	2400	x	=	53,280.00	kg	
- Berat Balok ( 40/60 )	=	171.8	x	0.4	x	0.6	x	2400	x	=	98,956.80	kg	
- Berat keramik	=	57.75	x	15.35	x	25				=	22,161.56	kg	
- Berat Spesi	=	57.75	x	15.35	x	21				=	18,615.71	kg	
- dinding geser t = 35 cm	=	4.5	x	0.35	x	5.3	x	2400	x	4	=	80,136.00	kg
- dinding memanjang	=	4.5	x	0.15	x	57.75	x	1700	x	4	=	265,072.50	kg
- dinding melintang	=	4.5	x	0.15	x	132.3	x	1700	x	=	151,814.25	kg	
												1,166,162.03	kg

**- Beban Hidup ( WLL )**

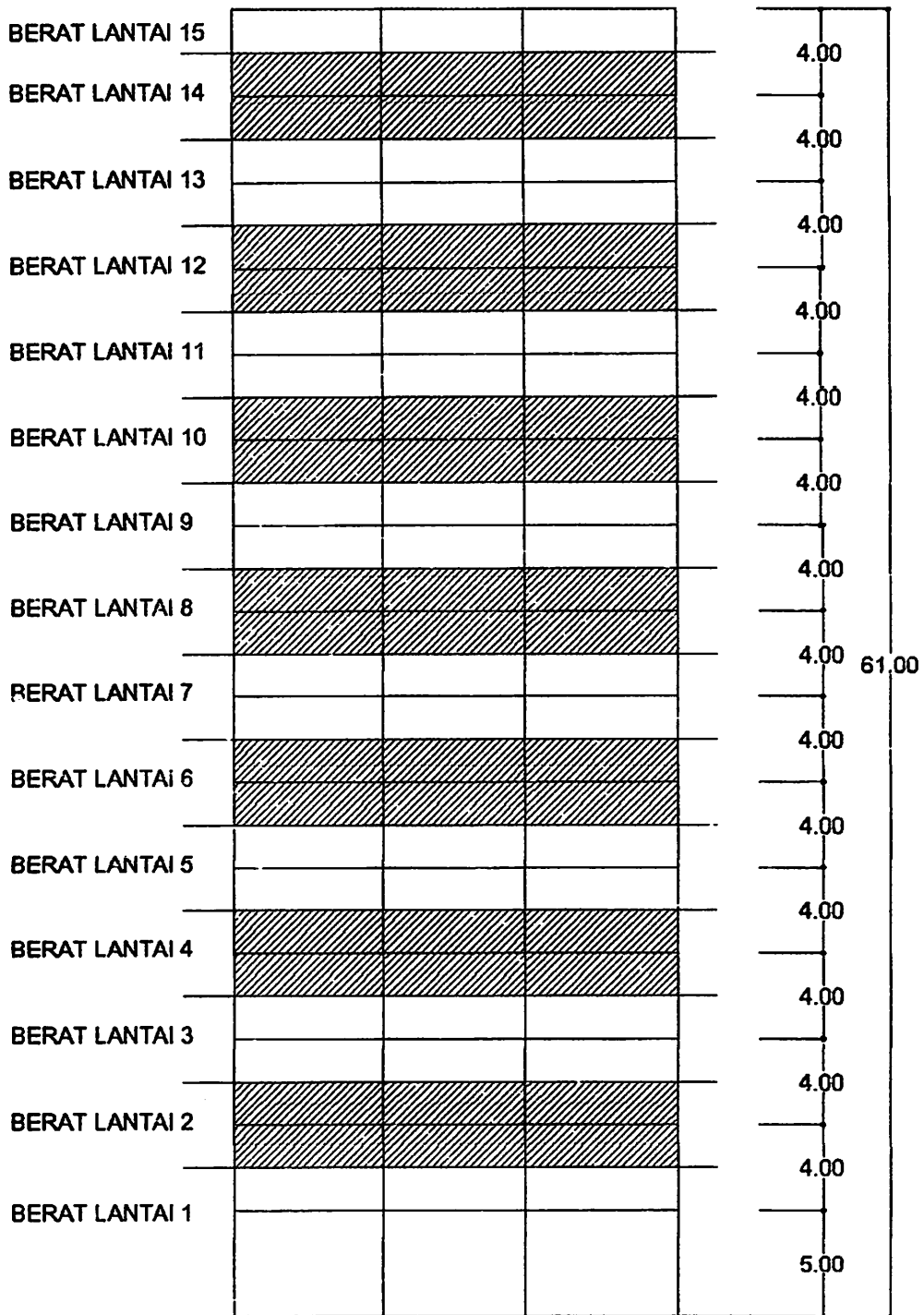
- Beban hidup Lantai	=	57.75	x	15.35	x	250				=	221,615.63	kg	
												221,615.63	kg

**- Beban Total**

**= 1,387,777.65 kg**

**• Tabel Berat sendiri Gedung**

NO	TINGKAT	Zi ( m )	Wi ( kg )
1	15 ( Atap )	61	771,329.78
2	14	57	1,309,512.90
3	13	53	1,309,512.90
4	12	49	1,309,512.90
5	11	45	1,309,512.90
6	10	41	1,309,512.90
7	9	37	1,309,512.90
8	8	33	1,309,512.90
9	7	29	1,309,512.90
10	6	25	1,309,512.90
11	5	21	1,309,512.90
12	4	17	1,309,512.90
13	3	13	1,309,512.90
14	2	9	1,309,512.90
15	1	5	1,387,777.65
	Σ		19,182,775.13



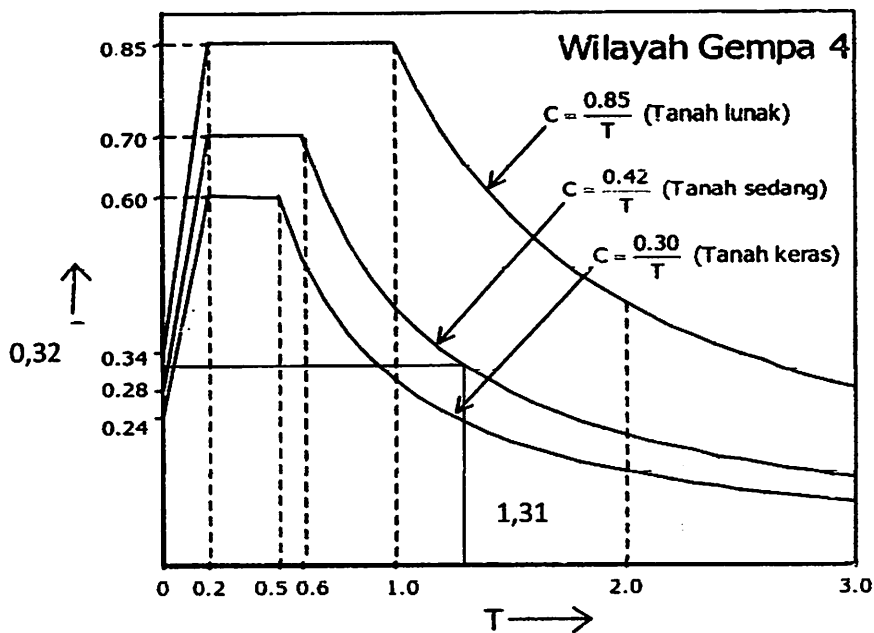
**Gambar 3.4. Pembagian berat per lantai**

## 2. Waktu Getar Gempa

Dari SNI – 1726 – 2002

Untuk Daerah Malang masuk pada wilayah gempa 4 maka didapat data :

1. Faktor Keutamaan Gedung ( I ) = 1
2. Parameter Daktilitas Struktur Gedung  $\mu = 2,0$  ( R ) = 3,2
3. Percepatan puncak muka tanah sedang (  $A_0$  ) = 0,28
4. Spektrum respon gempa rencana Tanah Sedang (  $T_c$  ) = 0,6 detik
5. (  $A_r$  ) Tanah Sedang = 0,42
6. (  $A_m$  ) Tanah Sedang = 0,70
7. Koefisien waktu getar alami fundamental gedung (  $\zeta$  ) = 0,17



**Gambar 3.5. Respon Spektrum Gempa rencana Wilayah 4**

Dengan menetapkan percepatan respons maksimum  $A_m$  sebesar :

$$\begin{aligned} A_m &= 2,5 \times A_0 \\ &= 2,5 \times 0,28 \\ &= 0,7 \end{aligned}$$

$$\begin{aligned} \text{Rumus Empiris} &= 0,06 \times H^{3/4} & \text{Dimana : } T &= \text{Waktu getar ( det )} \\ &= 0,06 \times 61^{3/4} & H &= \text{Ketinggian sampai puncak ( m )} \\ &= 1,310 \text{ detik} \end{aligned}$$

➤ Untuk  $T > T_c = 1,310 > 0,6$

$$C = A_r / T \qquad A_r = A_m \times T_c$$

$$C = 0,32 \qquad = 0,42$$

➤ Pembatas waktu getar alami fundamental

- $T = \zeta \times n \qquad n = 15 \text{ Lantai}$   
 $= 0,170 \times 15 = 2,55$

- Syarat  $T_1 < T$

$$T_1 = 1,310 < 2,55 \qquad \text{dipakai} = 1,310$$

- $V = \frac{C \times I}{R} \times W_t \qquad (\text{SNI} - 1726 - 2002)$

Dimana :  $V = \text{Gaya geser horizontal akibat gempa}$

$I = \text{Faktor keutamaan gedung}$

$R = \text{Daktail Parsial}$

$C = \text{Didapat dari diagram, tanah sedang } C = 0,32$

$W_t = \text{Berat bangunan Total}$

$$\begin{aligned} V &= \frac{0,321 \times 1}{3,2} \times 19.182.775,13 \\ &= 1.922.481,286 \text{ kg} \end{aligned}$$

- $F_i = \frac{W_i \times Z_i}{\sum W_i \times Z_i} \times V$  (SNI – 1726 – 2002)

➤  $\sum W_i \times Z_i = W_1 \times 5 + W_2 \times 9 + W_3 \times 13 + W_4 \times 17 + W_5 \times 21 + W_6 \times 25 + W_7 \times 29 + W_8 \times 33 + W_9 \times 37 + W_{10} \times 41 + W_{11} \times 45 + W_{12} \times 49 + W_{13} \times 53 + W_{14} \times 57 + W_{15} \times 61$

$= 19,182,775.13 \text{ kg}$

$F_i = \frac{1,387,777.65 \times 5}{19,182,775.13} \times 1.922.481,286$

$= 21.663,71 \text{ kg}$

➤ Tabel Perhitungan

NO	TINGKAT	Zi ( m )	Wi ( kg )	Wi x Zi ( t m )	Fi X,Y	
					Wi . Zi	30% Fi
					$\frac{\quad}{\sum W_i \times Z_i} \times V$	
1	15 ( Atap )	61	771,329.78	47,051,116.28	146,896.96	44,069.09
2	14	57	1,309,512.90	74,642,235.30	233,038.40	69,911.52
3	13	53	1,309,512.90	69,404,183.70	216,684.83	65,005.45
4	12	49	1,309,512.90	64,166,132.10	200,331.26	60,099.38
5	11	45	1,309,512.90	58,928,080.50	183,977.69	55,193.31
6	10	41	1,309,512.90	53,690,028.90	167,624.12	50,287.23
7	9	37	1,309,512.90	48,451,977.30	151,270.54	45,381.16
8	8	33	1,309,512.90	43,213,925.70	134,916.97	40,475.09
9	7	29	1,309,512.90	37,975,874.10	118,563.40	35,569.02
10	6	25	1,309,512.90	32,737,822.50	102,209.83	30,662.95
11	5	21	1,309,512.90	27,499,770.90	85,856.25	25,756.88
12	4	17	1,309,512.90	22,261,719.30	69,502.68	20,850.80
13	3	13	1,309,512.90	17,023,667.70	53,149.11	15,944.73
14	2	9	1,309,512.90	11,785,616.10	36,795.54	11,038.66
15	1	5	1,387,777.65	6,938,888.25	21,663.71	6,499.11
$\Sigma$			<b>19,182,775.13</b>	<b>615,771,038.63</b>		

### 3.4.2 Analisis Statika Pada STAAD PRO 2004

#### 1. In put beban

- **Beban Mati**

Untuk memasukkan beban mati pada STAAD PRO menggunakan *Selfweight* sebesar *-1*. *Selfweight* adalah berat sendiri bangunan tersebut.

- **Beban Hidup**

Sesuai PPI'87 beban hidup pada atap sebesar  $100 \text{ kg/m}^2$  sedangkan pada lantai sebesar  $250 \text{ kg/m}^2$ .

- **Beban Gempa**

Beban gempa menggunakan metode Statik Ekuivalen. Beban gempa diletakkan secara horisontal pada titik pusat massa gedung setiap lantai, koordinat pusat massa setiap lantai didapatkan dari program STAAD Pro dengan perintah *CG*, yang tertera pada tabel dibawah.

No	Lantai	Koordinat Global		
		x	Y	Z
1	15	29.03	61	7.68
2	14	28.79	57	7.67
3	13	28.79	53	7.67
4	12	28.79	49	7.67
5	11	28.79	45	7.67
6	10	28.79	41	7.67
7	9	28.79	37	7.67
8	8	28.79	33	7.67
9	7	28.79	29	7.67
10	6	28.79	25	7.67
11	5	28.79	21	7.67
12	4	28.79	17	7.67
13	3	28.79	13	7.67
14	2	28.79	9	7.67
15	1	28.56	5	7.67

- **Kombinasi Pembebanan**

Kombinasi pembebanan yang digunakan diambil dari SNI 03-2847-

2002 pasal 3.2.2 hal 13, antara lain :

1.  $1,4 \text{ DL}$
2.  $1,2 \text{ DL} + 1,6 \text{ LL}$
3.  $1,2 \text{ DL} + 1 \text{ LL} + 1,05 \text{ E}$
4.  $1,2 \text{ DL} + 1 \text{ LL} - 1,05 \text{ E}$
5.  $0,9 \text{ DL} + 1 \text{ E}$
6.  $0,9 \text{ DL} - 1 \text{ E}$



Tabel Momen Dan Gaya Geser

Lt.	No. Joint	Kombinasi 1				Kombinasi 2			
		Fx	Fz	My	Mz	Fx	Fz	My	Mz
		Kg	Kg	Kgm	kgm	Kg	Kg	kgm	kgm
15	311	-3334.249	-1233.358	-1287.40	-3297.80	-3980.88	-1483.4	-1546.90	-4189.90
	301	3865.844	1616.914	-1830.60	-3350.40	3468.08	1823.76	-2067.60	-4257.80
14	301	-13547.22	-2149.27	-2307.60	-3595.80	-14517.7	-2414.9	-2599.20	-4576.20
	291	12478.49	1558.486	-1788.70	-3517.70	12287.7	1785.51	-2044.30	-4474.70
13	291	-21952.92	-2233.49	-2371.40	-3488.70	-23116.7	-2482.3	-2649.50	-4434.60
	281	20907.58	1406.548	-1662.20	-3475.20	20906.4	1633.61	-1918.40	-4416.50
12	281	-30352.43	-2374.692	-2486.80	-3444.90	-31692.9	-2620.7	-2761.00	-4379.90
	271	29365.09	1237.906	-1519.10	-3419.70	29562.2	1459.21	-1770.20	-4344.70
11	271	-38775.4	-2528.411	-2612.50	-3376.90	-40304.4	-2775.9	-2886.50	-4291.20
	261	37850.29	1062.898	-1369.10	-3345.30	38257.9	1276.54	-1613.00	-4246.90
10	261	-47213.81	-2683.75	-2740.20	-3292.30	-48938.9	-2933	-3014.60	-4180.80
	251	46337.22	886.416	-1217.00	-3253.30	46958.6	1091.96	-1453.00	-4125.90
9	251	-55636.87	-2835.873	-2865.40	-3189.10	-57555.5	-3085.9	-3139.20	-4045.60
	241	54800.62	709.554	-1063.90	-3142.80	55630.7	906.825	-1291.70	-3980.30
8	241	-64020.45	-2983.728	-2986.70	-3067.80	-66122.1	-3233.2	-3258.80	-3886.20
	231	63224	532.686	-910.30	-3013.90	64252.5	721.472	-1129.40	-3810.20
7	231	-72350.22	-3126.907	-3103.60	-2927.60	-74619.5	-3374.6	-3372.70	-3701.80
	221	71600.87	356.478	-756.40	-2865.90	72814	536.562	-966.60	-3614.60
6	221	-80622.53	-3264.656	-3215.20	-2768.40	-83041.1	-3509.2	-3480.10	-3491.90
	211	79938.75	182.03	-602.80	-2698.90	81321	353.336	-803.60	-3393.50
5	211	-88848.69	-3395.9	-3320.80	-2589.10	-91396.8	-3635.8	-3579.90	-3255.20
	201	88265.46	9.811	-449.20	-2510.30	89802	172.531	-640.50	-3143.80
4	201	-97066.64	-3519.944	-3419.90	-2391.90	-99726.1	-3753.4	-3671.40	-2994.40
	191	96685.74	-163.267	-290.50	-2308.00	98367.9	-8.64	-472.20	-2875.10
3	191	-105481.1	-3636.235	-3519.00	-2169.00	-108243	-3861	-3761.00	-2698.40
	181	105560.7	-458.964	60.80	-2062.70	107398	-314.84	-106.60	-2548.10
2	181	-112532.1	-4091.987	-3881.80	-1953.80	-115368	-4313.6	-4118.40	-2414.50
	171	104981.8	-657.957	-272.50	-1922.70	106789	-525.25	-435.00	-2363.40
1	171	-111373.3	-2628.097	-3118.10	-1278.40	-114078	-2782.6	-3311.10	-1563.30
	161	106534.8	-2547.429	2039.90	-430.20	108178	-2573.2	2055.10	-535.00

Lt.	No. Joint	Kombinasi 3				Kombinasi 4			
		Fx	Fz	My	Mz	Fx	Fz	My	Mz
		Kg	Kg	Kgm	kgm	Kg	Kg	kgm	kgm
15	160	16239.64	9644.79	12665.00	2407.30	-23359.19	-12291.85	-15426.20	-9764.60
	150	-10169.66	-14634.72	15434.90	2495.60	16989.95	17953.87	-19196.20	-9971.70
14	150	40466.20	15387.77	17378.10	3576.30	-67322.22	-19788.13	-22110.60	-11608.10
	140	-32790.42	-13786.89	14420.10	3061.90	56171.88	17020.66	-18125.40	-10916.60
13	140	62931.80	16951.19	19497.60	4490.30	-105940.29	-21469.90	-24334.00	-12276.20
	130	-43569.53	-14170.83	14787.20	3982.00	83143.14	17117.05	-18253.70	-11736.80
12	130	71922.48	17781.62	20775.20	5369.20	-131050.85	-22584.09	-25825.00	-13058.70
	120	-39714.09	-14984.89	15836.20	4950.60	95544.35	17604.70	-19025.40	-12579.80
11	120	66225.68	18282.25	21483.20	6253.70	-141533.17	-23377.51	-26770.80	-13788.50
	110	-22091.60	-15890.93	17203.30	5894.80	94246.31	18169.89	-20099.60	-13354.00
10	110	46907.55	18578.18	21726.70	7094.50	-138432.95	-23969.73	-27256.50	-14437.00
	100	7921.87	-16710.91	18680.90	6768.00	80564.53	18645.70	-21279.50	-14016.80
9	100	15152.08	18660.67	21504.10	7850.90	-122862.97	-24341.10	-27270.20	-14958.20
	90	49326.16	-17341.38	20144.40	7532.10	55441.21	18931.05	-22443.00	-14527.90
8	90	-28317.32	18458.86	20753.40	8483.70	-95491.32	-24418.48	-26747.00	-15313.60
	80	101599.58	-17700.51	21498.30	8150.30	19359.99	18944.79	-23495.30	-14850.50
7	80	-83253.18	17887.87	19392.30	8956.90	-56532.09	-24116.23	-25603.40	-15466.20
	70	164651.29	-17709.62	22657.10	8587.50	-27604.70	18609.48	-24351.60	-14948.10
6	70	-149829.09	16861.74	17330.80	9234.20	-5801.12	-23346.93	-23747.90	-15378.80
	60	238795.50	-17289.51	23541.30	8805.70	-85755.05	17848.20	-24933.30	-14782.60
5	60	-228624.66	15294.05	14470.40	9280.20	57261.60	-22021.87	-21080.10	-15013.60
	50	324760.49	-16367.94	24083.90	8774.60	-155765.86	16589.92	-25173.30	-14318.00
4	50	-320637.86	13081.50	10689.60	9039.50	133580.22	-20036.09	-17477.30	-14320.20
	40	423681.13	-14897.19	24244.70	8421.00	-238566.09	14781.43	-25021.60	-13498.60
3	40	-427294.15	10089.18	5822.50	8482.80	224181.15	-17253.04	-12786.00	-13250.20
	30	537213.49	-12986.79	24154.90	7852.10	-335105.91	12298.20	-24249.00	-12363.30
2	30	-550149.39	6015.97	-521.20	7260.80	333597.47	-14038.55	-7122.20	-11534.90
	20	668540.04	-11152.40	24409.40	6367.70	-467565.08	10072.87	-25128.30	-10557.90
1	20	-703894.57	1406.06	-10328.70	4842.30	489699.94	-6573.76	4185.40	-7618.20
	10	828221.23	-21705.96	36671.20	8426.00	-624512.50	16851.78	-32790.90	-9371.30

Lt.	No. Joint	Kombinasi 5				Kombinasi 6			
		Fx	Fz	My	Mz	Fx	Fz	My	Mz
		Kg	Kg	kgm	kgm	Kg	Kg	kgm	kgm
15	311	17655.97	10175.45	13218.00	3965.90	-21942.86	-11761.20	-14873.20	-8205.90
	301	-11094.62	-15254.85	16138.80	4079.80	16064.99	17333.74	-18492.40	-8387.50
14	301	45185.29	16206.28	18260.90	5280.60	-62603.14	-18969.62	-21227.90	-9903.80
	291	-36459.26	-14401.89	15122.80	4727.80	52503.04	16405.66	-17422.60	-9250.60
13	291	70323.45	17784.73	20391.30	6140.50	-98548.63	-20656.36	-23440.30	-10626.00
	281	-49915.75	-14739.73	15451.90	5625.30	76796.91	16548.15	-17589.00	-10093.50
12	281	81974.39	18656.27	21701.50	6999.40	-120998.95	-21709.44	-24898.70	-11428.50
	271	-48751.66	-15499.00	16454.30	6566.80	86506.78	17090.59	-18407.40	-10963.60
11	271	78952.39	19204.47	22447.50	7850.20	-128806.48	-22455.29	-25806.50	-12192.00
	261	-33836.63	-16347.11	17771.30	7473.80	82501.28	17713.70	-19531.60	-11774.90
10	261	62318.52	19548.69	22730.10	8649.30	-123021.99	-22999.22	-26253.20	-12882.20
	251	-6533.13	-17108.46	19197.90	8301.00	66109.55	18248.14	-20762.60	-12483.80
9	251	33240.97	19677.82	22545.10	9354.40	-104774.07	-23323.94	-26229.20	-13454.70
	241	32171.45	-17680.07	20609.80	9009.60	38286.49	18592.35	-21977.70	-13050.40
8	241	-7569.01	19520.56	21830.10	9926.50	-74743.01	-23356.78	-25670.20	-13870.80
	231	81763.80	-17980.21	21911.60	9562.90	-475.80	18665.09	-23082.00	-13437.90
7	231	-59871.40	18991.89	20502.70	10329.50	-33150.31	-23012.20	-24493.00	-14093.60
	221	142157.13	-17930.39	23018.10	9925.40	-50098.86	18388.71	-23990.60	-13610.20
6	221	-123842.75	18005.63	18472.40	10526.80	20185.21	-22203.05	-22606.30	-14086.20
	211	213664.45	-17451.84	23849.80	10059.20	-110886.08	17685.88	-24624.80	-13529.10
5	211	-200060.16	16474.88	15640.40	10482.50	85826.13	-20841.04	-19910.00	-13811.30
	201	297005.24	-16472.62	24339.80	9932.50	-183521.09	16485.24	-24917.30	-13160.10
4	201	-289509.01	14295.97	11884.90	10142.10	164709.06	-18821.61	-16281.90	-13217.50
	191	393278.73	-14944.26	24446.40	9476.10	-268968.50	14734.35	-24819.90	-12443.50
3	191	-393546.95	11333.53	7042.10	9472.10	257928.35	-16008.69	-11566.50	-12260.80
	181	504020.18	-12937.54	24241.10	8781.70	-368299.21	12347.45	-24162.90	-11433.70
2	181	-514215.50	7396.70	805.10	8141.80	369531.36	-12657.82	-5795.90	-10653.90
	171	635540.87	-11035.61	24593.70	7226.80	-500564.25	10189.66	-24944.00	-9698.80
1	171	-668394.40	2300.42	-9261.60	5408.40	525200.16	-5679.40	5252.60	-7052.10
	161	794853.50	-20916.50	36042.40	8622.10	-657880.22	17641.24	-33419.70	-9175.20

Tabel Momen Dan Gaya Geser Maksimum

Lt.	No. Joint	Mz		My		Fz		Fx	
		(-)	(+)	(-)	(+)	(-)	(+)	(-)	(+)
		kgm	kgm	kgm	kgm	Kg	Kg	Kg	Kg
15	311	9764.60	3965.90	15426.20	13218.00	12291.85	10175.45	23359.19	17655.97
	301	9971.70	4079.80	19196.20	16138.80	15254.85	17953.87	11094.62	16989.95
14	301	11608.10	5280.60	22110.60	18260.90	19788.13	16206.28	67322.22	45185.29
	291	10916.60	4727.80	18125.40	15122.80	14401.89	17020.66	36459.26	56171.88
13	291	12276.20	6140.50	24334.00	20391.30	21489.90	17784.73	105940.29	70323.45
	281	11736.80	5625.30	18253.70	15451.90	14739.73	17117.05	49915.75	83143.14
12	281	13058.70	6999.40	25825.00	21701.50	22584.09	18656.27	131050.85	81974.39
	271	12579.80	6566.80	19025.40	16454.30	15499.00	17604.70	48751.66	95544.35
11	271	13788.50	7850.20	26770.80	22447.50	23377.51	19204.47	141533.17	78952.39
	261	13354.00	7473.80	20099.60	17771.30	16347.11	18169.89	33836.63	94246.31
10	261	14437.00	8649.30	27256.50	22730.10	23969.73	19548.69	138432.95	62318.52
	251	14016.80	8301.00	21279.50	19197.90	17108.46	18645.70	6533.13	80564.53
9	251	14958.20	9354.40	27270.20	22545.10	24341.10	19677.82	122862.97	33240.97
	241	14527.90	9009.60	22443.00	20609.80	17680.07	18931.05	-	55630.72
8	241	15313.60	9926.50	26747.00	21830.10	24418.48	19520.56	95491.32	-
	231	14850.50	9562.90	23495.30	21911.60	17980.21	18944.79	475.80	101599.58
7	231	15466.20	10329.50	25603.40	20502.70	24116.23	18991.89	83253.18	-
	221	14948.10	9925.40	24351.60	23018.10	17930.39	18609.48	50098.86	164651.29
6	221	15378.80	10526.80	23747.90	18472.40	23346.93	18005.63	149829.09	20185.21
	211	14782.60	10059.20	24933.30	23849.80	17451.84	17848.20	110886.08	238795.50
5	211	15013.60	10482.50	21080.10	15640.40	22021.87	16474.88	228624.66	85826.13
	201	14318.00	9932.50	25173.30	24339.80	16472.62	16589.92	183521.09	324760.49
4	201	14320.20	10142.10	17477.30	11884.90	20036.09	14295.97	320637.86	164709.06
	191	13498.60	9476.10	25021.60	24446.40	14944.26	14781.43	268968.50	423681.13
3	191	13250.20	9472.10	12786.00	7042.10	17253.04	11333.53	427294.15	257928.35
	181	12363.30	8781.70	24249.00	24241.10	12986.79	12347.45	368299.21	537213.49
2	181	11534.90	8141.80	7122.20	805.10	14038.55	7396.70	550149.39	369531.36
	171	10557.90	7226.80	25128.30	24593.70	11152.40	10189.66	500564.25	668540.04
1	171	7618.20	5408.40	10328.70	5252.60	6573.76	2300.42	703894.57	525200.16
	161	9371.30	8622.10	33419.70	36671.20	21705.96	17641.24	657880.22	828221.23

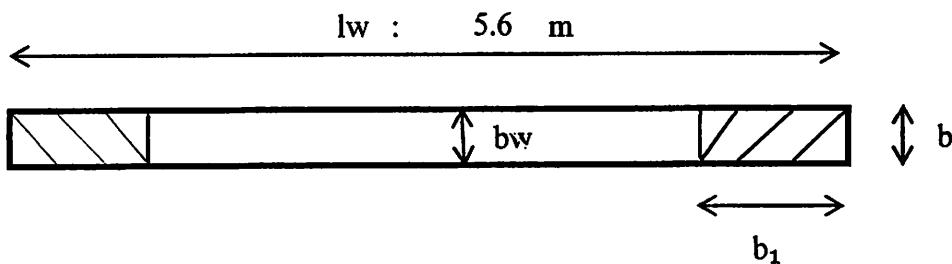
## BAB IV PENULANGAN DINDING GESER

### 4.1 Perhitungan Penulangan Dinding Geser

#### Data Perencanaan

- Kuat Tekan Beton ( $f_c$ ) : 30 Mpa
- Kuat leleh baja ( $f_y$ ) : 300 Mpa
- Faktor reduksi kekuatan
  - lentur dan tekan aksial  $\Phi$  : 0.65
  - Geser  $\Phi$  : 0.60

Luas penampang dinding ges :  $5600 \times 350 = 1960000 \text{ mm}^2$



- $bc = 0.017 \times lw \times \sqrt{\frac{\mu\Phi}{5}}$ 
  - $= 0.017 \times 5600 \times \sqrt{\frac{\mu\Phi}{5}}$
  - $= 214.1259 \text{ mm}$
- $b_1 \geq \frac{bc \times lw}{10 \cdot b}$ 
  - $\geq \frac{214.1 \times 5600}{3125}$
  - $\geq 383.71 \text{ mm}$
- $b \geq h_1/16$ 
  - $\geq 312.5 \text{ mm}$
- $b_1 \geq \frac{bc^2}{b}$ 
  - $\geq \frac{214.126^2}{350}$
  - $\geq 131 \text{ mm}$
- $bw \geq b \geq bc$ 
  - $350 \geq 312.5 \geq 214.12587$
- Jadi dimensi yang dipakai pada dinding geser pada bagian ujung
  - $b = 350$                        $b_1 = 384$

# 1. Penulangan pada segmen 1

## a. Penulangan Vertikal

$$M_u = 80088.00 \text{ kgm} = 8008800 \text{ kgcm}$$

$$P_u = 525200.16 \text{ kg}$$

$$M_n = \frac{M_u}{\Phi} = \frac{8008800}{0.65} = 12321230.77 \text{ kgcm}$$

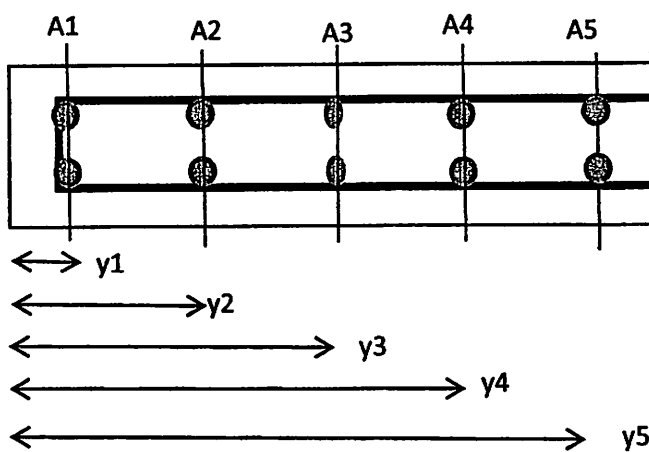
$$P_n = \frac{P_u}{\Phi} = \frac{525200.2}{0.65} = 808000.2462 \text{ kg}$$

$$l_w = 5.6 \text{ m}$$

Pendekatan pertama di misalkan  $d = 512.4 \text{ cm}$

$$A_v = \frac{M_n}{f_y \times d} = \frac{12321230.77}{3000 \times 512.4} = 8.015373 \text{ cm}^2$$

Dicoba tulangan 10 D 13  $A_s = 13.279 \text{ cm}^2$



$$y_1 = 7.6 \quad y_3 = 27.6 \quad y_5 = 47.6$$

$$y_2 = 17.6 \quad y_4 = 37.6$$

$$A = \left( \frac{1}{4} \times 3.14 \times 1.3^2 \right) \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A_1 \times y_1) + (A_2 \times y_2) + (A_3 \times y_3) + (A_4 \times y_4) + (A_5 \times y_5)}{A_1 + A_2 + A_3 + A_4 + A_5}$$

$$= 27.6 \text{ cm}$$

$$d = 560 - 27.6 = 532.4 \text{ cm}$$

Untuk rasio penulangan pada dinding geser berpedoman pada SNI03-2847-2002 pasal 23.6.2.(1)

$$V_u < \frac{1}{12} \times A_{cv} \times \sqrt{f_c}$$

$$< 894613.511 \text{ N}$$

Dimana :

$$A_{cv} = \text{Luas bruto penampang}$$

$$= 1960000 \text{ mm}^2$$

$$V_u = 21705.958 \text{ kg}$$

$$= 217059.58 \text{ N}$$

Karena  $V_u = 217059.58 \text{ N} < 894613.5106 \text{ N}$   
maka rasio penulangan untuk dinding geser adalah :

$$\rho > \rho_{min} = 0.0025$$

Jika dalam perhitungan dicoba menggunakan  $\rho_{min} = 0.00250$

Sehingga luas penampang yang diperlukan :

$$A_s = \rho \times b \times d$$

$$= 0.0025 \times 35 \times 532.4$$

$$= 46.585 \text{ cm}^2$$

$$\text{Dipasang tulangan untuk bagian tengah } 46 \text{ D } 13 = 61.081 \text{ cm}^2$$

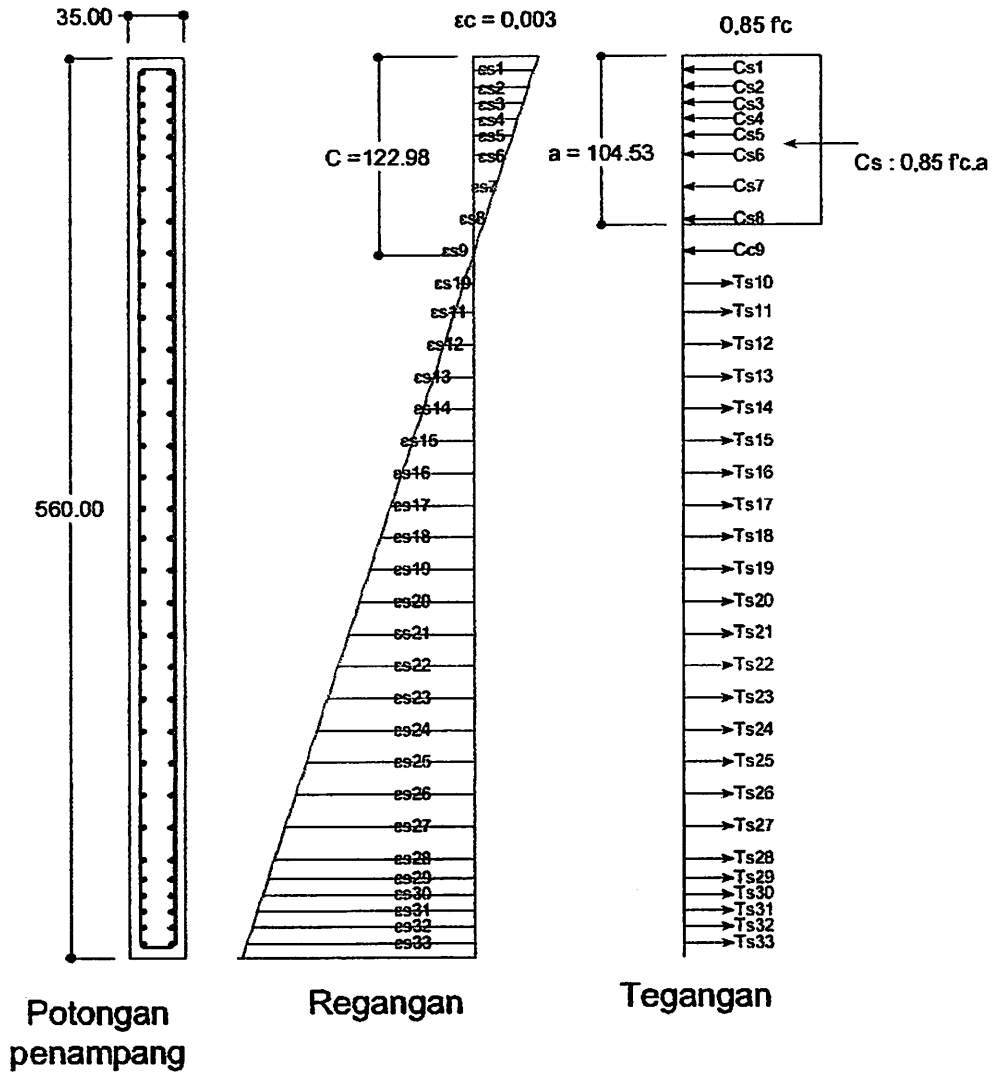
Cek  $\rho$  terpasang

$$\rho = \frac{A_s}{b.d} = \frac{61.081}{35 \times 532.4} = 0.00328$$

$$\rho > \rho_{min}$$

$$0.00328 > 0.00250 \text{ ..... OK}$$

dicoba nilai sampai memenuhi  $C_s + C_c = T_s + P_n$  sehingga  
dari hasil trial and error, didapat nilai  $c = 1229.809 \text{ mm}$



Gambar 4.1. Diagram tegangan dan regangan arah x



Tabel 4.1. Tabel jarak tulangan terhadap serat atas penampang

di	jarak cm	di	jarak cm	di	jarak cm
d1	7.6	d12	180	d23	400
d2	17.6	d13	200	d24	420
d3	27.6	d14	220	d25	440
d4	37.6	d15	240	d26	460
d5	47.6	d16	260	d27	480
d6	60	d17	280	d28	500
d7	80	d18	300	d29	512.4
d8	100	d19	320	d30	522.4
d9	120	d20	340	d31	532.4
d10	140	d21	360	d32	542.4
d11	160	d22	380	d33	552.4

Jarak masing-masing tulangan terhadap tengah-tengah penampang

Tabel 4.2. Tabel Jarak tulangan tengah penampang

y	jarak cm	y	jarak cm	y	jarak cm
y1	272.4	y12	100	y23	120
y2	262.4	y13	80	y24	140
y3	252.4	y14	60	y25	160
y4	242.4	y15	40	y26	180
y5	232.4	y16	20	y27	200
y6	220	y17	0	y28	220
y7	200	y18	20	y29	232.4
y8	180	y19	40	y30	242.4
y9	160	y20	60	y31	252.4
y10	140	y21	80	y32	262.4
y11	120	y22	100	y33	272.4

Menghitung regangan yang terjadi :

Tabel 4.3. Tabel regangan

εs	Nilai	εs	Nilai	εs	Nilai
εs1	0.00281	εs12	0.00139	εs23	0.00676
εs2	0.00257	εs13	0.00188	εs24	0.00725
εs3	0.00233	εs14	0.00237	εs25	0.00773
εs4	0.00208	εs15	0.00285	εs26	0.00822
εs5	0.00184	εs16	0.00334	εs27	0.00871
εs6	0.00154	εs17	0.00383	εs28	0.00920
εs7	0.00105	εs18	0.00432	εs29	0.00950
εs8	0.00056	εs19	0.00481	εs30	0.00974
εs9	0.00007	εs20	0.00529	εs31	0.00999
εs10	0.00042	εs21	0.00578	εs32	0.01023
εs11	0.00090	εs22	0.00627	εs33	0.01048

Untuk daerah tekan :

$$\frac{\epsilon_{s1}}{\epsilon_s} = \frac{c - d1}{c} \Rightarrow \epsilon_{s1} = \frac{c - d1}{c} \times \epsilon_c \quad ; \epsilon_c = 0,003$$

$$= \frac{122.9809 - 7.6}{122.9809} \times 0.003$$

$$= 0.002814605$$

Untuk daerah tarik :

$$\frac{\epsilon_{s10}}{\epsilon_s} = \frac{d10 - c}{c} \Rightarrow \epsilon_{s10} = \frac{d10 - c}{c} \times \epsilon_c \quad ; \epsilon_c = 0,003$$

$$= \frac{140 - 122.9809}{122.9809} \times 0.003$$

$$= 0.000415164$$

**Mencari nilai fs :**

Tabel 4.4. Tabel nilai fs

fs	Mpa	fs	Mpa	fs	Mpa
fs1	562.9211	fs12	278.18502	fs23	1351.522
fs2	514.133	fs13	375.76113	fs24	1449.098
fs3	465.345	fs14	473.33725	fs25	1546.674
fs4	416.5569	fs15	570.91336	fs26	1644.251
fs5	367.7689	fs16	668.48947	fs27	1741.827
fs6	307.2717	fs17	766.06559	fs28	1839.403
fs7	209.6955	fs18	863.6417	fs29	1899.9
fs8	112.1194	fs19	961.21781	fs30	1948.688
fs9	14.54332	fs20	1058.7939	fs31	1997.476
fs10	83.03279	fs21	1156.37	fs32	2046.264
fs11	180.6089	fs22	1253.9462	fs33	2095.052

**Keterangan tabel :**

**Untuk daerah tekan**

$$fs = \epsilon_s \times E_s$$

$$fs1 = 0.00281 \times 200000 = 562.92108 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan fs = 300 Mpa

$$fs9 = 0.00007 \times 200000 = 14.54332 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan fs = 14.54332 Mpa

**Untuk daerah tarik**

$$fs = \epsilon_s \times E_s$$

$$fs10 = 0.00042 \times 200000 = 83.032794 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan fs = 83.03279 Mpa

$$fs13 = 0.00188 \times 200000 = 375.76113 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan fs = 300 Mpa

Besarnya gaya-gaya yang bekerja :

Tabel 4.5. Tabel nilai Ts dan Cs

N	kN	N	kN	N	kN
Cs1	79.67143	Ts12	73.877993	Ts23	79.67143
Cs2	79.67143	Ts13	79.671429	Ts24	79.67143
Cs3	79.67143	Ts14	79.671429	Ts25	79.67143
Cs4	79.67143	Ts15	79.671429	Ts26	79.67143
Cs5	79.67143	Ts16	79.671429	Ts27	79.67143
Cs6	79.67143	Ts17	79.671429	Ts28	79.67143
Cs7	55.68915	Ts18	79.671429	Ts29	79.67143
Cs8	29.77572	Ts19	79.671429	Ts30	79.67143
Cs9	3.86229	Ts20	79.671429	Ts31	79.67143
Ts10	22.05114	Ts21	79.671429	Ts32	79.67143
Ts11	47.96457	Ts22	79.671429	Ts33	79.67143

Keterangan tabel :

Untuk daerah tekan :

$$C_s = A_s \times f_s$$

$$C_{s1} = (2 \times 0,25 \times 22/7 \times 16) \times 300 = 79671.42857 \text{ N} \\ = 79.67142857 \text{ kN}$$

$$C_{s9} = (2 \times 0,25 \times 22/7 \times 16) \times 14.54 = 3862.290173 \text{ N} \\ = 3.862290173 \text{ kN}$$

Untuk daerah tarik :

$$T_s = A_s \times f_s$$

$$T_{s10} = (2 \times 0,25 \times 22/7 \times 13) \times 83.03 = 22051.13766 \text{ N} \\ = 22.05113766 \text{ kN}$$

$$T_{s13} = (2 \times 0,25 \times 22/7 \times 13) \times 300 = 79671.42857 \text{ N} \\ = 79.67142857 \text{ kN}$$

$$C_c = 0,85 \cdot f_c \cdot \beta_c \cdot b$$

$$= 0,85 \times 30 \times 0,85 \times 1229.809 \times 350$$

$$= 9329639.89 \text{ N}$$

$$= 9329.63989 \text{ kN}$$

**Kontrol  $\Sigma H = 0$**

$$(Cs1 + Cs2 + \dots + Cs9) + Cc = (Ts10 + Ts17 + \dots + Ts33) + Pn$$

$$567.3557254 + 9329.639892 = 1816.993696 + 8080.002462$$

$$9896.996 \text{ kN} = 9896.996 \text{ kN}$$

sehingga besarnya momen yang terjadi terhadap titik berat penampang :

Tabel 4.6. Tabel nilai Mn

Mn	kNm	N	kNm	N	kNm
Mn1	217.025	Mn12	73.878	Mn23	95.606
Mn2	209.058	Mn13	63.737	Mn24	111.540
Mn3	201.091	Mn14	47.803	Mn25	127.474
Mn4	193.124	Mn15	31.869	Mn26	143.409
Mn5	185.16	Mn16	15.934	Mn27	159.343
Mn6	175.28	Mn17	0.000	Mn28	175.277
Mn7	111.38	Mn18	15.934	Mn29	185.156
Mn8	53.596	Mn19	31.869	Mn30	193.124
Mn9	6.180	Mn20	47.803	Mn31	201.091
Mn10	30.872	Mn21	63.737	Mn32	209.058
Mn11	57.557	Mn22	79.671	Mn33	217.025

Keterangan tabel :

$$Mn1 = Cc1 \times y1$$

$$= 79.67143 \times 272.4 = 21702.497 \text{ kNcm} = 217.02 \text{ kNm}$$

$$Mn2 = Cc2 \times y2$$

$$= 79.67143 \times 262.4 = 20905.783 \text{ kNcm} = 209.06 \text{ kNm}$$

Jika  $c = 1229.8092 \text{ mm}$ , maka  $a = \beta.c$

$$= 0.85 \times 1229.8092$$

$$= 1045.3378 \text{ mm}$$

$$yc = h/2 - a/2$$

$$= 5600 / 2 - 1045.3378 / 2$$

$$= 2277.331 \text{ mm}$$

$$Mn = (Cc \times yc) + (Mn1 + Mn2 + \dots + Mn33)$$

$$= (9329.63989 \times 2.2773311) + 3730.65$$

$$= 24977.3301 \text{ kNm} > Mn = 1232.123077 \text{ kNm}$$



### Kontrol terhadap sumbu X

$$M_u = 18252.000 \text{ kgcm} = 1825200 \text{ Nmm}$$

$$P_u = 525200.16 \text{ kg} = 5252001.6 \text{ N}$$

Kuat Nominal Penampang :

untuk mengetahui nilai c dapat diselesaikan dengan menggunakan persamaan

Jika diketahui data sebagai berikut :

$$A_s' \text{ 33 D 16} = 6637.714286 \text{ mm}^2 \quad f_y = 300 \text{ Mpa}$$

$$A_s \text{ 33 D 16} = 6637.714286 \text{ mm}^2 \quad \beta = 0.85$$

$$d' = 76 \text{ mm} \quad P_u = 525200.16 \text{ kg}$$

$$b = 5600 \text{ mm} \quad = 5252001.6 \text{ N}$$

Maka  $C_c + C_s = T_s + P_n$

$$\text{Dimana : } C_c \text{ (Beton tertekan)} = 0,85 \cdot f_c \cdot a \cdot b \quad ; \quad a = \beta \cdot c$$

$$C_s \text{ (Baja tertekan)} = A_s' (f_s1 - 0,85 \cdot f_c)$$

$$T_s \text{ (Baja tertarik)} = A_s1 \cdot F_y1$$

Momen Nominal yang disumbangkan oleh beton :

$$M_{nd1} = C_c \times \left[ d - \frac{a}{2} \right]$$

$$M_{nd2} = C_s \cdot (d - d')$$

$$M_{nd} = M_{nd1} + M_{nd2} > M_n = \frac{M_u}{\Phi}$$

untuk mendapatkan nilai c, maka :

$$f_s' = \epsilon_s' \cdot E_s = \frac{0,003 (c - d')}{c} \cdot E_s = \frac{600 (c - d')}{c} \quad ; \quad E_s : 200000 \text{ Mpa}$$

Maka :

$$C_c + C_s = T_s + P_u$$

$$(0,85 \cdot f_c \cdot \beta \cdot c \cdot b) + \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f_c \cdot A_s' = A_s \cdot F_y + P_u$$

apabila persamaan tersebut dikalikan c, maka :

$$(0,85 \cdot f_c \cdot \beta \cdot c^2 \cdot b) + ((600 (c - d')).As' - 0,85 \cdot f_c \cdot As' \cdot c) = (As \cdot f_y + P_u) c$$

Setelah dilakukan pengelompokan, maka didapatkan persamaan kuadrat :

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot c \cdot As' - 600 \cdot d' \cdot As' - 0,85 \cdot f_c \cdot As' \cdot c) - (As \cdot f_y \cdot c) - P_u \cdot c = 0$$

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot As' - 0,85 \cdot f_c \cdot As' - As \cdot f_y - P_u) \cdot c - 600 \cdot d' \cdot As' = 0$$

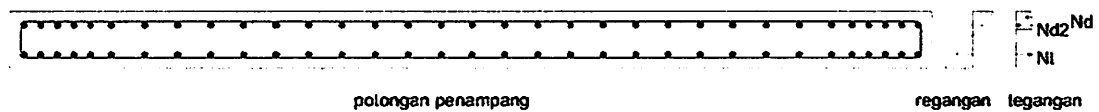
$$121380 \quad c^2 \quad -5421263.314 \quad c \quad - 302679771.4 \quad = 0$$

dari persamaan di atas, di dapatkan nilai c = 77.0343 mm

$$a = \beta \times c = 0.85 \times 77.0343 = 65.4791 \text{ mm}$$

$$\epsilon_y = \frac{f_y}{E_s} = \frac{300}{200000} = 0.00150$$

$$\epsilon_s = 0,003 \cdot \frac{c - d'}{c} = 0.003 \cdot \frac{77.03 - 76}{77.0343} = 0.000040$$



Gambar 4.2. Diagram tegangan dan regangan arah y

Karena  $\epsilon_s > \epsilon_y$ , maka dapat disimpulkan bahwa tulangan leleh meluluh, dengan demikian maka yang digunakan adalah  $f_y = 300 \text{ Mpa}$

Gaya-gaya yang timbul :

$$\begin{aligned} C_c &= 0,85 \cdot f_c \cdot a \cdot b \\ &= 0.85 \times 30 \times 65.479 \times 5600 \\ &= 9350420.407 \text{ N} \end{aligned}$$

$$\begin{aligned} C_s &= \frac{600 (c - d')}{c} \cdot As' - 0,85 \cdot f_c \cdot As' \\ &= 53471.47917 - 169261.7143 = -115790.2351 \text{ N} \end{aligned}$$

$$\begin{aligned} T_s &= As \cdot F_y \\ &= (66 \times 3.14 \times 8^2) \times 300 \\ &= 3982628.571 \text{ N} \end{aligned}$$

**Kontrol :**

$$\begin{aligned} C_c + C_s &= T_s + P_u \\ 9350420.407 + -115790.235 &= 3982628.571 + 5252001.6 \\ 9234630.171 \text{ N} &= 9234630.171 \text{ N} \dots\dots \text{Ok} \end{aligned}$$

$$e = \frac{M_u}{P_u} = \frac{1825200}{5252001.6} = 0.347524647 \text{ mm}$$

sehingga momen nominal yang disumbangkan oleh beton dan baja adalah sebesar :

$$\begin{aligned} M_{nd1} &= C_c \times \left( d - \frac{a}{2} \right) \\ &= 9350420.407 \times \left( 274 - \frac{65.4791}{2} \right) \\ &= 2255886474 \text{ Nmm} \end{aligned}$$

$$\begin{aligned} M_{nd2} &= C_s \cdot (d - d') \\ &= -115790.2351 \times (274 - 76) \\ &= -22926466.55 \text{ Nmm} \end{aligned}$$

$$\begin{aligned} M_{nd} &= M_{nd1} + M_{nd2} \\ &= 2255886474 + -22926466.55 \\ &= 2232960007 \text{ Nmm} \end{aligned}$$

$$M_n = \frac{M_u}{\Phi} = \frac{1825200}{0.65} = 2808000 \text{ Nmm}$$

$$M_{nd} = 2232960007 \text{ Nmm} > M_n = 2808000 \text{ Nmm} \dots\dots \text{Ok}$$



**b. Penulangan Horizontal**

Berdasarkan SNI03-2847-2002 pasal 13.1

$$\Phi V_n \geq V_u$$

$$V_u = 217059.58 \text{ kg} \quad \text{Dimana :}$$

$$\Phi = 0.6 \quad V_c = V \text{ yang disumbangkan oleh beton}$$

$$V_n = V_c + V_s \quad V_s = V \text{ yang disumbangkan tulangan}$$

Berdasarkan SNI03-2847-2002 pasal 13.3.1.(2)

$$\begin{aligned} V_c &= \left[ 1 + \frac{V_u}{14.A_g} \right] \left[ \frac{\sqrt{f_c}}{6} \right] b_w . d \\ &= \left[ 1 + \frac{217059.58}{14 \times 1960000} \right] \left[ \frac{\sqrt{30}}{6} \right] 350 \times 3524 \\ &= 1215000 \text{ N} = 121500 \text{ kg} \end{aligned}$$

$$\begin{aligned} V_s &= \frac{A_v . f_y . d}{S} \\ &= \frac{283.6429 \times 300 \times 3524}{300} \\ &= 999557.429 \text{ N} = 99955.74 \text{ kg} \end{aligned}$$

$$V_n = 121500 + 99955.74286 = 221456 \text{ kg}$$

$$V_n \geq V_u$$

$$221456 \text{ kg} \geq 217059.58 \text{ kg} \dots\dots\dots \text{Ok}$$

Direncanakan tulangan D 19 - 300

$$\begin{aligned} A_v &= 1/4 \times 22/7 \times 19^2 \\ &= 283.642857 \text{ mm}^2 \geq 119.8143095 \text{ mm}^2 \dots\dots\dots \text{OK} \end{aligned}$$

Syarat :

$$\begin{aligned} A_v &\geq \frac{75 \sqrt{f_c} \times b_w \times s}{1200 \times f_y} \\ &\geq \frac{75 \times \sqrt{30} \times 350 \times 300}{1200 \times 300} \\ &\geq 119.814309 \text{ mm}^2 \end{aligned}$$

- c. Berdasarkan buku T. Paulay-M.J.N.Priestley hal 150, panjang sambungan lewatan ls sama dengan ld, sedangkan letak penyaluran dinyatakan dalam Ld.

$$Ld = mdb \times ldb$$

Dimana :

$$ldb = \frac{1.38 \times Ab \times fy}{c \times \sqrt{fc}}$$

$$mdb = \text{Faktor modifikasi} = 1.3$$

$$Ab = \text{Luas tulangan}$$

$$c = 3 \times \text{diameter tulangan}$$

Untuk tulangan D 13

$$\begin{aligned} Ab &= 3.14 \times 7^2 \\ &= 132.7857143 \text{ mm}^2 \end{aligned}$$

$$c = 3 \times 13 = 39 \text{ mm}$$

$$\begin{aligned} ldb &= \frac{1.38 \times 132.78571 \times 300}{39 \times 5.477} \\ &= 257.3513559 \text{ mm} \end{aligned}$$

Jadi untuk :

$$\begin{aligned} Ld &= mdb \times ldb \\ &= 1.3 \times 257.3514 \\ &= 334.5567627 \text{ mm} \end{aligned}$$

## 2 Penulangan pada segmen 2 ( ada bukaan )

### a. Penulangan Vertikal

$$M_u = 24917.30 \text{ kgm} = 2491730 \text{ kgcm}$$

$$P_u = 550149.39 \text{ kg}$$

$$M_n = \frac{M_u}{\Phi} = \frac{2491730}{0.65} = 3833430.769 \text{ kgcm}$$

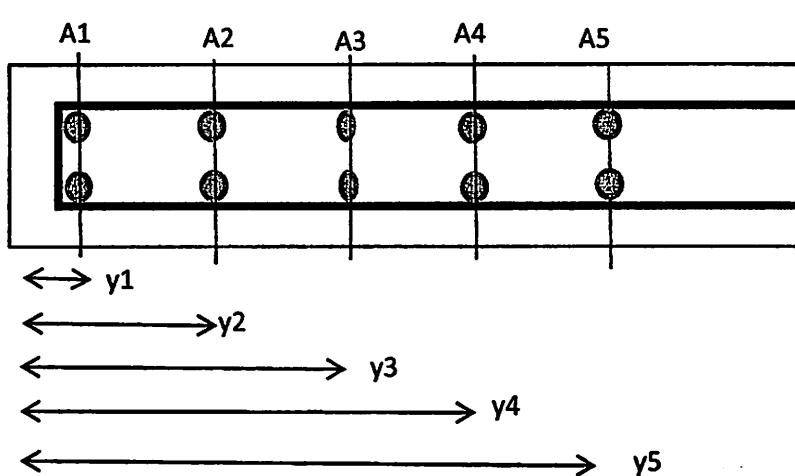
$$P_n = \frac{P_u}{\Phi} = \frac{550149.4}{0.65} = 846383.6769 \text{ kg}$$

$$l_w = 2.1 \text{ m}$$

Pendekatan pertama di misalkan  $d = 162.4 \text{ cm}$

$$A_v = \frac{M_n}{f_y \times d} = \frac{3833430.769}{3000 \times 162.4} = 7.86829 \text{ cm}^2$$

Dicoba tulangan 10 D 13  $A_s = 13.279 \text{ cm}^2$



$$y_1 = 7.6 \quad y_3 = 27.6 \quad y_5 = 47.6$$

$$y_2 = 17.6 \quad y_4 = 37.6$$

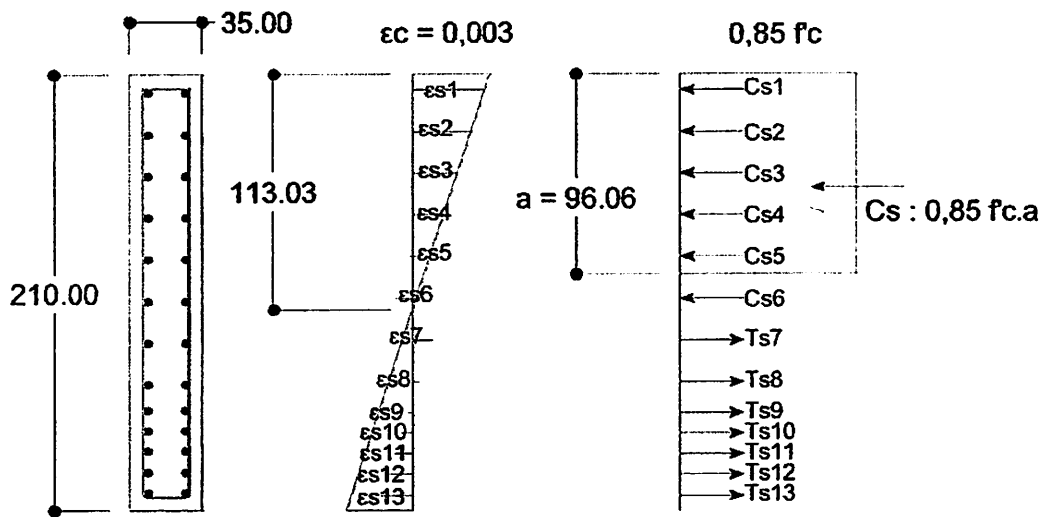
$$A = \left( \frac{1}{4} \times 3.14 \times 1.3^2 \right) \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A_1 \times y_1) + (A_2 \times y_2) + (A_3 \times y_3) + \dots + (A_5 \times y_5)}{A_1 + A_2 + A_3 + \dots + A_5}$$

$$= 27.6 \text{ cm}$$

$$d = 210 - 27.6 = 182.4 \text{ cm}$$



Potongan penampang      Regangan      Tegangan

Gambar 4.1. Diagram tegangan dan regangan

Tabel 4.1. Tabel jarak tulangan terhadap serat atas penampang

di	jarak cm	di	jarak cm	di	jarak cm
d1	7.6	d6	107.6	d11	182.4
d2	27.6	d7	127.6	d12	192.4
d3	47.6	d8	147.6	d13	202.4
d4	67.6	d9	162.4		
d5	87.6	d10	172.4		

Menghitung regangan yang terjadi :

Tabel 4.3. Tabel regangan pada

es	Nilai	es	Nilai	es	Nilai
es1	0.00280	es6	0.00014	es11	0.00184
es2	0.00227	es7	0.00039	es12	0.00211
es3	0.00174	es8	0.00092	es13	0.00237
es4	0.00121	es9	0.00131		
es5	0.00067	es10	0.00158		

Untuk rasio penulangan pada dinding geser berpedoman pada SNI03-2847-2002 pasal 23.6.2.(1)

$$V_u < \frac{1}{12} \times A_{cv} \times f_c$$

$$< 894613.511 \text{ N}$$

Dimana :

$$A_{cv} = \text{Luas bruto penampang}$$

$$= 1960000 \text{ mm}^2$$

$$V_u = 14038.548 \text{ kg}$$

$$= 140385.48 \text{ N}$$

$$\text{Karena } V_u = 140385.48 \text{ N} < 894613.5106 \text{ N}$$

maka rasio penulangan untuk dinding geser adalah :

$$\rho > \rho_{\min} = 0.0025$$

Jika dalam perhitungan dicoba menggunakan  $\rho_{\min} = 0.00250$

Sehingga luas penampang yang diperlukan :

$$A_s = \rho \times b \times d$$

$$= 0.0025 \times 35 \times 182.4$$

$$= 15.96 \text{ cm}^2$$

$$\text{Dipasang tulangan untuk bagian tengah } 16 \text{ D } 13 = 21.246 \text{ cm}^2$$

Cek  $\rho$  terpasang

$$\rho = \frac{A_s}{b.d} = \frac{21.246}{35 \times 182.4} = 0.00333$$

$$\rho > \rho_{\min 2}$$

$$0.00333 > 0.00250 \text{ ..... OK}$$

dicoba nilai sampai memenuhi  $C_s + C_c = T_s + P_n$  sehingga  
 dari hasil trial and error, didapat nilai  $c = 1130.311 \text{ mm}$

Untuk daerah tekan :

$$\frac{\epsilon_{s1}}{\epsilon_s} = \frac{c - d1}{c} \qquad \epsilon_{s1} = \frac{c - d1}{c} \times \epsilon_c \qquad ; \epsilon_c = 0,003$$

$$= \frac{113.0311 - 7.6}{113.0311} \times 0.003$$

$$= 0.002798286$$

Untuk daerah tarik :

$$\frac{\epsilon_{s7}}{\epsilon_s} = \frac{d7 - c}{c} \qquad \epsilon_{s7} = \frac{d7 - c}{c} \times \epsilon_c \qquad ; \epsilon_c = 0,003$$

$$= \frac{128 - 113.0311}{113.0311} \times 0.003$$

$$= 0.00039$$

Mencari nilai fs :

Tabel 4.4. Tabel nilai fs

fs	Mpa	fs	Mpa	fs	Mpa
fs1	559.6571	fs6	28.829815	fs11	368.229
fs2	453.4917	fs7	77.335647	fs12	421.3117
fs3	347.3262	fs8	183.50111	fs13	474.3945
fs4	241.1607	fs9	262.06355		
fs5	134.9953	fs10	315.14628		

Keterangan tabel :

**Untuk daerah tekan**

$$fs = \epsilon_s \times E_s$$

$$fs1 = 0.00280 \times 200000 = 559.65712 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $f_y = 300 \text{ Mpa}$

**Untuk daerah tarik**

$$fs = \epsilon_s \times E_s$$

$$fs7 = 0.00039 \times 200000 = 77.335647 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $fs = 77.33565 \text{ Mpa}$

$$fs12 = 0.00211 \times 200000 = 421.31174 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $f_y = 300 \text{ Mpa}$

Besarnya gaya-gaya yang bekerja :

Tabel 4.4. Tabel nilai fs

N	kN	N	kN	N	kN
Cs1	79.67143	Cs6	7.6563752	Ts11	79.67143
Cs2	79.67143	Ts7	20.538138	Ts12	79.67143
Cs3	79.67143	Ts8	48.732652	Ts13	79.67143
Cs4	64.0454	Ts9	69.596591		
Cs5	35.85089	Ts10	79.671429		

Keterangan tabel :

Untuk daerah tekan :

$$C_s = A_s' \times f_s'$$

$$C_{s1} = (2 \times 0,25 \times 22/7 \times 13) \times 300 = 79671.42857 \text{ N} \\ = 79.67142857 \text{ kN}$$

$$C_{s6} = (2 \times 0,25 \times 22/7 \times 13) \times 28.83 = 7656.375177 \text{ N} \\ = 7.656375177 \text{ kN}$$

Untuk daerah tarik :

$$T_s = A_s \times f_s$$

$$T_{s7} = (2 \times 0,25 \times 22/7 \times 13) \times 77.3 = 20538.1382 \text{ N} \\ = 20.5381382 \text{ kN}$$

$$T_{s12} = (2 \times 0,25 \times 22/7 \times 13) \times 421.3 = 111888.3615 \text{ N} \\ = 111.8883615 \text{ kN}$$

$$C_c = 0,85 \cdot f_c \cdot \beta_c \cdot b = 8574.823 \text{ kN} \\ = 0.85 \times 30 \times 0.85 \times 1130.311 \times 350 \\ = 8574822.58 \text{ N} \\ = 8574.82258 \text{ kN}$$

**Kontrol  $\sum H = 0$**

$$(C_{s1} + C_{s2} + \dots + C_{s6}) + C_c = (T_{s7} + T_{s8} + \dots + T_{s13}) + P_n \\ 346.5669514 + 8574.822582 = 457.5530955 + 8463.836769 \\ 8921.390 \text{ kN} = 8921.390 \text{ kN}$$

Mencari titik tengah penampang tulangan

$$A = 1/4 \times 22/7 \times 1.3^2 \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A1 \times d1) + (A2 \times d2) + (A3 \times d3) + \dots + (A13 \times d13)}{A1 + A2 + A3 + \dots + A13}$$

$$= \frac{4070.678857}{34.52428571}$$

$$= 117.9076923 \text{ cm}$$

Tabel 4.1. Tabel jarak tulangan terhadap tengah penampang

y	jarak cm	y	jarak cm	y	jarak cm
y1	110.3077	y6	10.307692	y11	64.49231
y2	90.30769	y7	9.6923077	y12	74.49231
y3	70.30769	y8	29.692308	y13	84.49231
y4	50.30769	y9	44.492308		
y5	30.30769	y10	54.492308		

sehingga besarnya momen yang terjadi terhadap titik berat penampang :

Tabel 4.4. Tabel nilai fs

Mn	kNm	Mn	kNm	Mn	kNm
Mn1	87.884	Mn6	0.789	Mn11	51.382
Mn2	71.949	Mn7	1.991	Mn12	59.349
Mn3	56.015	Mn8	14.470	Mn13	67.316
Mn4	32.220	Mn9	30.965		
Mn5	10.866	Mn10	43.415		

Keterangan tabel :

$$Mn1 = Cc1 \times y1$$

$$= 79.67143 \times 110.31 = 8788.3714 \text{ kNcm} = 87.88 \text{ kNm}$$

$$Mn2 = Cc2 \times y2$$

$$= 79.67143 \times 90.308 = 7194.9429 \text{ kNcm} = 71.95 \text{ kNm}$$



$$\begin{aligned}
\text{Jika } c &= 1130.3111 \text{ mm, maka } a = \beta \cdot c \\
&= 0.85 \times 1130.3111 \\
&= 960.76444 \text{ mm}
\end{aligned}$$

$$\begin{aligned}
y_c &= y - a/2 \\
&= 1179.077 - 960.76444 / 2 \\
&= 698.6947 \text{ mm}
\end{aligned}$$

$$\begin{aligned}
M_n &= (C_c \times y_c) + (M_{n1} + M_{n2} + \dots + M_{n13}) \\
&= (8574.82258 \times 0.6986947) + 528.61 \\
&= 6519.79362 \text{ kNm} > M_n = 3833.430769 \text{ kNm} \quad \dots \text{ Ok}
\end{aligned}$$

### Kontrol terhadap sumbu X

$$M_u = 3131.500 \text{ kgcm} = 313150 \text{ Nmm}$$

$$P_u = 550149.39 \text{ kg} = 5501493.9 \text{ N}$$

Kuat Nominal Penampang :

untuk mengetahui nilai c dapat diselesaikan dengan menggunakan persamaan

Jika diketahui data sebagai berikut :

$$A_s' \text{ 13 D 16} = 2614.857143 \text{ mm}^2 \quad f_y = 300 \text{ Mpa}$$

$$f_c = 30 \text{ Mpa}$$

$$A_s \text{ 13 D 16} = 2614.857143 \text{ mm}^2 \quad \beta = 0.85$$

$$d' = 76 \text{ mm} \quad P_u = 550149.39 \text{ kg}$$

$$b = 2100 \text{ mm} \quad = 5501493.9 \text{ N}$$

Maka  $C_c + C_s = T_s + P_u$

$$\text{Dimana : } C_c \text{ (Beton tertekan)} = 0,85 \cdot f_c \cdot a \cdot b \quad ; \quad a = \beta \cdot c$$

$$C_s \text{ (Baja tertekan)} = A_s' (f_{s1} - 0,85 \cdot f_c)$$

$$T_s \text{ (Baja tertarik)} = A_s1 \cdot F_{y1}$$

Momen Nominal yang disumbangkan oleh beton :

$$M_{nd1} = C_c \times \left( d - \frac{a}{2} \right)$$

$$M_{nd2} = N_{d2} \cdot (d - d')$$

$$M_n = M_{nd1} + M_{nd2} > M_n = \frac{M_u}{\Phi}$$

untuk mendapatkan nilai c, maka :

$$f_s' = \epsilon_s' \cdot E_s = \frac{0,003 (c - d')}{c} \cdot E_s = \frac{600 (c - d')}{c} ; E_s : 200000 \text{ Mpa}$$

Maka :

$$C_c + C_s = T_s + P_u$$

$$(0,85 \cdot f_c \cdot \beta \cdot c \cdot b) + \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f_c \cdot A_s' = A_s \cdot F_y + P_u$$

apabila persamaan tersebut dikalikan c, maka :

$$(0,85 \cdot f_c \cdot \beta \cdot c^2 \cdot b) + ((600 (c - d')) \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \cdot c) = (A_s \cdot f_y + P_u) c$$

Setelah dilakukan pengelompokan, maka didapatkan persamaan kuadrat :

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot c \cdot A_s' - 600 \cdot d' \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \cdot c) - (A_s \cdot f_y \cdot c) - P_u \cdot c = 0$$

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot A_s' - 0,85 \cdot f_c \cdot A_s' - A_s \cdot f_y - P_u) \cdot c - 600 \cdot d' \cdot A_s' = 0$$

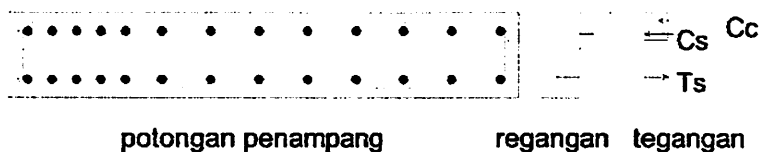
$$45518 \quad c^2 \quad -5568172,757 \quad c \quad - 119237485,7 = 0$$

dari persamaan di atas, di dapatkan nilai c = 140.9197 mm

$$a = \beta \times c = 0,85 \times 140,9197 = 119,7817 \text{ mm}$$

$$\epsilon_y = \frac{f_y}{E_s} = \frac{300}{200000} = 0,00150$$

$$\epsilon_s = 0,003 \cdot \frac{c - d'}{c} = 0,003 \cdot \frac{141 - 76}{140,9197} = 0,001382$$



Gambar 4.2. Diagram tegangan dan regangan arah y

Karena  $\epsilon_s > \epsilon_y$ , maka dapat disimpulkan bahwa tulangan leleh meluluh, dengan demikian maka yang digunakan adalah  $f_y = 300 \text{ Mpa}$

Gaya-gaya yang timbul :

$$\begin{aligned} C_c &= 0,85 \cdot f_c \cdot a \cdot b \\ &= 0,85 \times 30 \times 119,782 \times 2100 \\ &= 6414310,782 \quad \text{N} \end{aligned}$$

$$C_s = \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f'c \cdot A_s'$$

$$= 722776.2608 - 66678.85714 = 656097.4037 \text{ N}$$

$$T_s = A_s \cdot F_y$$

$$= (2614.85714 + 2614.857) \times 300$$

$$= 1568914.286 \text{ N}$$

$$N_d = C_c + C_s$$

**Kontrol :**

$$C_c + C_s = T_s + P_u$$

$$6414310.782 + 656097.4037 = 1568914.286 + 5501493.9$$

$$7070408.186 \text{ N} = 7070408.186 \text{ N} \dots\dots \text{Ok}$$

$$e = \frac{M_u}{P_u} = \frac{313150}{5501493.9} = 0.056920903 \text{ mm}$$

sehingga momen nominal yang disumbangkan oleh beton dan baja adalah sebesar :

$$M_{nd1} = C_c \times \left( d - \frac{a}{2} \right)$$

$$= 6414310.782 \times \left( 274 - \frac{119.7817}{2} \right)$$

$$= 1373362585 \text{ Nmm}$$

$$M_{nd2} = C_s \cdot (d - d')$$

$$= 656097.4037 \times (274 - 76)$$

$$= 129907285.9 \text{ Nmm}$$

$$M_{nd} = M_{nd1} + M_{nd2}$$

$$= 1373362585 + 129907285.9$$

$$= 1503269871 \text{ Nmm}$$

$$M_n = \frac{M_u}{\Phi} = \frac{313150}{0.65} = 481769 \text{ Nmm}$$

$$M_{nd} = 1503269871 \text{ Nmm} > M_n = 481769 \text{ Nmm} \dots\dots \text{Ok}$$

**b. Penulangan Horizontal**

Berdasarkan SNI03-2847-2002 pasal 13.1

$$\Phi V_n \geq V_u$$

$V_u = 244184.83 \text{ kg}$       Dimana :

$\Phi = 0.6$        $V_c = V$  yang disumbangkan oleh beton

$V_n = V_c + V_s$        $V_s = V$  yang disumbangkan tulangan

Berdasarkan SNI03-2847-2002 pasal 13.3.1.(2)

$$V_c = \left[ 1 + \frac{V_u}{14 \cdot A_g} \right] \left[ \frac{\sqrt{f_c}}{6} \right] b_w \cdot d$$

$$= \left[ 1 + \frac{24418.483}{14 \times 735000} \right] \left[ \frac{\sqrt{30}}{6} \right] 350 \times 2024$$

$$= 648212 \text{ N} = 64821.24 \text{ kg}$$

$$V_s = \frac{A_v \cdot f_y \cdot d}{s}$$

$$= \frac{491.0714 \times 300 \times 2024.0}{150}$$

$$= 1987857.14 \text{ N} = 198785.7 \text{ kg}$$

$V_n = 64821 + 198785.7143 = 263607 \text{ kg}$

$V_n \geq V_u$

$263607 \text{ kg} \geq 244185 \text{ kg} \dots\dots\dots \text{Ok}$

Direncanakan tulangan D      25 - 150

$A_v = 1/4 \times 22/7 \times 25^2$

$= 491.071429 \text{ mm}^2 \geq 59.90715473 \text{ mm}^2 \dots\dots\dots \text{OK}$

Syarat :

$$A_v \geq \frac{75 \times f_c \times b_w \times s}{1200 \times f_y}$$

$$\geq \frac{75 \times 30 \times 350 \times 150}{1200 \times 300}$$

$$\geq 59.9071547 \text{ mm}^2$$

### c. Panjang Penyaluran

Berdasarkan buku T. Paulay-M.J.N.Priestley hal 150, panjang sambungan lewatan ls sama dengan ld, sedangkan letak penyaluran dinyatakan dalam Ld.

$$Ld = mdb \times ldb$$

Dimana :

$$ldb = \frac{1.38 \times Ab \times fy}{c \times fc}$$

$$mdb = \text{Faktor modifikasi} = 1.3$$

$$Ab = \text{Luas tulangan}$$

$$c = 3 \times \text{diameter tulangan}$$

Untuk tulangan D 16

$$\begin{aligned} Ab &= 3.14 \times 8^2 \\ &= 201.1428571 \text{ mm}^2 \end{aligned}$$

$$c = 3 \times 16 = 48 \text{ mm}$$

$$\begin{aligned} ldb &= \frac{1.38 \times 201.14286 \times 300}{48 \times 30} \\ &= 316.7401304 \text{ mm} \end{aligned}$$

Jadi untuk :

$$\begin{aligned} Ld &= mdb \times ldb \\ &= 1.3 \times 316.7401 \\ &= 411.7621695 \text{ mm} \end{aligned}$$

## Penulangan pada segmen 2 tanpa bukaan

### a. Penulangan Vertikal

$$M_u = 39586.90 \text{ kgm} = 3958690 \text{ kgcm}$$

$$P_u = 550149.39 \text{ kg}$$

$$M_n = \frac{M_u}{\Phi} = \frac{3958690}{0.65} = 6090292.308 \text{ kgcm}$$

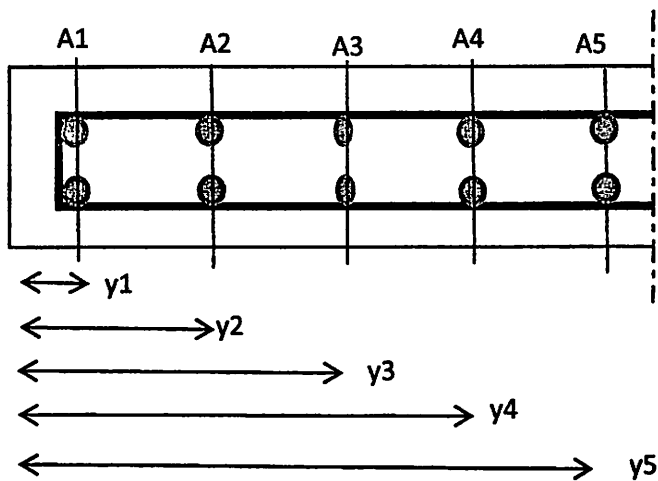
$$P_n = \frac{P_u}{\Phi} = \frac{550149.4}{0.65} = 846383.6769 \text{ kg}$$

$$l_w = 5.6 \text{ m}$$

$$\text{Pendekatan pertama di misalkan } d = 512.4 \text{ cm}$$

$$A_v = \frac{M_n}{f_y \times d} = \frac{6090292.308}{3000 \times 512.4} = 3.961939 \text{ cm}^2$$

$$\text{Dicoba tulangan 10 D 13} \quad A_s = 13.279 \text{ cm}^2$$



$$y_1 = 7.6 \quad y_3 = 27.6 \quad y_5 = 47.6$$

$$y_2 = 17.6 \quad y_4 = 37.6$$

$$A = \left( \frac{1}{4} \times 3.14 \times 1.3^2 \right) \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A_1 \times y_1) + (A_2 \times y_2) + (A_3 \times y_3) + (A_4 \times y_4) + (A_5 \times y_5)}{A_1 + A_2 + A_3 + A_4 + A_5}$$

$$= 27.6 \text{ cm}$$

$$d = 560 - 27.6 = 532.4 \text{ cm}$$

Untuk rasio penulangan pada dinding geser berpedoman pada SNI03-2847-2002 pasal 23.6.2.(1)

$$V_u < \frac{1}{12} \times A_{cv} \times f_c$$

$$< 894613.511 \text{ N}$$

Dimana :

$$A_{cv} = \text{Luas bruto penampang}$$

$$= 1960000 \text{ mm}^2$$

$$V_u = 17253.043 \text{ kg}$$

$$= 172530.43 \text{ N}$$

$$\text{Karena } V_u = 172530.43 \text{ N} < 894613.5106 \text{ N}$$

maka rasio penulangan untuk dinding geser adalah :

$$\rho > \rho_{min} = 0.0025$$

Jika dalam perhitungan dicoba menggunakan  $\rho_{min} = 0.00250$

Sehingga luas penampang yang diperlukan :

$$A_s = \rho \times b \times d$$

$$= 0.00250 \times 35 \times 532.4$$

$$= 46.585 \text{ cm}^2$$

$$\text{Dipasang tulangan untuk bagian tengah } 46 \text{ D } 13 = 61.081 \text{ cm}^2$$

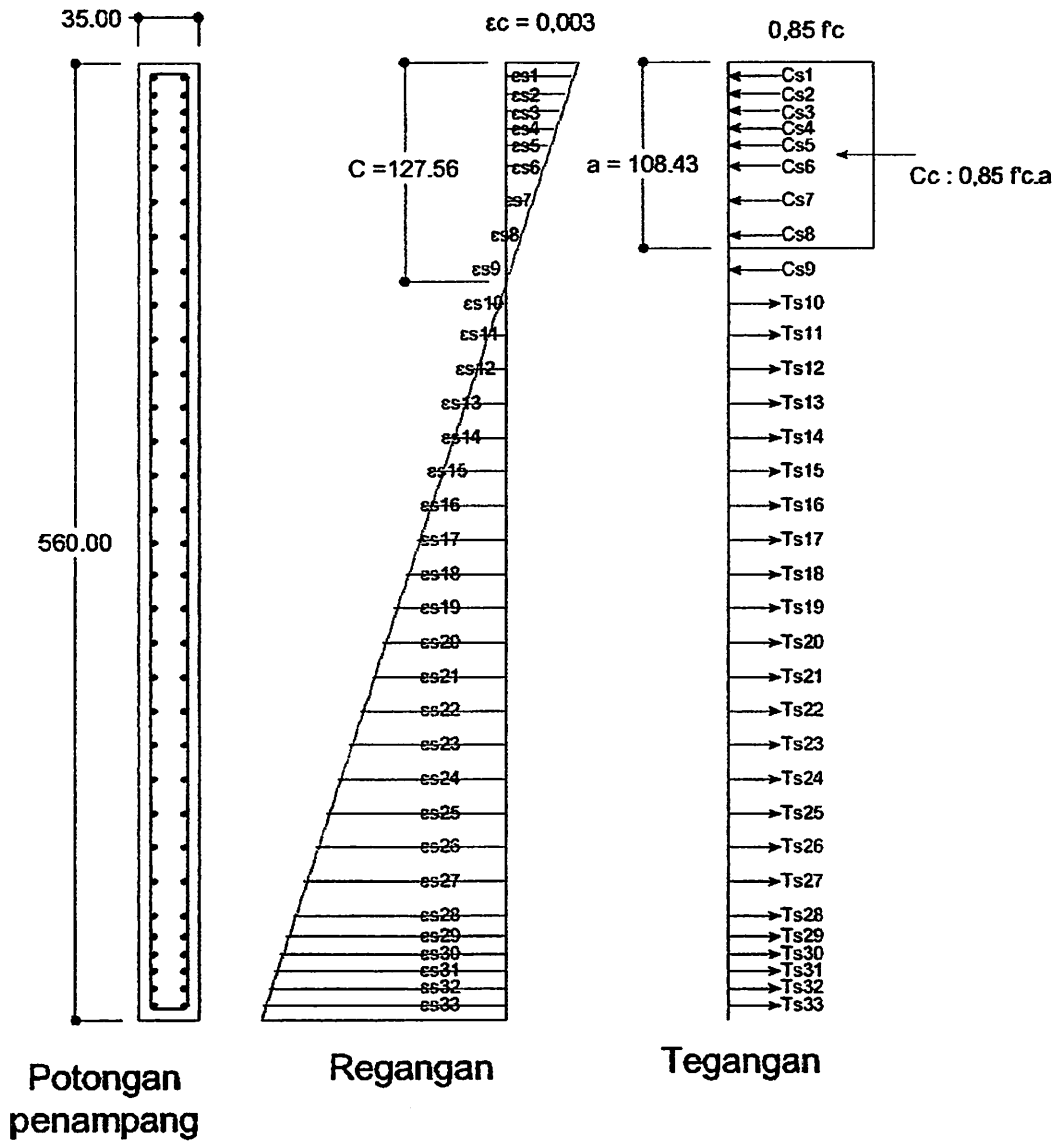
Cek  $\rho$  terpasang

$$\rho = \frac{A_s}{b.d} = \frac{61.081}{35 \times 532.4} = 0.00328$$

$$\rho > \rho_{min2}$$

$$0.00328 > 0.00250 \text{ ..... OK}$$

dicoba nilai sampai memenuhi  $C_s + C_c = T_s + P_n$  sehingga  
 dari hasil trial and error, didapat nilai  $c = 1275.621 \text{ mm}$



Gambar 4.1. Diagram tegangan dan regangan arah x



Tabel 4.1. Tabel jarak tulangan terhadap serat atas

di	jarak cm	di	jarak cm	di	jarak cm
d1	7.6	d12	180	d23	400
d2	17.6	d13	200	d24	420
d3	27.6	d14	220	d25	440
d4	37.6	d15	240	d26	460
d5	47.6	d16	260	d27	480
d6	60	d17	280	d28	500
d7	80	d18	300	d29	512.4
d8	100	d19	320	d30	522.4
d9	120	d20	340	d31	532.4
d10	140	d21	360	d32	542.4
d11	160	d22	380	d33	552.4

Jarak masing-masing tulangan terhadap tengah-tengah penampang

Tabel 4.2. Tabel Jarak tulangan tengah penampang

y	jarak cm	y	jarak cm	y	jarak cm
y1	272.4	y12	100	y23	120
y2	262.4	y13	80	y24	140
y3	252.4	y14	60	y25	160
y4	242.4	y15	40	y26	180
y5	232.4	y16	20	y27	200
y6	220	y17	0	y28	220
y7	200	y18	20	y29	232.4
y8	180	y19	40	y30	242.4
y9	160	y20	60	y31	252.4
y10	140	y21	80	y32	262.4
y11	120	y22	100	y33	272.4

Menghitung regangan yang terjadi :

Tabel 4.3. Tabel regangan

εs	Nilai	εs	Nilai	εs	Nilai
εs1	0.00282	εs12	0.00123	εs23	0.00641
εs2	0.00259	εs13	0.00170	εs24	0.00688
εs3	0.00235	εs14	0.00217	εs25	0.00735
εs4	0.00212	εs15	0.00264	εs26	0.00782
εs5	0.00188	εs16	0.00311	εs27	0.00829
εs6	0.00159	εs17	0.00359	εs28	0.00876
εs7	0.00112	εs18	0.00406	εs29	0.00905
εs8	0.00065	εs19	0.00453	εs30	0.00929
εs9	0.00018	εs20	0.00500	εs31	0.00952
εs10	0.00029	εs21	0.00547	εs32	0.00976
εs11	0.00076	εs22	0.00594	εs33	0.00999

Untuk daerah tekan :

$$\frac{\epsilon_{s1}}{\epsilon_s} = \frac{c - d1}{c} \quad \Rightarrow \quad \epsilon_{s1} = \frac{c - d1}{c} \times \epsilon_c \quad ; \epsilon_c = 0,003$$

$$= \frac{127.5621 - 7.6}{127.5621} \times 0.003$$

$$= 0.002821264$$

Untuk daerah tarik :

$$\frac{\epsilon_{s11}}{\epsilon_s} = \frac{d11 - c}{c} \quad \Rightarrow \quad \epsilon_{s11} = \frac{d11 - c}{c} \times \epsilon_c \quad ; \epsilon_c = 0,003$$

$$= \frac{160 - 127.5621}{127.5621} \times 0.003$$

$$= 0.000763$$

Mencari nilai fs :

Tabel 4.4. Tabel nilai fs

fs	Mpa	fs	Mpa	fs	Mpa
fs1	564.2527	fs12	246.64646	fs23	1281.437
fs2	517.2168	fs13	340.71829	fs24	1375.508
fs3	470.1809	fs14	434.79011	fs25	1469.58
fs4	423.145	fs15	528.86194	fs26	1563.652
fs5	376.109	fs16	622.93377	fs27	1657.724
fs6	317.7845	fs17	717.0056	fs28	1751.796
fs7	223.7127	fs18	811.07743	fs29	1810.12
fs8	129.6409	fs19	905.14926	fs30	1857.156
fs9	35.56903	fs20	999.22109	fs31	1904.192
fs10	58.5028	fs21	1093.2929	fs32	1951.228
fs11	152.5746	fs22	1187.3647	fs33	1998.264

Keterangan tabel :

**Untuk daerah tekan**

$$fs = \epsilon_s \times E_s$$

$$fs1 = 0.00282 \times 200000 = 564.25271 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fy = 300 \text{ Mpa}$

$$fs10 = 0.00029 \times 200000 = 58.5028 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fy = 58.5028 \text{ Mpa}$

**Untuk daerah tarik**

$$fs = \epsilon_s \times E_s$$

$$fs11 = 0.00076 \times 200000 = 152.57463 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fs = 152.5746 \text{ Mpa}$

$$fs15 = 0.00264 \times 200000 = 528.86194 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fy = 300 \text{ Mpa}$

Besarnya gaya-gaya yang bekerja :

Tabel 4.5. Tabel nilai Ts dan Cs

N	kN	N	kN	N	kN
Cs1	79.67143	Ts12	65.502252	Ts23	79.67143
Cs2	79.67143	Ts13	79.671429	Ts24	79.67143
Cs3	79.67143	Ts14	79.671429	Ts25	79.67143
Cs4	79.67143	Ts15	79.671429	Ts26	79.67143
Cs5	79.67143	Ts16	79.671429	Ts27	79.67143
Cs6	79.67143	Ts17	79.671429	Ts28	79.67143
Cs7	59.4117	Ts18	79.671429	Ts29	79.67143
Cs8	34.42891	Ts19	79.671429	Ts30	79.67143
Cs9	9.446118	Ts20	79.671429	Ts31	79.67143
Cs10	15.53667	Ts21	79.671429	Ts32	79.67143
Ts11	40.51946	Ts22	79.671429	Ts33	79.67143

Keterangan tabel :

Untuk daerah tekan :

$$Cs = As \times fs$$

$$Cs1 = (2 \times 0,25 \times 22/7 \times 13) \times 300 = 120685.7143 \text{ N} \\ = 120.6857143 \text{ kN}$$

$$Cs10 = (2 \times 0,25 \times 22/7 \times 13) \times 58.5 = 23534.84061 \text{ N} \\ = 23.53484061 \text{ kN}$$

Untuk daerah tarik :

$$Ts = As \times fs$$

$$Ts11 = (2 \times 0,25 \times 22/7 \times 13) \times 152.6 = 61378.59335 \text{ N} \\ = 61.37859335 \text{ kN}$$

$$Ts13 = (2 \times 0,25 \times 22/7 \times 13) \times 300 = 120685.7143 \text{ N} \\ = 120.6857143 \text{ kN}$$

$$Cc = 0,85 \cdot fc \cdot \beta \cdot c \cdot b$$

$$= 0.85 \times 30 \times 0.85 \times 1275.621 \times 350$$

$$= 9677179.81 \text{ N}$$

$$= 9677.17981 \text{ kN}$$

**Kontrol  $\sum H = 0$**

$$(Cs1 + Cs2 + \dots + Cs8) + Cc = (Ts9 + Ts10 + \dots + Ts33) + Pn$$

$$581.3152944 + 9677.179811 = 1794.658386 + 8463.836769$$

$$10258.495 \text{ kN} = 10258.495 \text{ kN}$$

sehingga besarnya momen yang terjadi terhadap titik berat penampang :

Tabel 4.6. Tabel nilai Mn

Mn	kNm	N	kNm	N	kNm
Mn1	217.025	Mn12	65.502	Mn23	95.606
Mn2	209.058	Mn13	63.737	Mn24	111.540
Mn3	201.091	Mn14	47.803	Mn25	127.474
Mn4	193.124	Mn15	31.869	Mn26	143.409
Mn5	185.16	Mn16	15.934	Mn27	159.343
Mn6	175.28	Mn17	0.000	Mn28	175.277
Mn7	118.82	Mn18	15.934	Mn29	185.156
Mn8	61.972	Mn19	31.869	Mn30	193.124
Mn9	15.114	Mn20	47.803	Mn31	201.091
Mn10	21.751	Mn21	63.737	Mn32	209.058
Mn11	48.623	Mn22	79.671	Mn33	217.025

Keterangan tabel :

$$Mn1 = Nd1 \times y1$$

$$= 79.67143 \times 272.4 = 21702.497 \text{ kNcm} = 217.02 \text{ kNm}$$

$$Mn2 = Nd2 \times y1$$

$$= 79.67143 \times 262.4 = 20905.783 \text{ kNcm} = 209.06 \text{ kNm}$$

Jika  $c = 1275.6210 \text{ mm}$ , maka  $a = \beta.c$

$$= 0.85 \times 1275.621$$

$$= 1084.2779 \text{ mm}$$

$$yc = h/2 - a/2$$

$$= 5600 / 2 - 1084.2779 / 2$$

$$= 2257.861 \text{ mm}$$

$$Mn = (Cc \times yc) + (Mn1 + Mn2 + \dots + Mn33)$$

$$= (9677.17981 \times 2.2578611) + 3728.98$$

$$= 25578.7035 \text{ kNm} > Mn = 6090.292308 \text{ kNm}$$

### Kontrol terhadap sumbu X

$$M_u = 16214.500 \text{ kgcm} = 1621450 \text{ Nmm}$$

$$P_u = 550149.39 \text{ kg} = 5501493.9 \text{ N}$$

Kuat Nominal Penampang :

untuk mengetahui nilai c dapat diselesaikan dengan menggunakan persamaan

Jika diketahui data sebagai berikut :

$$A_s' \text{ 33 D 16} = 6637.714286 \text{ mm}^2 \quad f_y = 300 \text{ Mpa}$$

$$A_s \text{ 33 D 16} = 6637.714286 \text{ mm}^2 \quad \beta = 0.85$$

$$d' = 76 \text{ mm} \quad P_u = 550149.39 \text{ kg}$$

$$b = 5600 \text{ mm} \quad = 5501493.9 \text{ N}$$

Maka  $C_c + C_s = T_s + P_u$

$$\text{Dimana : } C_c \text{ (Beton tertekan)} = 0,85 \cdot f_c \cdot a \cdot b \quad ; \quad a = \beta \cdot c$$

$$C_s \text{ (Baja tertekan)} = A_s' (f_{s1} - 0,85 \cdot f_c)$$

$$T_s \text{ (Baja tertarik)} = A_{s1} \cdot F_{y1}$$

Momen Nominal yang disumbangkan oleh beton :

$$M_{nd1} = C_c \times \left[ d - \frac{a}{2} \right]$$

$$M_{nd2} = C_s \cdot (d - d')$$

$$M_n = M_{nd1} + M_{nd2} > M_n = \frac{M_u}{\Phi}$$

untuk mendapatkan nilai c, maka :

$$f_s' = \epsilon_s' \cdot E_s = \frac{0,003 (c - d')}{c} \cdot E_s = \frac{600 (c - d')}{c} \quad ; \quad E_s : 200000 \text{ Mpa}$$

Maka :

$$N_{d1} + N_{d2} = N_t + P_u$$

$$(0,85 \cdot f_c \cdot \beta \cdot c \cdot b) + \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f_c \cdot A_s' = A_s \cdot F_y + P_u$$

apabila persamaan tersebut dikalikan c, maka :

$$(0,85 \cdot f_c \cdot \beta \cdot c^2 \cdot b) + ((600 (c - d')).As' - 0,85 \cdot f_c \cdot As' \cdot c) = (As \cdot f_y + Pu) \cdot c$$

Setelah dilakukan pengelompokan, maka didapatkan persamaan kuadrat :

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot c \cdot As' - 600 \cdot d' \cdot As' - 0,85 \cdot f_c \cdot As' \cdot c) - (As \cdot f_y \cdot c) - Pu \cdot c = 0$$

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot As' - 0,85 \cdot f_c \cdot As' - As \cdot f_y - Pu) \cdot c - 600 \cdot d' \cdot As' = 0$$

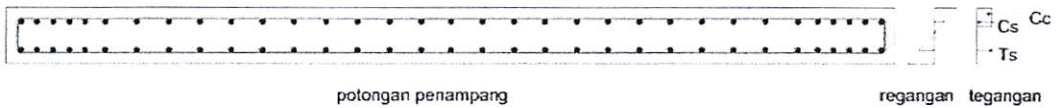
$$121380 \quad c^2 \quad -5670755.614 \quad c \quad - 302679771.4 = 0$$

dari persamaan di atas, di dapatkan nilai c = 78.4896 mm

$$a = \beta \times c = 0.85 \times 78.4896 = 66.7161 \text{ mm}$$

$$\epsilon_y = \frac{f_y}{E_s} = \frac{300}{200000} = 0.00150$$

$$\epsilon_s = 0,003 \cdot \frac{c - d'}{c} = 0.003 \cdot \frac{78.4896 - 76}{78.4896} = 0.000095$$



Gambar 4.2. Diagram tegangan dan regangan arah y

Karena  $\epsilon_s > \epsilon_y$ , maka dapat disimpulkan bahwa tulangan leleh meluluh, dengan demikian maka yang digunakan adalah  $f_y = 300$  Mpa

Gaya-gaya yang timbul :

$$\begin{aligned} C_c &= 0,85 \cdot f_c \cdot a \cdot b \\ &= 0.85 \times 30 \times 66.716 \times 5600 \\ &= 9527062.047 \text{ N} \end{aligned}$$

$$\begin{aligned} C_s &= \frac{600 (c - d')}{c} \cdot As' - 0,85 \cdot f_c \cdot As' \\ &= 126322.1388 - 169261.7143 = -42939.57552 \text{ N} \end{aligned}$$

$$\begin{aligned} T_s &= As \cdot F_y \\ &= (66 \times 3.14 \times 8^2) \times 300 \\ &= 3982628.571 \text{ N} \end{aligned}$$



**Kontrol :**

$$\begin{aligned} C_c + C_s &= T_s + P_u \\ 9527062.05 + -42939.5755 &= 3982628.571 + 5501493.9 \\ 9484122.471 \text{ N} &= 9484122.471 \text{ N} \dots\dots \text{Ok} \end{aligned}$$

$$e = \frac{M_u}{P_u} = \frac{1621450}{5501493.9} = 0.294729037 \text{ mm}$$

sehingga momen nominal yang disumbangkan oleh beton dan baja adalah sebesar :

$$\begin{aligned} M_{nd1} &= C_c \times \left( d - \frac{a}{2} \right) \\ &= 9527062.047 \times \left( 274 - \frac{66.7161}{2} \right) \\ &= 2292610690 \text{ Nmm} \end{aligned}$$

$$\begin{aligned} M_{nd2} &= C_s \cdot (d - d') \\ &= -42939.57552 \times (274 - 76) \\ &= -8502035.953 \text{ Nmm} \end{aligned}$$

$$\begin{aligned} M_{nd} &= M_{nd1} + M_{nd2} \\ &= 2292610690 + -8502035.953 \\ &= 2284108654 \text{ Nmm} \end{aligned}$$

$$M_n = \frac{M_u}{\Phi} = \frac{1621450}{0.65} = 2494538 \text{ Nmm}$$

$$M_{nd} = 2284108654 \text{ Nmm} > M_n = 2494538 \text{ Nmm} \dots\dots \text{Ok}$$



### c. Panjang Penyaluran

Berdasarkan buku T. Paulay-M.J.N.Priestley hal 150, panjang sambungan lewatan  $l_s$  sama dengan  $l_d$ , sedangkan letak penyaluran dinyatakan dalam  $L_d$ .

$$L_d = m_{db} \times l_{db}$$

Dimana :

$$l_{db} = \frac{1.38 \times A_b \times f_y}{c \times \sqrt{f_c}}$$

$$m_{db} = \text{Faktor modifikasi} = 1.3$$

$$A_b = \text{Luas tulangan}$$

$$c = 3 \times \text{diameter tulangan}$$

Untuk tulangan D 13

$$\begin{aligned} A_b &= 3.14 \times 7^2 \\ &= 132.7857143 \text{ mm}^2 \end{aligned}$$

$$c = 3 \times 13 = 39 \text{ mm}$$

$$\begin{aligned} l_{db} &= \frac{1.38 \times 132.78571 \times 300}{39 \times \sqrt{5.477}} \\ &= 257.3513559 \text{ mm} \end{aligned}$$

Jadi untuk :

$$\begin{aligned} L_d &= m_{db} \times l_{db} \\ &= 1.3 \times 257.3514 \\ &= 334.5567627 \text{ mm} \end{aligned}$$

**b. Penulangan Horizontal**

Berdasarkan SNI03-2847-2002 pasal 13.1

$$\Phi V_n \geq V_u$$

$$V_u = 220218.71 \text{ kg} \quad \text{Dimana :}$$

$$\Phi = 0.6 \quad V_c = V \text{ yang disumbangkan oleh beton}$$

$$V_n = V_c + V_s \quad V_s = V \text{ yang disumbangkan tulangan}$$

Berdasarkan SNI03-2847-2002 pasal 13.3.1.(2)

$$\begin{aligned} V_c &= \left[ 1 + \frac{V_u}{14.A_g} \right] \left[ \frac{\sqrt{f_c}}{6} \right] b_w . d \\ &= \left[ 1 + \frac{220218.71}{14 \times 1960000} \right] \left[ \frac{\sqrt{30}}{6} \right] 350 \times 5524 \\ &= 1779109 \text{ N} = 177910.9 \text{ kg} \end{aligned}$$

$$\begin{aligned} V_s &= \frac{A_v . f_y . d}{s} \\ &= \frac{132.7857 \times 300 \times 5524}{300} \\ &= 733508.286 \text{ N} = 73350.83 \text{ kg} \end{aligned}$$

$$V_n = 177911 + 73350.82857 = 251262 \text{ kg}$$

$$V_n \geq V_u$$

$$251262 \text{ kg} \geq 220219 \text{ kg} \dots\dots\dots \text{Ok}$$

Direncanakan tulangan D 13 - 300

$$\begin{aligned} A_v &= 1/4 \times 22/7 \times 13^2 \\ &= 132.785714 \text{ mm}^2 \geq 119.8143095 \text{ mm}^2 \dots\dots\dots \text{OK} \end{aligned}$$

Syarat :

$$\begin{aligned} A_v &\geq \frac{75 \sqrt{f_c} \times b_w \times s}{1200 \times f_y} \\ &\geq \frac{75 \times \sqrt{30} \times 350 \times 300}{1200 \times 300} \\ &\geq 119.814309 \text{ mm}^2 \end{aligned}$$

### 3 Penulangan pada segmen 3 ( ada bukaan )

#### a. Penulangan Vertikal

$$M_u = 26229.20 \text{ kgm} = 2622920 \text{ kgcm}$$

$$P_u = 324760.49 \text{ kg}$$

$$M_n = \frac{M_u}{\Phi} = \frac{2622920}{0.65} = 4035261.538 \text{ kgcm}$$

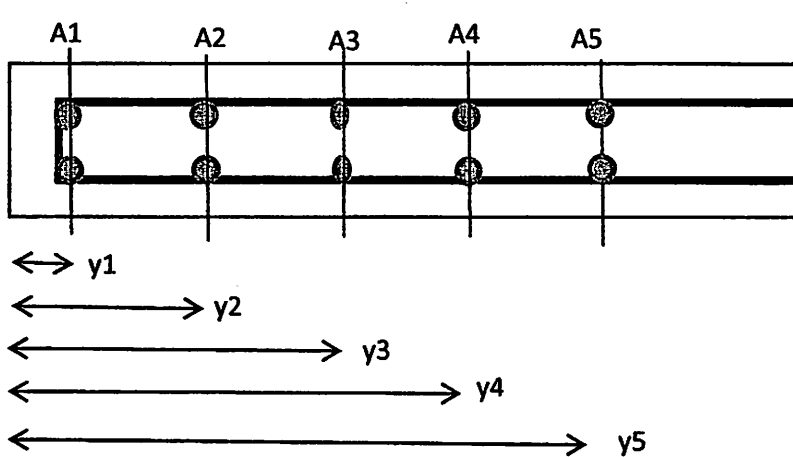
$$P_n = \frac{P_u}{\Phi} = \frac{324760.5}{0.65} = 499631.5231 \text{ kg}$$

$$l_w = 2.1 \text{ m}$$

Pendekatan pertama di misalkan  $d = 162.4 \text{ cm}$

$$A_v = \frac{M_n}{f_y \times d} = \frac{4035261.538}{3000 \times 162.4} = 8.282557 \text{ cm}^2$$

Dicoba tulangan 10 D 13  $A_s = 13.279 \text{ cm}^2$



$$y_1 = 7.6 \quad y_3 = 27.6 \quad y_5 = 47.6$$

$$y_2 = 17.6 \quad y_4 = 37.6$$

$$A = (1/4 \times 3.14 \times 1.3^2) \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A_1 \times y_1) + (A_2 \times y_2) + (A_3 \times y_3) + \dots + (A_5 \times y_5)}{A_1 + A_2 + A_3 + \dots + A_5}$$

$$= 27.6 \text{ cm}$$

$$d = 210 - 27.6 = 182.4 \text{ cm}$$

Untuk rasio penulangan pada dinding geser berpedoman pada SNI03-2847-2002 pasal 23.6.2.(1)

$$V_u < \frac{1}{12} \times A_{cv} \times \sqrt{f_c}$$

$$< 894613.511 \text{ N}$$

Dimana :

$$A_{cv} = \text{Luas bruto penampang}$$

$$= 1960000 \text{ mm}^2$$

$$V_u = 20036.085 \text{ kg}$$

$$= 200360.85 \text{ N}$$

Karena  $V_u = 200360.85 \text{ N} < 894613.5106 \text{ N}$   
maka rasio penulangan untuk dinding geser adalah :

$$\rho > \rho_{min} = 0.0025$$

Jika dalam perhitungan dicoba menggunakan  $\rho_{min} = 0.00250$

Sehingga luas penampang yang diperlukan :

$$A_s = \rho \times b \times d$$

$$= 0.0025 \times 35 \times 182.4$$

$$= 15.96 \text{ cm}^2$$

$$\text{Dipasang tulangan untuk bagian tengah } 16 \text{ D } 13 = 21.246 \text{ cm}^2$$

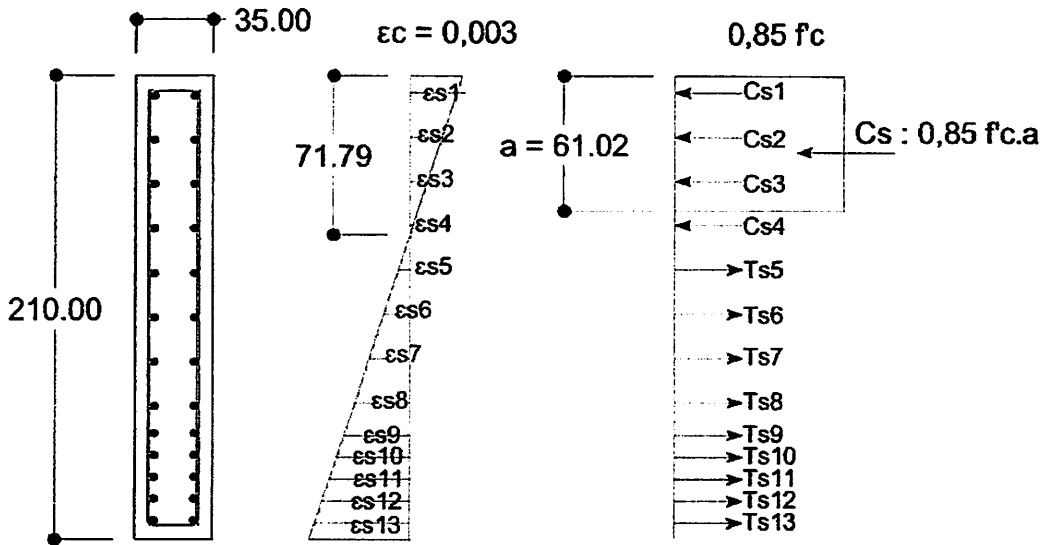
Cek  $\rho$  terpasang

$$\rho = \frac{A_s}{b.d} = \frac{21.246}{35 \times 182.4} = 0.00333$$

$$\rho > \rho_{min2}$$

$$0.00333 > 0.00250 \dots\dots\dots \text{OK}$$

dicoba nilai sampai memenuhi  $C_s + C_c = T_s + P_n$  sehingga  
dari hasil trial and error, didapat nilai  $c = 717.910 \text{ mm}$



Potongan Regangan Tegangan  
penampang

Gambar 4.1. Diagram tegangan dan regangan

Tabel 4.1. Tabel jarak tulangan terhadap serat atas penampang

di	jarak cm
d1	7.6
d2	27.6
d3	47.6
d4	67.6
d5	87.6

di	jarak cm
d6	107.6
d7	127.6
d8	147.6
d9	162.4
d10	172.4

di	jarak cm
d11	182.4
d12	192.4
d13	202.4

Menghitung regangan yang terjadi :

Tabel 4.3. Tabel regangan pada

es	Nilai
es1	0.00268
es2	0.00185
es3	0.00101
es4	0.00018
es5	0.00066

es	Nilai
es6	0.00150
es7	0.00233
es8	0.00317
es9	0.00379
es10	0.00420

es	Nilai
es11	0.00462
es12	0.00504
es13	0.00546

Untuk daerah tekan :

$$\frac{\epsilon_{s1}}{\epsilon_s} = \frac{c - d1}{c} \longrightarrow \epsilon_{s1} = \frac{c - d1}{c} \times \epsilon_c \quad ; \epsilon_c = 0,003$$

$$= \frac{71.7910 - 7.6}{71.7910} \times 0.003$$

$$= 0.002682411$$

Untuk daerah tarik :

$$\frac{\epsilon_{s5}}{\epsilon_s} = \frac{d5 - c}{c} \longrightarrow \epsilon_{s5} = \frac{d5 - c}{c} \times \epsilon_c \quad ; \epsilon_c = 0,003$$

$$= \frac{87.6 - 71.7910}{71.7910} \times 0.003$$

$$= 0.00066$$

Mencari nilai fs :

Tabel 4.4. Tabel nilai fs

fs	Mpa
fs1	536.4823
fs2	369.3304
fs3	202.1785
fs4	35.02667
fs5	132.1252

fs	Mpa
fs6	299.27707
fs7	466.42894
fs8	633.58081
fs9	757.2732
fs10	840.84913

fs	Mpa
fs11	924.4251
fs12	1008.001
fs13	1091.577

Keterangan tabel :

**Untuk daerah tekan**

$$f_s = \epsilon_s \times E_s$$

$$f_{s1} = 0.00268 \times 200000 = 536.48229 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $f_y = 300 \text{ Mpa}$

**Untuk daerah tarik**

$$f_s = \epsilon_s \times E_s$$

$$f_{s5} = 0.00066 \times 200000 = 132.1252 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $f_s = 132.1252 \text{ Mpa}$

$$f_{s12} = 0.00504 \times 200000 = 1008.001 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $f_y = 300 \text{ Mpa}$

Besarnya gaya-gaya yang bekerja :

Tabel 4.4. Tabel nilai fs

N	kN
Cs1	79.67143
Cs2	79.67143
Cs3	53.69285
Cs4	9.302084
Cs5	35.08868

N	kN
Cs6	79.479439
Ts7	79.671429
Ts8	79.671429
Ts9	79.671429
Ts10	79.671429

N	kN
Ts11	79.67143
Ts12	79.67143
Ts13	79.67143

Keterangan tabel :

Untuk daerah tekan :

$$C_s = A_s' \times f_s'$$

$$C_{s1} = (2 \times 0,25 \times 22/7 \times 16) \times 300 = 120685.7143 \text{ N} \\ = 120.6857143 \text{ kN}$$

$$C_{s4} = (2 \times 0,25 \times 22/7 \times 16) \times 35.03 = 14090.73083 \text{ N} \\ = 14.09073083 \text{ kN}$$

Untuk daerah tarik :

$$T_s = A_s \times f_s$$

$$T_{s5} = (2 \times 0,25 \times 22/7 \times 16) \times 132.1 = 53152.07915 \text{ N} \\ = 53.15207915 \text{ kN}$$

$$T_{s12} = (2 \times 0,25 \times 22/7 \times 16) \times 300 = 120685.7143 \text{ N} \\ = 120.6857143 \text{ kN}$$

$$C_c = 0,85 \cdot f_c \cdot \beta_c \cdot b = 5446.245 \text{ kN} \\ = 0,85 \times 30 \times 0,85 \times 717.910 \times 350 \\ = 5446244.74 \text{ N} \\ = 5446.24474 \text{ kN}$$

**Kontrol  $\sum H = 0$**

$$(C_{s1} + C_{s2} + \dots + C_{s4}) + C_c = (T_{s5} + T_{s6} + \dots + T_{s13}) + P_n \\ 222.3377865 + 5446.244738 = 672.2681158 + 4996.315231 \\ 5668.583 \text{ kN} = 5668.583 \text{ kN}$$

Mencari titik tengah penampang tulangan

$$A = \frac{1}{4} \times 22/7 \times 1.3^2 \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A1 \times d1) + (A2 \times d2) + (A3 \times d3) + \dots + (A13 \times d13)}{A1 + A2 + A3 + \dots + A13}$$

$$= \frac{4070.678857}{34.52428571}$$

$$= 117.9076923 \text{ cm}$$

Tabel 4.1. Tabel jarak tulangan terhadap tengah penampang

y	jarak cm	y	jarak cm	y	jarak cm
y1	110.3077	y6	10.307692	y11	64.49231
y2	90.30769	y7	9.6923077	y12	74.49231
y3	70.30769	y8	29.692308	y13	84.49231
y4	50.30769	y9	44.492308		
y5	30.30769	y10	54.492308		

sehingga besarnya momen yang terjadi terhadap titik berat penampang :

Tabel 4.4. Tabel nilai fs

Mn	kNm	Mn	kNm	Mn	kNm
Mn1	87.884	Mn6	8.192	Mn11	51.382
Mn2	71.949	Mn7	7.722	Mn12	59.349
Mn3	37.750	Mn8	23.656	Mn13	67.316
Mn4	4.680	Mn9	35.448		
Mn5	10.635	Mn10	43.415		

Keterangan tabel :

$$Mn1 = Cc1 \times y1$$

$$= 79.67143 \times 110.31 = 8788.3714 \text{ kNcm} = 87.88 \text{ kNm}$$

$$Mn2 = Cc2 \times y2$$

$$= 79.67143 \times 90.308 = 7194.9429 \text{ kNcm} = 71.95 \text{ kNm}$$



$$\begin{aligned} \text{Jika } c &= 717.9100 \text{ mm, maka } a = \beta \cdot c \\ &= 0.85 \times 717.91 \\ &= 610.2235 \text{ mm} \end{aligned}$$

$$\begin{aligned} y_c &= y - a/2 \\ &= 1179.077 - 610.2235 / 2 \\ &= 873.9652 \text{ mm} \end{aligned}$$

$$\begin{aligned} M_n &= (C_c \times y_c) + (M_{n1} + M_{n2} + \dots + M_{n13}) \\ &= (5446.24474 \times 0.8739652) + 509.38 \\ &= 5269.206 \text{ kNm} > M_n = 4035.26 \text{ kNm} \dots \text{ Ok} \end{aligned}$$

### Kontrol terhadap sumbu X

$$M_u = 15466.200 \text{ kgcm} = 1546620 \text{ Nmm}$$

$$P_u = 324760.49 \text{ kg} = 3247604.9 \text{ N}$$

Kuat Nominal Penampang :

untuk mengetahui nilai c dapat diselesaikan dengan menggunakan persamaan

Jika diketahui data sebagai berikut :

$$\begin{aligned} A_s' \text{ 13 D 16} &= 2614.857143 \text{ mm}^2 & f_y &= 300 \text{ Mpa} \\ & & f_c &= 30 \text{ Mpa} \\ A_s \text{ 13 D 16} &= 2614.857143 \text{ mm}^2 & \beta &= 0.85 \\ d' &= 76 \text{ mm} & P_u &= 324760.49 \text{ kg} \\ b &= 2100 \text{ mm} & &= 3247604.9 \text{ N} \end{aligned}$$

Maka  $C_c + C_s = T_s + P_u$

$$\text{Dimana : } C_c \text{ (Beton tertekan)} = 0,85 \cdot f_c \cdot a \cdot b \quad ; \quad a = \beta \cdot c$$

$$C_s \text{ (Baja tertekan)} = A_s' (f_{s1} - 0,85 \cdot f_c)$$

$$T_s \text{ (Baja tertarik)} = A_{s1} \cdot F_{y1}$$

Momen Nominal yang disumbangkan oleh beton :

$$M_{nd1} = C_c \times d - \frac{a}{2}$$

$$M_{nd2} = N_{d2} \cdot (d - d')$$

$$M_n = M_{nd1} + M_{nd2} > M_n = \frac{M_u}{\Phi}$$

untuk mendapatkan nilai c, maka :

$$f_s' = \epsilon_s' \cdot E_s = \frac{0,003 (c - d')}{c} \cdot E_s = \frac{600 (c - d')}{c} ; \quad E_s : 200000 \text{ Mpa}$$

Maka :

$$N_{d1} + N_{d2} = N_t + P_u$$

$$(0,85 \cdot f_c \cdot \beta \cdot c \cdot b) + \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f_c \cdot A_s' = A_s \cdot f_y + P_u$$

apabila persamaan tersebut dikalikan c, maka :

$$(0,85 \cdot f_c \cdot \beta \cdot c^2 \cdot b) + ((600 (c - d')) \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \cdot c) = (A_s \cdot f_y + P_u) \cdot c$$

Setelah dilakukan pengelompokan, maka didapatkan persamaan kuadrat :

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot c \cdot A_s' - 600 \cdot d' \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \cdot c) - (A_s \cdot f_y \cdot c) - P_u \cdot c = 0$$

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot A_s' - 0,85 \cdot f_c \cdot A_s' - A_s \cdot f_y - P_u) \cdot c - 600 \cdot d' \cdot A_s' = 0$$

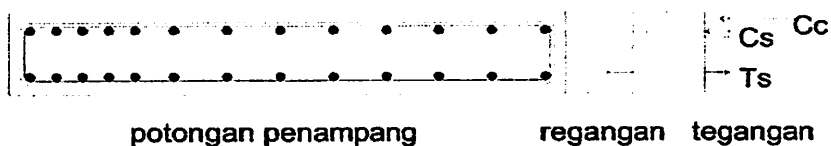
$$45518 \quad c^2 \quad -3314283.757 \quad c \quad - 119237485.7 = 0$$

dari persamaan di atas, di dapatkan nilai c = 99.2163 mm

$$a = \beta \times c = 0.85 \times 99.2163 = 84.3338 \text{ mm}$$

$$\epsilon_y = \frac{f_y}{E_s} = \frac{300}{200000} = 0.00150$$

$$\epsilon_s = 0,003 \cdot \frac{c - d'}{c} = 0.003 \cdot \frac{99 - 76}{99.2163} = 0.000702$$



Gambar 4.2. Diagram tegangan dan regangan arah y

Karena  $\epsilon_s > \epsilon_y$ , maka dapat disimpulkan bahwa tulangan leleh meluluh, dengan demikian maka yang digunakan adalah  $f_y = 300 \text{ Mpa}$

Gaya-gaya yang timbul :

$$\begin{aligned} C_c &= 0,85 \cdot f_c \cdot a \cdot b \\ &= 0.85 \times 30 \times 84.334 \times 2100 \\ &= 4516077.251 \text{ N} \end{aligned}$$

$$C_s = \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f'c \cdot A_s'$$

$$= 367120.7922 - 66678.85714 = 300441.9351 \text{ N}$$

$$T_s = A_s \cdot F_y$$

$$= (2614.857 + 2614.857) \times 300$$

$$= 1568914.286 \text{ N}$$

$$N_d = C_c + C_s$$

**Kontrol :**

$$N_d = N_t + P_u$$

$$4816519.186 = 1568914.286 + 3247604.9$$

$$4816519.186 \text{ N} = 4816519.186 \text{ N} \dots\dots \text{Ok}$$

$$e = \frac{M_u}{P_u} = \frac{1546620}{3247604.9} = 0.476234039 \text{ mm}$$

sehingga momen nominal yang disumbangkan oleh beton dan baja adalah sebesar :

$$M_{nd1} = C_c \times \left( d - \frac{a}{2} \right)$$

$$= 4816519.186 \times \left( 274 - \frac{84.3338}{2} \right)$$

$$= 1116628472 \text{ Nmm}$$

$$M_{nd2} = C_s \cdot (d - d')$$

$$= 300441.9351 \times (274 - 76)$$

$$= 59487503.14 \text{ Nmm}$$

$$M_{nd} = M_{nd1} + M_{nd2}$$

$$= 1116628472 + 59487503.14$$

$$= 1176115976 \text{ Nmm}$$

$$M_n = \frac{M_u}{\Phi} = \frac{1546620}{0.7} = 2379415 \text{ Nmm}$$

$$M_{nd} = 1176115976 \text{ Nmm} > M_n = 2379415 \text{ Nmm} \dots\dots \text{Ok}$$

**b. Penulangan Horizontal**

Berdasarkan SNI03-2847-2002 pasal 13.1

$$\Phi V_n \geq V_u$$

$$V_u = 244184.83 \text{ kg} \quad \text{Dimana :}$$

$$\Phi = 0.6 \quad V_c = V \text{ yang disumbangkan oleh beton}$$

$$V_n = V_c + V_s \quad V_s = V \text{ yang disumbangkan tulangan}$$

Berdasarkan SNI03-2847-2002 pasal 13.3.1.(2)

$$\begin{aligned} V_c &= \left[ 1 + \frac{V_u}{14.A_g} \right] \left[ \frac{\sqrt{f_c}}{6} \right] b_w . d \\ &= \left[ 1 + \frac{244184.83}{14 \times 735000} \right] \left[ \frac{\sqrt{30}}{6} \right] 350 \times 2024 \\ &= 662024 \text{ N} = 66202.36 \text{ kg} \end{aligned}$$

$$\begin{aligned} V_s &= \frac{A_v . f_y . d}{s} \\ &= \frac{491.0714 \times 300 \times 2024}{150} \\ &= 1987857.14 \text{ N} = 198785.7 \text{ kg} \end{aligned}$$

$$V_n = 66202 + 198785.7143 = 264988 \text{ kg}$$

$$V_n \geq V_u$$

$$264988 \text{ kg} \geq 244185 \text{ kg} \dots\dots\dots \text{Ok}$$

Direncanakan tulangan D 25 - 150

$$\begin{aligned} A_v &= 1/4 \times 22/7 \times 25^2 \\ &= 491.071 \text{ mm}^2 \geq 59.907 \text{ mm}^2 \dots\dots\dots \text{OK} \end{aligned}$$

Syarat :

$$\begin{aligned} A_v &\geq \frac{75 \quad f_c \times b_w \times s}{1200 \times f_y} \\ &\geq \frac{75 \times 30 \times 350 \times 150}{1200 \times 300} \\ &\geq 59.9071547 \text{ mm}^2 \end{aligned}$$

### c. Panjang Penyaluran

Berdasarkan buku T. Paulay-M.J.N.Priestley hal 150, panjang sambungan lewatan ls sama dengan ld, sedangkan letak penyaluran dinyatakan dalam Ld.

$$L_d = m_{db} \times l_{db}$$

Dimana :

$$l_{db} = \frac{1.38 \times A_b \times f_y}{c \times f_c}$$

$$m_{db} = \text{Faktor modifikasi} = 1.3$$

$$A_b = \text{Luas tulangan}$$

$$c = 3 \times \text{diameter tulangan}$$

Untuk tulangan D 13

$$\begin{aligned} A_b &= 3.14 \times 8^2 \\ &= 132.7857143 \text{ mm}^2 \end{aligned}$$

$$c = 3 \times 13 = 39 \text{ mm}$$

$$\begin{aligned} l_{db} &= \frac{1.38 \times 132.78571 \times 300}{39 \times 30} \\ &= 257.3513559 \text{ mm} \end{aligned}$$

Jadi untuk :

$$\begin{aligned} L_d &= m_{db} \times l_{db} \\ &= 1.3 \times 257.3514 \\ &= 334.5567627 \text{ mm} \end{aligned}$$

### Penulangan pada segmen 3 tanpa bukaan

#### a. Penulangan Vertikal

$$M_u = 28989.80 \text{ kgm} = 2898980 \text{ kgcm}$$

$$P_u = 164651.29 \text{ kg}$$

$$M_n = \frac{M_u}{\Phi} = \frac{2898980}{0.65} = 4459969.231 \text{ kgcm}$$

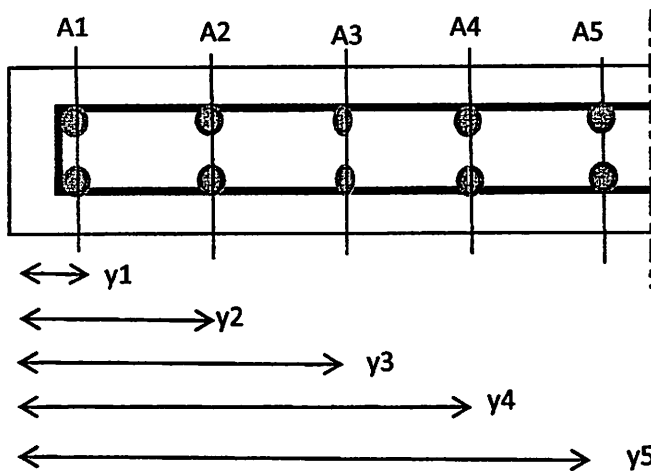
$$P_n = \frac{P_u}{\Phi} = \frac{164651.3}{0.65} = 253309.6769 \text{ kg}$$

$$l_w = 5.6 \text{ m}$$

Pendekatan pertama di misalkan  $d = 512.4 \text{ cm}$

$$A_v = \frac{M_n}{f_y \times d} = \frac{4459969.231}{3000 \times 512.4} = 2.9014 \text{ cm}^2$$

Dicoba tulangan 10 D 13  $A_s = 13.279 \text{ cm}^2$



$$y_1 = 7.6 \quad y_3 = 27.6 \quad y_5 = 47.6$$

$$y_2 = 17.6 \quad y_4 = 37.6$$

$$A = \left( \frac{1}{4} \times 3.14 \times 1.3^2 \right) \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A_1 \times y_1) + (A_2 \times y_2) + (A_3 \times y_3) + (A_4 \times y_4) + (A_5 \times y_5)}{A_1 + A_2 + A_3 + A_4 + A_5}$$

$$= 27.6 \text{ cm}$$

$$d = 560 - 27.6 = 532.4 \text{ cm}$$

Untuk rasio penulangan pada dinding geser berpedoman pada SNI03-2847-2002 pasal 23.6.2.(1)

$$V_u < \frac{1}{12} \times A_{cv} \times \sqrt{f_c}$$

$$< 894613.511 \text{ N}$$

Dimana :

$$A_{cv} = \text{Luas bruto penampang}$$

$$= 1960000 \text{ mm}^2$$

$$V_u = 24418.483 \text{ kg}$$

$$= 244184.83 \text{ N}$$

Karena  $V_u = 244184.83 \text{ N} < 894613.5106 \text{ N}$   
maka rasio penulangan untuk dinding geser adalah :

$$\rho > \rho_{min} = 0.0025$$

Jika dalam perhitungan dicoba menggunakan  $\rho_{min} = 0.00250$

Sehingga luas penampang yang diperlukan :

$$A_s = \rho \times b \times d$$

$$= 0.00250 \times 35 \times 532.4$$

$$= 46.585 \text{ cm}^2$$

Dipasang tulangan untuk bagian tengah 46 D 13 = 61.081 cm<sup>2</sup>

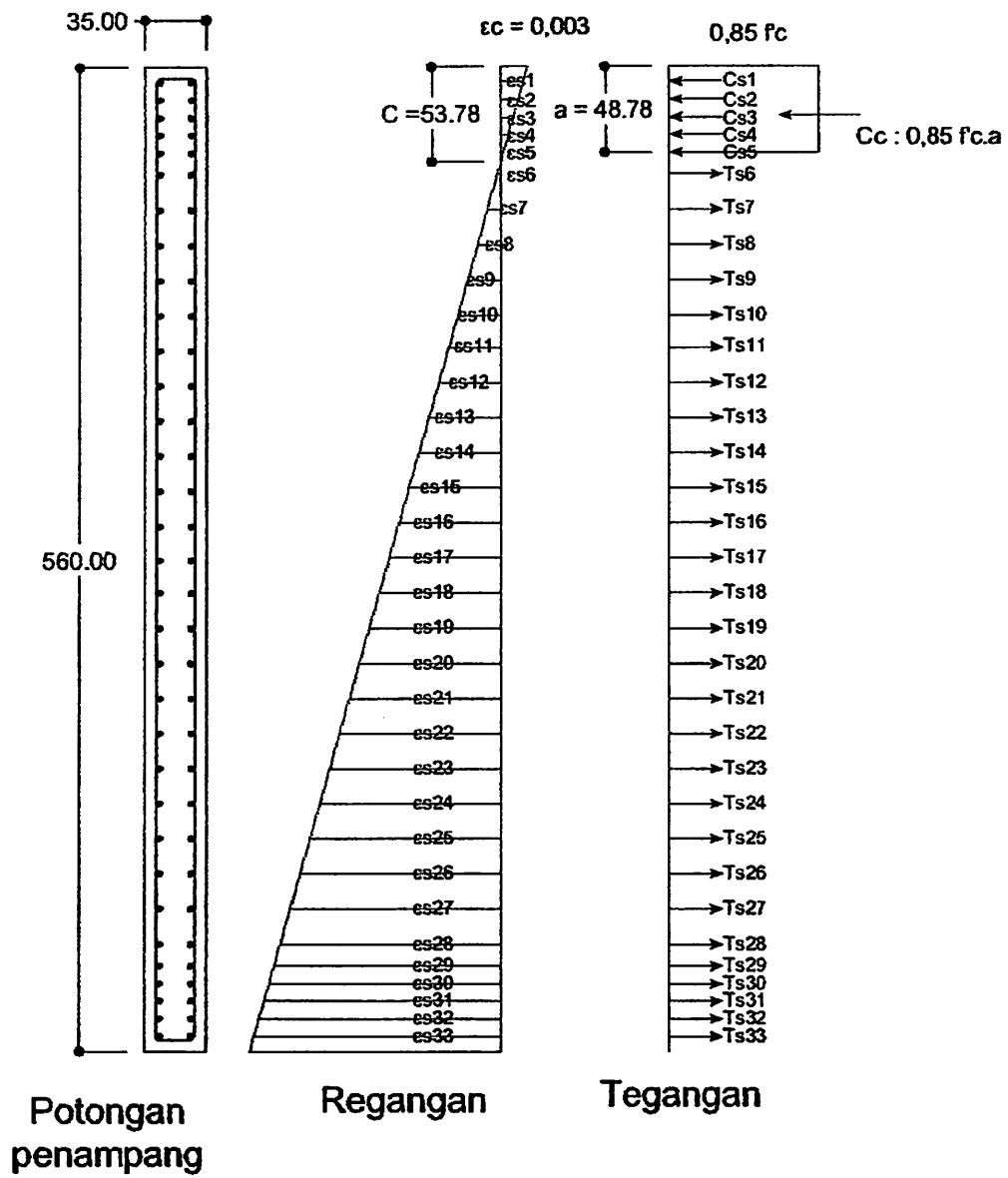
Cek p terpasang

$$\rho = \frac{A_s}{b.d} = \frac{61.081}{35 \times 532.4} = 0.00328$$

$$\rho > \rho_{min2}$$

$$0.00328 > 0.00250 \dots\dots\dots \text{OK}$$

dicoba nilai sampai memenuhi  $C_s + C_c = T_s + P_n$  sehingga  
dari hasil trial and error, didapat nilai  $c = 573.864 \text{ mm}$



Gambar 4.1. Diagram tegangan dan regangan arah x



Tabel 4.1. Tabel jarak tulangan terhadap serat atas

di	jarak cm	di	jarak cm	di	jarak cm
d1	7.6	d12	180	d23	400
d2	17.6	d13	200	d24	420
d3	27.6	d14	220	d25	440
d4	37.6	d15	240	d26	460
d5	47.6	d16	260	d27	480
d6	60	d17	280	d28	500
d7	80	d18	300	d29	512.4
d8	100	d19	320	d30	522.4
d9	120	d20	340	d31	532.4
d10	140	d21	360	d32	542.4
d11	160	d22	380	d33	552.4

Jarak masing-masing tulangan terhadap tengah-tengah penampang

Tabel 4.2. Tabel Jarak tulangan tengah penampang

y	jarak cm	y	jarak cm	y	jarak cm
y1	272.4	y12	100	y23	120
y2	262.4	y13	80	y24	140
y3	252.4	y14	60	y25	160
y4	242.4	y15	40	y26	180
y5	232.4	y16	20	y27	200
y6	220	y17	0	y28	220
y7	200	y18	20	y29	232.4
y8	180	y19	40	y30	242.4
y9	160	y20	60	y31	252.4
y10	140	y21	80	y32	262.4
y11	120	y22	100	y33	272.4

Menghitung regangan yang terjadi :

Tabel 4.3. Tabel regangan

εs	Nilai	εs	Nilai	εs	Nilai
εs1	0.00260	εs12	0.00641	εs23	0.01791
εs2	0.00208	εs13	0.00746	εs24	0.01896
εs3	0.00156	εs14	0.00850	εs25	0.02000
εs4	0.00103	εs15	0.00955	εs26	0.02105
εs5	0.00051	εs16	0.01059	εs27	0.02209
εs6	0.00014	εs17	0.01164	εs28	0.02314
εs7	0.00118	εs18	0.01268	εs29	0.02379
εs8	0.00223	εs19	0.01373	εs30	0.02431
εs9	0.00327	εs20	0.01477	εs31	0.02483
εs10	0.00432	εs21	0.01582	εs32	0.02536
εs11	0.00536	εs22	0.01687	εs33	0.02588

Untuk daerah tekan :

$$\frac{\epsilon_{s1}}{\epsilon_s} = \frac{c - d1}{c} \quad \Rightarrow \quad \epsilon_{s1} = \frac{c - d1}{c} \times \epsilon_c \quad ; \epsilon_c = 0,003$$

$$= \frac{57.3864 - 7.6}{57.3864} \times 0.003$$

$$= 0.002602693$$

Untuk daerah tarik :

$$\frac{\epsilon_{s6}}{\epsilon_s} = \frac{d6 - c}{c} \quad \Rightarrow \quad \epsilon_{s6} = \frac{d6 - c}{c} \times \epsilon_c \quad ; \epsilon_c = 0,003$$

$$= \frac{60 - 57.3864}{57.3864} \times 0.003$$

$$= 0.000137$$

Mencari nilai fs :

Tabel 4.4. Tabel nilai fs

fs	Mpa	fs	Mpa	fs	Mpa
fs1	520.5386	fs12	1281.9797	fs23	3582.177
fs2	415.9842	fs13	1491.0885	fs24	3791.286
fs3	311.4298	fs14	1700.1974	fs25	4000.395
fs4	206.8754	fs15	1909.3062	fs26	4209.504
fs5	102.3209	fs16	2118.4151	fs27	4418.612
fs6	27.32655	fs17	2327.5239	fs28	4627.721
fs7	236.4354	fs18	2536.6328	fs29	4757.369
fs8	445.5443	fs19	2745.7416	fs30	4861.923
fs9	654.6531	fs20	2954.8505	fs31	4966.478
fs10	863.762	fs21	3163.9593	fs32	5071.032
fs11	1072.871	fs22	3373.0682	fs33	5175.586

Keterangan tabel :

**Untuk daerah tekan**

$$fs = \epsilon_s \times E_s$$

$$fs1 = 0.00260 \times 200000 = 520.53864 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fy = 300 \text{ Mpa}$

$$fs5 = 0.00051 \times 200000 = 102.32093 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fy = 102.3209 \text{ Mpa}$

**Untuk daerah tarik**

$$fs = \epsilon_s \times E_s$$

$$fs6 = 0.00014 \times 200000 = 27.326554 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fs = 27.32655 \text{ Mpa}$

$$fs15 = 0.00955 \times 200000 = 1909.3062 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fy = 300 \text{ Mpa}$

Besarnya gaya-gaya yang bekerja :

Tabel 4.5. Tabel nilai Ts dan Cs

N	kN	N	kN	N	kN
Cs1	79.67143	Ts12	79.671429	Ts23	79.67143
Cs2	79.67143	Ts13	79.671429	Ts24	79.67143
Cs3	79.67143	Ts14	79.671429	Ts25	79.67143
Cs4	54.94018	Ts15	79.671429	Ts26	79.67143
Cs5	27.17352	Ts16	79.671429	Ts27	79.67143
Cs6	7.257152	Ts17	79.671429	Ts28	79.67143
Cc7	62.79049	Ts18	79.671429	Ts29	79.67143
Ts8	79.67143	Ts19	79.671429	Ts30	79.67143
Ts9	79.67143	Ts20	79.671429	Ts31	79.67143
Ts10	79.67143	Ts21	79.671429	Ts32	79.67143
Ts11	79.67143	Ts22	79.671429	Ts33	79.67143

Keterangan tabel :

Untuk daerah tekan :

$$C_s = A_s \times f_s$$

$$C_{s1} = (2 \times 0,25 \times 22/7 \times 13) \times 300 = 79671.42857 \text{ N} \\ = 79.67142857 \text{ kN}$$

$$C_{s5} = (2 \times 0,25 \times 22/7 \times 13) \times 102.3 = 27173.51661 \text{ N} \\ = 27.17351661 \text{ kN}$$

Untuk daerah tarik :

$$T_s = A_s \times f_s$$

$$T_{s6} = (2 \times 0,25 \times 22/7 \times 13) \times 27.3 = 7257.151928 \text{ N} \\ = 7.257151928 \text{ kN}$$

$$T_{s15} = (2 \times 0,25 \times 22/7 \times 13) \times 300 = 79671.42857 \text{ N} \\ = 79.67142857 \text{ kN}$$

$$C_c = 0,85 \cdot f_c \cdot \beta_c \cdot b$$

$$= 0.85 \times 30 \times 0.85 \times 573.864 \times 350$$

$$= 4353474.25 \text{ N}$$

$$= 4353.47425 \text{ kN}$$

**Kontrol  $\sum H = 0$**

$$\begin{aligned}
 (Cs1 + Cs2 + \dots + Cs7) + Cc &= (Ts8 + Ts9 + \dots + Ts33) + Pn \\
 321.1279871 + 4353.474253 &= 2141.504783 + 2533.096769 \\
 4674.602 \text{ kN} &= 4674.602 \text{ kN}
 \end{aligned}$$

sehingga besarnya momen yang terjadi terhadap titik berat penampang :

Tabel 4.6. Tabel nilai Mn

Mn	kNm	N	kNm	N	kNm
Mn1	217.025	Mn12	79.671	Mn23	95.606
Mn2	209.058	Mn13	63.737	Mn24	111.540
Mn3	201.091	Mn14	47.803	Mn25	127.474
Mn4	133.175	Mn15	31.869	Mn26	143.409
Mn5	63.15	Mn16	15.934	Mn27	159.343
Mn6	15.97	Mn17	0.000	Mn28	175.277
Mn7	125.58	Mn18	15.934	Mn29	185.156
Mn8	143.409	Mn19	31.869	Mn30	193.124
Mn9	127.474	Mn20	47.803	Mn31	201.091
Mn10	111.540	Mn21	63.737	Mn32	209.058
Mn11	95.606	Mn22	79.671	Mn33	217.025

Keterangan tabel :

$$\begin{aligned}
 Mn1 &= Nd1 \times y1 \\
 &= 79.67143 \times 272.4 = 21702.497 \text{ kNcm} = 217.02 \text{ kNm}
 \end{aligned}$$

$$\begin{aligned}
 Mn2 &= Nd2 \times y1 \\
 &= 79.67143 \times 262.4 = 20905.783 \text{ kNcm} = 209.06 \text{ kNm}
 \end{aligned}$$

$$\begin{aligned}
 \text{Jika } c &= 573.8638 \text{ mm, maka } a = \beta.c \\
 &= 0.85 \times 573.8638 \\
 &= 487.78423 \text{ mm}
 \end{aligned}$$

$$\begin{aligned}
 yc &= h/2 - a/2 \\
 &= 5600 / 2 - 487.78423 / 2 \\
 &= 2556.108 \text{ mm}
 \end{aligned}$$

$$\begin{aligned}
 Mn &= (Cc \times yc) + (Mn1 + Mn2 + \dots + Mn33) \\
 &= (4353.47425 \times 2.5561079) + 3739.21 \\
 &= 14867.1555 \text{ kNm} > Mn = 4459.969231 \text{ kNm}
 \end{aligned}$$

### Kontrol terhadap sumbu X

$$M_u = 17129.200 \text{ kgcm} = 1712920 \text{ Nmm}$$

$$P_u = 164651.29 \text{ kg} = 1646512.9 \text{ N}$$

Kuat Nominal Penampang :

untuk mengetahui nilai c dapat diselesaikan dengan menggunakan persamaan

Jika diketahui data sebagai berikut :

$$\begin{aligned} A_s' \text{ 33 D 16} &= 6637.714286 \text{ mm}^2 & f_y &= 300 \text{ Mpa} \\ A_s \text{ 33 D 16} &= 6637.714286 \text{ mm}^2 & \beta &= 0.85 \\ d' &= 76 \text{ mm} & P_u &= 164651.29 \text{ kg} \\ b &= 5600 \text{ mm} & &= 1646512.9 \text{ N} \end{aligned}$$

Maka  $C_c + C_s = T_s + P_u$

$$\text{Dimana : } C_c \text{ (Beton tertekan)} = 0,85 \cdot f_c \cdot a \cdot b \quad ; \quad a = \beta \cdot c$$

$$C_s \text{ (Baja tertekan)} = A_s' (f_{s1} - 0,85 \cdot f_c)$$

$$T_s \text{ (Baja tertarik)} = A_s1 \cdot F_{y1}$$

Momen Nominal yang disumbangkan oleh beton :

$$M_{nd1} = C_c \times \left[ d - \frac{a}{2} \right]$$

$$M_{nd2} = C_s \cdot (d - d')$$

$$M_n = M_{nd1} + M_{nd2} > M_n = \frac{M_u}{\Phi}$$

untuk mendapatkan nilai c, maka :

$$f_s' = \epsilon_s' \cdot E_s = \frac{0,003 (c - d')}{c} \cdot E_s = \frac{600 (c - d')}{c} \quad ; \quad E_s : 200000 \text{ Mpa}$$

Maka :

$$N_{d1} + N_{d2} = N_t + P_u$$

$$(0,85 \cdot f_c \cdot \beta \cdot c \cdot b) + \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f_c \cdot A_s' = A_s \cdot F_y + P_u$$

apabila persamaan tersebut dikalikan c, maka :

$$(0,85 \cdot f_c \cdot \beta \cdot c^2 \cdot b) + ((600 (c - d')).As' - 0,85 \cdot f_c \cdot As' \cdot c) = (As \cdot f_y + Pu) c$$

Setelah dilakukan pengelompokan, maka didapatkan persamaan kuadrat :

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot c \cdot As' - 600 \cdot d' \cdot As' - 0,85 \cdot f_c \cdot As' \cdot c) - (As \cdot f_y \cdot c) - Pu \cdot c = 0$$

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot As' - 0,85 \cdot f_c \cdot As' - As \cdot f_y - Pu) \cdot c - 600 \cdot d' \cdot As' = 0$$

$$121380 \quad c^2 \quad -1815774.614 \quad c \quad - 302679771.4 = 0$$

dari persamaan di atas, di dapatkan nilai c = 57.9733 mm

$$a = \beta \times c = 0.85 \times 57.9733 = 49.2773 \text{ mm}$$

$$\epsilon_y = \frac{f_y}{E_s} = \frac{300}{200000} = 0.00150$$

$$\epsilon_s = 0,003 \cdot \frac{c - d'}{c} = 0.003 \cdot \frac{57.9733 - 76}{57.9733} = -0.000933$$



Gambar 4.2. Diagram tegangan dan regangan arah y

Karena  $\epsilon_s > \epsilon_y$ , maka dapat disimpulkan bahwa tulangan leleh meluluh, dengan demikian maka yang digunakan adalah  $f_y = 300 \text{ Mpa}$

Gaya-gaya yang timbul :

$$\begin{aligned} C_c &= 0,85 \cdot f_c \cdot a \cdot b \\ &= 0.85 \times 30 \times 49.277 \times 5600 \\ &= 7036796.673 \text{ N} \end{aligned}$$

$$\begin{aligned} C_s &= \frac{600 (c - d')}{c} \cdot As' - 0,85 \cdot f_c \cdot As' \\ &= -1238393.488 - 169261.7143 = -1407655.202 \text{ N} \end{aligned}$$

$$\begin{aligned} T_s &= As \cdot F_y \\ &= (66 \times 3.14 \times 8^2) \times 300 \\ &= 3982628.571 \text{ N} \end{aligned}$$

**Kontrol :**

$$\begin{aligned} C_c + C_s &= T_s + P_u \\ 7036796.67 + -1407655.2 &= 3982628.571 + 1646512.9 \\ 5629141.471 \text{ N} &= 5629141.471 \text{ N} \dots\dots \text{Ok} \end{aligned}$$

$$e = \frac{M_u}{P_u} = \frac{1712920}{1646512.9} = 1.040331965 \text{ mm}$$

sehingga momen nominal yang disumbangkan oleh beton dan baja adalah sebesar :

$$\begin{aligned} M_{nd1} &= C_c \times \left( d - \frac{a}{2} \right) \\ &= 7036796.673 \times \left( 274 - \frac{49.2773}{2} \right) \\ &= 1754705162 \text{ Nmm} \end{aligned}$$

$$\begin{aligned} M_{nd2} &= C_s \cdot (d - d') \\ &= -1407655.202 \times (274 - 76) \\ &= -278715730 \text{ Nmm} \end{aligned}$$

$$\begin{aligned} M_{nd} &= M_{nd1} + M_{nd2} \\ &= 1754705162 + -278715730 \\ &= 1475989432 \text{ Nmm} \end{aligned}$$

$$M_n = \frac{M_u}{\Phi} = \frac{1712920}{0.65} = 2635262 \text{ Nmm}$$

$$M_{nd} = 1475989432 \text{ Nmm} > M_n = 2635262 \text{ Nmm} \dots\dots \text{Ok}$$



**b. Penulangan Horizontal**

Berdasarkan SNI03-2847-2002 pasal 13.1

$$\Phi V_n \geq V_u$$



$V_u = 243410.99 \text{ kg}$  Dimana :

$\Phi = 0.6$   $V_c = V$  yang disumbangkan oleh beton

$V_n = V_c + V_s$   $V_s = V$  yang disumbangkan tulangan

Berdasarkan SNI03-2847-2002 pasal 13.3.1.(2)

$$\begin{aligned} V_c &= \left[ 1 + \frac{V_u}{14.A_g} \right] \left[ \frac{\sqrt{f_c}}{6} \right] b_w . d \\ &= \left[ 1 + \frac{243410.99}{14 \times 1960000} \right] \left[ \frac{\sqrt{30}}{6} \right] 350 \times 5524 \\ &= 1780601 \text{ N} = 178060.1 \text{ kg} \end{aligned}$$

$$\begin{aligned} V_s &= \frac{A_v . f_y . d}{s} \\ &= \frac{132.7857 \times 300 \times 5524}{300} \\ &= 733508.286 \text{ N} = 73350.83 \text{ kg} \end{aligned}$$

$$V_n = 178060 + 73350.82857 = 251411 \text{ kg}$$

$$V_n \geq V_u$$

$$251411 \text{ kg} \geq 243411 \text{ kg} \dots\dots\dots \text{Ok}$$

Direncanakan tulangan D 13 - 300

$$\begin{aligned} A_v &= 1/4 \times 22/7 \times 13^2 \\ &= 132.785714 \text{ mm}^2 \geq 119.8143095 \text{ mm}^2 \dots\dots\dots \text{OK} \end{aligned}$$

Syarat :

$$\begin{aligned} A_v &\geq \frac{75 \sqrt{f_c} \times b_w \times s}{1200 \times f_y} \\ &\geq \frac{75 \times \sqrt{30} \times 350 \times 300}{1200 \times 300} \\ &\geq 119.814309 \text{ mm}^2 \end{aligned}$$

### c. Panjang Penyaluran

Berdasarkan buku T. Paulay-M.J.N.Priestley hal 150, panjang sambungan lewatan ls sama dengan ld, sedangkan letak penyaluran dinyatakan dalam Ld.

$$L_d = m_{db} \times l_{db}$$

Dimana :

$$l_{db} = \frac{1.38 \times A_b \times f_y}{c \times \sqrt{f_c}}$$

$$m_{db} = \text{Faktor modifikasi} = 1.3$$

$$A_b = \text{Luas tulangan}$$

$$c = 3 \times \text{diameter tulangan}$$

Untuk tulangan D 13

$$\begin{aligned} A_b &= 3.14 \times 7^2 \\ &= 132.7857143 \text{ mm}^2 \end{aligned}$$

$$c = 3 \times 13 = 39 \text{ mm}$$

$$\begin{aligned} l_{db} &= \frac{1.38 \times 132.78571 \times 300}{39 \times \sqrt{5.477}} \\ &= 257.3513559 \text{ mm} \end{aligned}$$

Jadi untuk :

$$\begin{aligned} L_d &= m_{db} \times l_{db} \\ &= 1.3 \times 257.3514 \\ &= 334.5567627 \text{ mm} \end{aligned}$$

#### 4 Penulangan pada segmen 4 ( ada bukaan )

##### a. Penulangan Vertikal

$$M_u = 27538.00 \text{ kgm} = 2753800 \text{ kgcm}$$

$$P_u = 141533.17 \text{ kg}$$

$$M_n = \frac{M_u}{\Phi} = \frac{2753800}{0.65} = 4236615.385 \text{ kgcm}$$

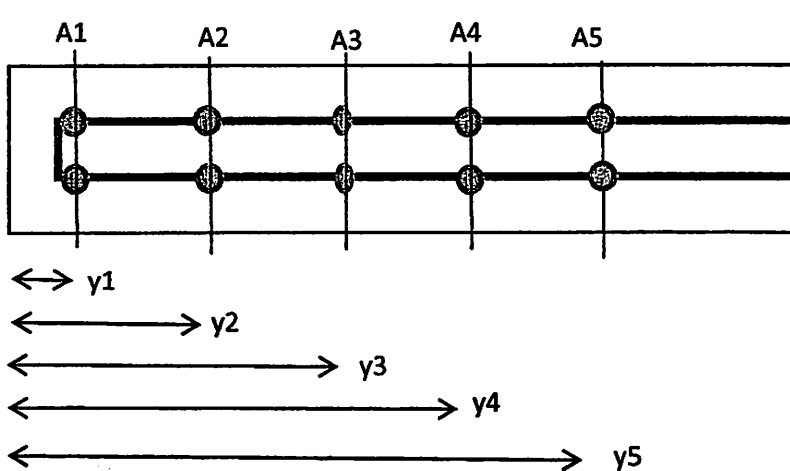
$$P_n = \frac{P_u}{\Phi} = \frac{141533.2}{0.65} = 217743.3385 \text{ kg}$$

$$l_w = 2.1 \text{ m}$$

Pendekatan pertama di misalkan  $d = 162.4 \text{ cm}$

$$A_v = \frac{M_n}{f_y \times d} = \frac{4236615.385}{3000 \times 162.4} = 8.695844 \text{ cm}^2$$

Dicoba tulangan 10 D 13  $A_s = 13.279 \text{ cm}^2$



$$y_1 = 7.6 \quad y_3 = 27.6 \quad y_5 = 47.6$$

$$y_2 = 17.6 \quad y_4 = 37.6$$

$$A = (1/4 \times 3.14 \times 1.3^2) \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A_1 \times y_1) + (A_2 \times y_2) + (A_3 \times y_3) + \dots + (A_5 \times y_5)}{A_1 + A_2 + A_3 + \dots + A_5}$$

$$= 27.6 \text{ cm}$$

$$d = 210 - 27.6 = 182.4 \text{ cm}$$

Untuk rasio penulangan pada dinding geser berpedoman pada SNI03-2847-2002 pasal 23.6.2.(1)

$$V_u < \frac{1}{12} \times A_{cv} \times \sqrt{f_c}$$

$$< 894613.511 \text{ N}$$

Dimana :

$$A_{cv} = \text{Luas bruto penampang}$$

$$= 1960000 \text{ mm}^2$$

$$V_u = 23969.729 \text{ kg}$$

$$= 239697.29 \text{ N}$$

$$\text{Karena } V_u = 239697.29 \text{ N} < 894613.5106 \text{ N}$$

maka rasio penulangan untuk dinding geser adalah :

$$\rho > \rho_{min} = 0.0025$$

Jika dalam perhitungan dicoba menggunakan  $\rho_{min} = 0.00250$

Sehingga luas penampang yang diperlukan :

$$A_s = \rho \times b \times d$$

$$= 0.0025 \times 35 \times 182.4$$

$$= 15.96 \text{ cm}^2$$

$$\text{Dipasang tulangan untuk bagian tengah } 16 \text{ D } 13 = 21.246 \text{ cm}^2$$

Cek  $\rho$  terpasang

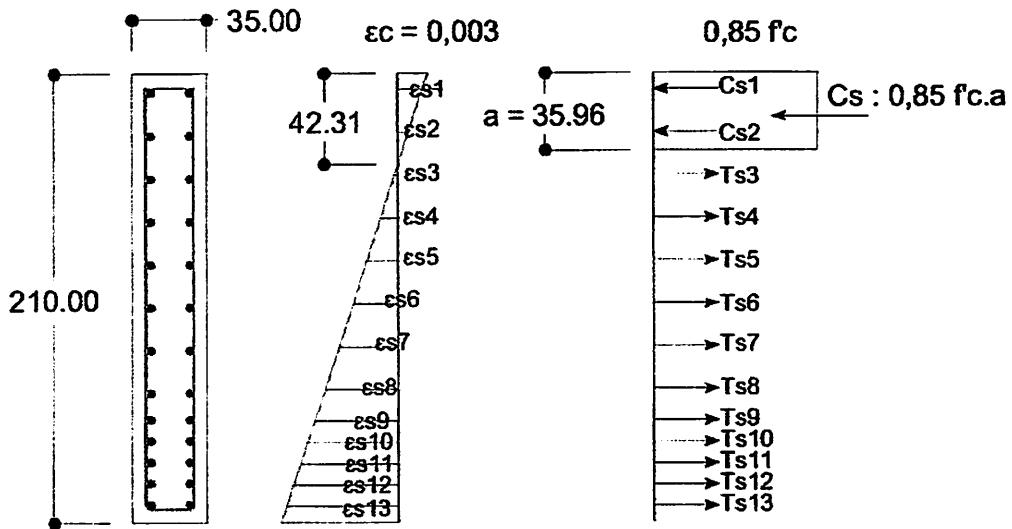
$$\rho = \frac{A_s}{b.d} = \frac{21.246}{35 \times 182.4} = 0.00333$$

$$\rho > \rho_{min}$$

$$0.00333 > 0.00250 \dots\dots\dots \text{OK}$$

dicoba nilai sampai memenuhi  $C_s + C_c = T_s + P_n$  sehingga

$$\text{dari hasil trial and error, didapat nilai } c = 423.114 \text{ mm}$$



Potongan penampang      Regangan      Tegangan

Gambar 4.1. Diagram tegangan dan regangan

Tabel 4.1. Tabel jarak tulangan terhadap serat atas penampang

di	jarak cm	di	jarak cm	di	jarak cm
d1	7.6	d6	107.6	d11	182.4
d2	27.6	d7	127.6	d12	192.4
d3	47.6	d8	147.6	d13	202.4
d4	67.6	d9	162.4		
d5	87.6	d10	172.4		

Menghitung regangan yang terjadi :

Tabel 4.3. Tabel regangan pada

es	Nilai	es	Nilai	es	Nilai
es1	0.00246	es6	0.00463	es11	0.00993
es2	0.00104	es7	0.00605	es12	0.01064
es3	0.00037	es8	0.00747	es13	0.01135
es4	0.00179	es9	0.00851		
es5	0.00321	es10	0.00922		

Untuk daerah tekan :

$$\frac{\epsilon_{s1}}{\epsilon_s} = \frac{c - d1}{c} \qquad \epsilon_{s1} = \frac{c - d1}{c} \times \epsilon_c \qquad ; \epsilon_c = 0,003$$

$$= \frac{42.3114 - 7.6}{42.3114} \times 0.003$$

$$= 0.002461139$$

Untuk daerah tarik :

$$\frac{\epsilon_{s3}}{\epsilon_s} = \frac{d3 - c}{c} \qquad \epsilon_{s3} = \frac{d3 - c}{c} \times \epsilon_c \qquad ; \epsilon_c = 0,003$$

$$= \frac{47.6 - 42.3114}{42.3114} \times 0.003$$

$$= 0.00037$$

Mencari nilai fs :

Tabel 4.4. Tabel nilai fs

fs	Mpa
fs1	492.2277
fs2	208.6164
fs3	74.99491
fs4	358.6062
fs5	642.2175

fs	Mpa
fs6	925.82884
fs7	1209.4401
fs8	1493.0515
fs9	1702.9238
fs10	1844.7295

fs	Mpa
fs11	1986.535
fs12	2128.341
fs13	2270.146

Keterangan tabel :

**Untuk daerah tekan**

$$fs = \epsilon_s \times E_s$$

$$fs1 = 0.00246 \times 200000 = 492.2277 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $f_y = 300 \text{ Mpa}$

**Untuk daerah tarik**

$$fs = \epsilon_s \times E_s$$

$$fs3 = 0.00037 \times 200000 = 74.994913 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $fs = 74.99491 \text{ Mpa}$

$$fs4 = 0.00179 \times 200000 = 358.60622 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $f_y = 300 \text{ Mpa}$

Besarnya gaya-gaya yang bekerja :

Tabel 4.4. Tabel nilai fs

N	kN	N	kN	N	kN
Cs1	120.6857	Cs6	120.68571	Ts11	120.6857
Cs2	83.9234	Ts7	120.68571	Ts12	120.6857
Cs3	30.16938	Ts8	120.68571	Ts13	120.6857
Cs4	120.6857	Ts9	120.68571		
Cs5	120.6857	Ts10	120.68571		

Keterangan tabel :

Untuk daerah tekan :

$$C_s = A_s' \times f_s'$$

$$C_{s1} = (2 \times 0,25 \times 22/7 \times 16) \times 300 = 120685.7143 \text{ N} \\ = 120.6857143 \text{ kN}$$

$$C_{s2} = (2 \times 0,25 \times 22/7 \times 16) \times 208.6 = 83923.39553 \text{ N} \\ = 83.92339553 \text{ kN}$$

Untuk daerah tarik :

$$T_s = A_s \times f_s$$

$$T_{s3} = (2 \times 0,25 \times 22/7 \times 16) \times 75 = 30169.38203 \text{ N} \\ = 30.16938203 \text{ kN}$$

$$T_{s4} = (2 \times 0,25 \times 22/7 \times 16) \times 300 = 120685.7143 \text{ N} \\ = 120.6857143 \text{ kN}$$

$$C_c = 0,85 \cdot f_c \cdot \beta_c \cdot b \cdot d = 3209.851 \text{ kN} \\ = 0.85 \times 30 \times 0.85 \times 423.114 \times 350 \\ = 3209850.86 \text{ N} \\ = 3209.85086 \text{ kN}$$

**Kontrol  $\sum H = 0$**

$$(C_{s1} + C_{s2}) + C_c = (T_{s3} + T_{s4} + \dots + T_{s13}) + P_n \\ 204.6091098 + 3209.850858 = 1237.026525 + 2177.433385 \\ 3414.460 \text{ kN} = 3414.460 \text{ kN}$$

Mencari titik tengah penampang tulangan

$$A = \frac{1}{4} \times 22/7 \times 1.3^2 \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A1 \times d1) + (A2 \times d2) + (A3 \times d3) + \dots + (A13 \times d13)}{A1 + A2 + A3 + \dots + A13}$$

$$= \frac{4070.678857}{34.52428571}$$

$$= 117.9076923 \text{ cm}$$

Tabel 4.1. Tabel jarak tulangan terhadap tengah penampang

y	jarak cm	y	jarak cm	y	jarak cm
y1	110.3077	y6	10.307692	y11	64.49231
y2	90.30769	y7	9.6923077	y12	74.49231
y3	70.30769	y8	29.692308	y13	84.49231
y4	50.30769	y9	44.492308		
y5	30.30769	y10	54.492308		

sehingga besarnya momen yang terjadi terhadap titik berat penampang :

Tabel 4.4. Tabel nilai fs

Mn	kNm	Mn	kNm	Mn	kNm
Mn1	133.126	Mn6	12.440	Mn11	77.833
Mn2	75.789	Mn7	11.697	Mn12	89.902
Mn3	21.211	Mn8	35.834	Mn13	101.970
Mn4	60.714	Mn9	53.696		
Mn5	36.577	Mn10	65.764		

Keterangan tabel :

$$Mn1 = Cc1 \times y1$$

$$= 120.6857 \times 110.31 = 13312.563 \text{ kNcm} = 133.13 \text{ kNm}$$

$$Mn2 = Cc2 \times y2$$

$$= 83.9234 \times 90.308 = 7578.9282 \text{ kNcm} = 75.79 \text{ kNm}$$



$$\begin{aligned} \text{Jika } c &= 423.1143 \text{ mm, maka } a = \beta \cdot c \\ &= 0.85 \times 423.1143 \\ &= 359.64716 \text{ mm} \end{aligned}$$

$$\begin{aligned} y_c &= y - a/2 \\ &= 1179.077 - 359.64716 / 2 \\ &= 999.2533 \text{ mm} \end{aligned}$$

$$\begin{aligned} M_n &= (C_c \times y_c) + (M_{n1} + M_{n2} + \dots + M_{n13}) \\ &= (3209.85086 \times 0.9992533) + 776.55 \\ &= 3984.008 \text{ kNm} > M_n = 423.66 \text{ kNm} \dots \text{ Ok} \end{aligned}$$

### Kontrol terhadap sumbu X

$$M_u = 17129.200 \text{ kgcm} = 1712920 \text{ Nmm}$$

$$P_u = 141533.17 \text{ kg} = 1415331.7 \text{ N}$$

Kuat Nominal Penampang :

untuk mengetahui nilai c dapat diselesaikan dengan menggunakan persamaan

Jika diketahui data sebagai berikut :

$$\begin{aligned} A_s' \text{ 13 D 16} &= 2614.857143 \text{ mm}^2 & f_y &= 300 \text{ Mpa} \\ & & f_c &= 30 \text{ Mpa} \\ A_s \text{ 13 D 16} &= 2614.857143 \text{ mm}^2 & \beta &= 0.85 \\ d' &= 76 \text{ mm} & P_u &= 141533.17 \text{ kg} \\ b &= 2100 \text{ mm} & &= 1415331.7 \text{ N} \end{aligned}$$

Maka  $C_c + C_s = T_s + P_u$

$$\text{Dimana : } C_c \text{ (Beton tertekan)} = 0,85 \cdot f_c \cdot a \cdot b \quad ; \quad a = \beta \cdot c$$

$$C_s \text{ (Baja tertekan)} = A_s' (f_{s1} - 0,85 \cdot f_c)$$

$$T_s \text{ (Baja tertarik)} = A_s1 \cdot F_{y1}$$

Momen Nominal yang disumbangkan oleh beton :

$$M_{nd1} = C_c \times d - \frac{a}{2}$$

$$M_{nd2} = N_{d2} \cdot (d - d')$$

$$M_n = M_{nd1} + M_{nd2} > M_n = \frac{M_u}{\Phi}$$

untuk mendapatkan nilai  $c$ , maka :

$$f_s' = \epsilon_s' \cdot E_s = \frac{0,003 (c - d')}{c} \cdot E_s = \frac{600 (c - d')}{c} ; \quad E_s : 200000 \text{ Mpa}$$

Maka :

$$N_{d1} + N_{d2} = N_t + P_u$$

$$(0,85 \cdot f_c \cdot \beta \cdot c \cdot b) + \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f_c \cdot A_s' = A_s \cdot f_y + P_u$$

apabila persamaan tersebut dikalikan  $c$ , maka :

$$(0,85 \cdot f_c \cdot \beta \cdot c^2 \cdot b) + ((600 (c - d')) \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \cdot c) = (A_s \cdot f_y + P_u) c$$

Setelah dilakukan pengelompokan, maka didapatkan persamaan kuadrat :

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot c \cdot A_s' - 600 \cdot d' \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \cdot c) - (A_s \cdot f_y \cdot c) - P_u \cdot c = 0$$

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot A_s' \cdot c - 0,85 \cdot f_c \cdot A_s' \cdot c - A_s \cdot f_y \cdot c - P_u \cdot c) - 600 \cdot d' \cdot A_s' = 0$$

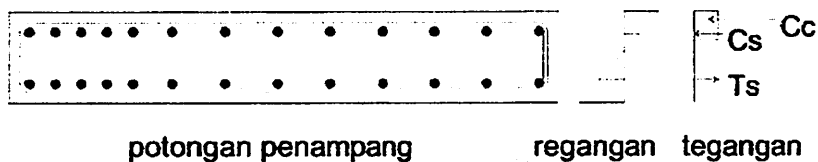
$$45518 \quad c^2 \quad -1482010,557 \quad c \quad - 119237485,7 = 0$$

dari persamaan di atas, di dapatkan nilai  $c = 69.9882 \text{ mm}$

$$a = \beta \times c = 0,85 \times 69.9882 = 59.4900 \text{ mm}$$

$$\epsilon_y = \frac{f_y}{E_s} = \frac{300}{200000} = 0,00150$$

$$\epsilon_s = 0,003 \cdot \frac{c - d'}{c} = 0,003 \cdot \frac{70 - 76}{69.9882} = -0,000258$$



Gambar 4.2. Diagram tegangan dan regangan arah y

Karena  $\epsilon_s > \epsilon_y$ , maka dapat disimpulkan bahwa tulangan leleh meluluh, dengan demikian maka yang digunakan adalah  $f_y = 300 \text{ Mpa}$

Gaya-gaya yang timbul :

$$\begin{aligned} C_c &= 0,85 \cdot f_c \cdot a \cdot b \\ &= 0,85 \times 30 \times 59.490 \times 2100 \\ &= 3185689,528 \quad \text{N} \end{aligned}$$

$$C_s = \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f'c \cdot A_s'$$

$$= -134764.685 - 66678.85714 = -201443.5422 \text{ N}$$

$$T_s = A_s \cdot F_y$$

$$= (2614.857 + 2614.857) \times 300$$

$$= 1568914.286 \text{ N}$$

$$N_d = C_c + C_s$$

**Kontrol :**

$$N_d = N_t + P_u$$

$$2984245.986 = 1568914.286 + 1415331.7$$

$$2984245.986 \text{ N} = 2984245.986 \text{ N} \text{ ..... Ok}$$

$$e = \frac{M_u}{P_u} = \frac{1712920}{1415331.7} = 1.210260464 \text{ mm}$$

sehingga momen nominal yang disumbangkan oleh beton dan baja adalah sebesar :

$$M_{nd1} = C_c \times \left( d - \frac{a}{2} \right)$$

$$= 2984245.986 \times \left( 274 - \frac{59.4900}{2} \right)$$

$$= 728917002.5 \text{ Nmm}$$

$$M_{nd2} = C_s \cdot (d - d')$$

$$= -201443.5422 \times (274 - 76)$$

$$= -39885821.35 \text{ Nmm}$$

$$M_{nd} = M_{nd1} + M_{nd2}$$

$$= 728917002.5 + -39885821.35$$

$$= 689031181.1 \text{ 0}$$

$$M_n = \frac{M_u}{\phi} = \frac{1712920}{0.7} = 2635262 \text{ Nmm}$$

$$M_{nd} = 689031181 \text{ Nmm} > M_n = 2635262 \text{ Nmm} \text{ .....Ok}$$

**b. Penulangan Horizontal**

Berdasarkan SNI03-2847-2002 pasal 13.1

$$\Phi V_n \geq V_u$$

$$V_u = 239697.29 \text{ kg} \quad \text{Dimana :}$$

$$\Phi = 0.6 \quad V_c = V \text{ yang disumbangkan oleh beton}$$

$$V_n = V_c + V_s \quad V_s = V \text{ yang disumbangkan tulangan}$$

Berdasarkan SNI03-2847-2002 pasal 13.3.1.(2)

$$\begin{aligned} V_c &= \left[ 1 + \frac{V_u}{14.A_g} \right] \left[ \frac{\sqrt{f_c}}{6} \right] b_w . d \\ &= \left[ 1 + \frac{239697.29}{14 \times 735000} \right] \left[ \frac{\sqrt{30}}{6} \right] 350 \times 2024 \\ &= 661742 \text{ N} = 66174.16 \text{ kg} \end{aligned}$$

$$\begin{aligned} V_s &= \frac{A_v . f_y . d}{s} \\ &= \frac{491.0714 \times 300 \times 2024}{150} \\ &= 1987857.14 \text{ N} = 198785.7 \text{ kg} \end{aligned}$$

$$V_n = 66174 + 198785.7143 = 264960 \text{ kg}$$

$$V_n \geq V_u$$

$$264960 \text{ kg} \geq 239697 \text{ kg} \dots\dots\dots \text{Ok}$$

Direncanakan tulangan D 25 - 150

$$\begin{aligned} A_v &= 1/4 \times 22/7 \times 25^2 \\ &= 491.071 \text{ mm}^2 \geq 59.907 \text{ mm}^2 \dots\dots\dots \text{OK} \end{aligned}$$

Syarat :

$$\begin{aligned} A_v &\geq \frac{75 \quad f_c \times b_w \times s}{1200 \times f_y} \\ &\geq \frac{75 \times 30 \times 350 \times 150}{1200 \times 300} \\ &\geq 59.9071547 \text{ mm}^2 \end{aligned}$$

### c. Panjang Penyaluran

Berdasarkan buku T. Paulay-M.J.N.Priestley hal 150, panjang sambungan lewatan ls sama dengan ld, sedangkan letak penyaluran dinyatakan dalam Ld.

$$L_d = m_{db} \times l_{db}$$

Dimana :

$$l_{db} = \frac{1.38 \times A_b \times f_y}{c \times f_c}$$

$$m_{db} = \text{Faktor modifikasi} = 1.3$$

$$A_b = \text{Luas tulangan}$$

$$c = 3 \times \text{diameter tulangan}$$

Untuk tulangan D 16

$$\begin{aligned} A_b &= 3.14 \times 8^2 \\ &= 201.1428571 \text{ mm}^2 \end{aligned}$$

$$c = 3 \times 16 = 48 \text{ mm}$$

$$\begin{aligned} l_{db} &= \frac{1.38 \times 201.14286 \times 300}{48 \times 30} \\ &= 316.7401304 \text{ mm} \end{aligned}$$

Jadi untuk :

$$\begin{aligned} L_d &= m_{db} \times l_{db} \\ &= 1.3 \times 316.7401 \\ &= 411.7621695 \text{ mm} \end{aligned}$$

## Penulangan pada segmen 4 tanpa bukaan

### a. Penulangan Vertikal

$$M_u = 27538.00 \text{ kgm} = 2753800 \text{ kgcm}$$

$$P_u = 141533.17 \text{ kg}$$

$$M_n = \frac{M_u}{\Phi} = \frac{2753800}{0.65} = 4236615.385 \text{ kgcm}$$

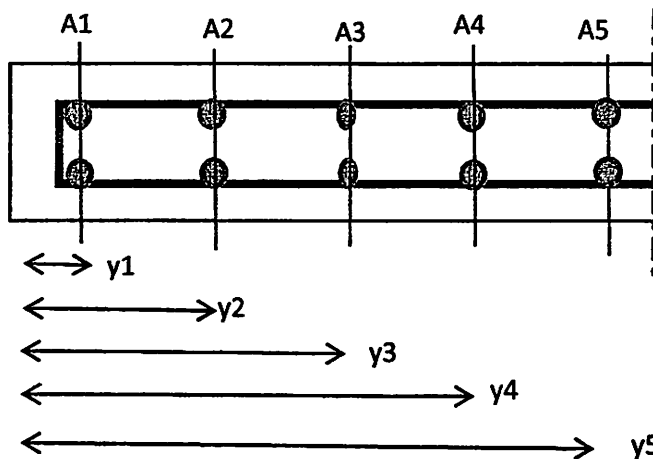
$$P_n = \frac{P_u}{\Phi} = \frac{141533.2}{0.65} = 217743.3385 \text{ kg}$$

$$l_w = 5.6 \text{ m}$$

$$\text{Pendekatan pertama di misalkan } d = 512.4 \text{ cm}$$

$$A_v = \frac{M_n}{f_y \times d} = \frac{4236615.385}{3000 \times 512.4} = 2.7561 \text{ cm}^2$$

$$\text{Dicoba tulangan 10 D 13 } A_s = 13.279 \text{ cm}^2$$



$$y_1 = 7.6 \quad y_3 = 27.6 \quad y_5 = 47.6$$

$$y_2 = 17.6 \quad y_4 = 37.6$$

$$A = \left( \frac{1}{4} \times 3.14 \times 1.3^2 \right) \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A_1 \times y_1) + (A_2 \times y_2) + (A_3 \times y_3) + (A_4 \times y_4) + (A_5 \times y_5)}{A_1 + A_2 + A_3 + A_4 + A_5}$$

$$= 27.6 \text{ cm}$$

$$d = 560 - 27.6 = 532.4 \text{ cm}$$

Untuk rasio penulangan pada dinding geser berpedoman pada SNI03-2847-2002 pasal 23.6.2.(1)

$$V_u < \frac{1}{12} \times A_{cv} \times f_c$$

$$< 894613.511 \text{ N}$$

Dimana :

$$A_{cv} = \text{Luas bruto penampang}$$

$$= 1960000 \text{ mm}^2$$

$$V_u = 23969.729 \text{ kg}$$

$$= 239697.29 \text{ N}$$

Karena  $V_u = 239697.29 \text{ N} < 894613.5106 \text{ N}$   
maka rasio penulangan untuk dinding geser adalah :

$$\rho > \rho_{min} = 0.0025$$

Jika dalam perhitungan dicoba menggunakan  $\rho_{min} = 0.00250$

Sehingga luas penampang yang diperlukan :

$$A_s = \rho \times b \times d$$

$$= 0.00250 \times 35 \times 532.4$$

$$= 46.585 \text{ cm}^2$$

Dipasang tulangan untuk bagian tengah 46 D 13 = 61.081 cm<sup>2</sup>

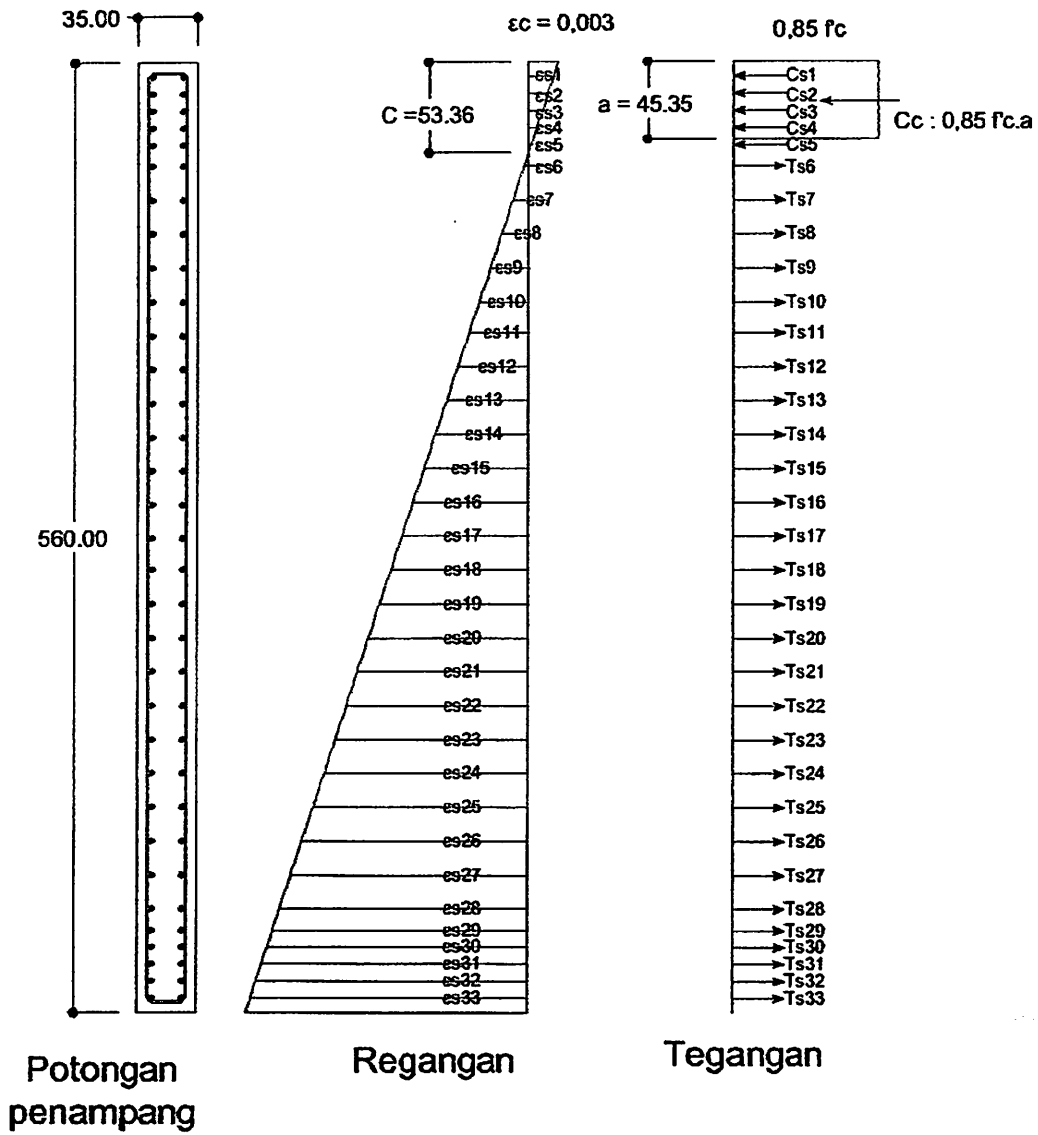
Cek  $\rho$  terpasang

$$\rho = \frac{A_s}{b \cdot d} = \frac{61.081}{35 \times 532.4} = 0.00328$$

$$\rho > \rho_{min2}$$

$$0.00328 > 0.00250 \dots\dots\dots \text{OK}$$

dicoba nilai sampai memenuhi  $C_s + C_c = T_s + P_n$  sehingga  
dari hasil trial and error, didapat nilai  $c = 533.569 \text{ mm}$



Gambar 4.1. Diagram tegangan dan regangan arah x



Tabel 4.1. Tabel jarak tulangan terhadap serat atas

di	jarak cm	di	jarak cm	di	jarak cm
d1	7.6	d12	180	d23	400
d2	17.6	d13	200	d24	420
d3	27.6	d14	220	d25	440
d4	37.6	d15	240	d26	460
d5	47.6	d16	260	d27	480
d6	60	d17	280	d28	500
d7	80	d18	300	d29	512.4
d8	100	d19	320	d30	522.4
d9	120	d20	340	d31	532.4
d10	140	d21	360	d32	542.4
d11	160	d22	380	d33	552.4

Jarak masing-masing tulangan terhadap tengah-tengah penampang

Tabel 4.2. Tabel Jarak tulangan tengah penampang

y	jarak cm	y	jarak cm	y	jarak cm
y1	272.4	y12	100	y23	120
y2	262.4	y13	80	y24	140
y3	252.4	y14	60	y25	160
y4	242.4	y15	40	y26	180
y5	232.4	y16	20	y27	200
y6	220	y17	0	y28	220
y7	200	y18	20	y29	232.4
y8	180	y19	40	y30	242.4
y9	160	y20	60	y31	252.4
y10	140	y21	80	y32	262.4
y11	120	y22	100	y33	272.4

Menghitung regangan yang terjadi :

Tabel 4.3. Tabel regangan

es	Nilai	es	Nilai	es	Nilai
es1	0.00257	es12	0.00712	es23	0.01949
es2	0.00201	es13	0.00825	es24	0.02061
es3	0.00145	es14	0.00937	es25	0.02174
es4	0.00089	es15	0.01049	es26	0.02286
es5	0.00032	es16	0.01162	es27	0.02399
es6	0.00037	es17	0.01274	es28	0.02511
es7	0.00150	es18	0.01387	es29	0.02581
es8	0.00262	es19	0.01499	es30	0.02637
es9	0.00375	es20	0.01612	es31	0.02693
es10	0.00487	es21	0.01724	es32	0.02750
es11	0.00600	es22	0.01837	es33	0.02806

Untuk daerah tekan :

$$\begin{aligned} \frac{\epsilon_{s1}}{\epsilon_s} &= \frac{c - d1}{c} \quad \Rightarrow \quad \epsilon_{s1} = \frac{c - d1}{c} \times \epsilon_c \quad ; \epsilon_c = 0,003 \\ &= \frac{53.3569 - 7.6}{53.3569} \times 0.003 \\ &= 0.002572689 \end{aligned}$$

Untuk daerah tarik :

$$\begin{aligned} \frac{\epsilon_{s8}}{\epsilon_s} &= \frac{d8 - c}{c} \quad \Rightarrow \quad \epsilon_{s8} = \frac{d8 - c}{c} \times \epsilon_c \quad ; \epsilon_c = 0,003 \\ &= \frac{100 - 53.3569}{53.3569} \times 0.003 \\ &= 0.002623 \end{aligned}$$

Mencari nilai fs :

Tabel 4.4. Tabel nilai fs

fs	Mpa	fs	Mpa	fs	Mpa
fs1	514.5377	fs12	1424.1067	fs23	3898.015
fs2	402.0873	fs13	1649.0075	fs24	4122.916
fs3	289.637	fs14	1873.9082	fs25	4347.816
fs4	177.1866	fs15	2098.809	fs26	4572.717
fs5	64.73622	fs16	2323.7097	fs27	4797.618
fs6	74.70225	fs17	2548.6105	fs28	5022.519
fs7	299.603	fs18	2773.5112	fs29	5161.957
fs8	524.5037	fs19	2998.412	fs30	5274.408
fs9	749.4045	fs20	3223.3127	fs31	5386.858
fs10	974.3052	fs21	3448.2135	fs32	5499.308
fs11	1199.206	fs22	3673.1142	fs33	5611.759

Keterangan tabel :

**Untuk daerah tekan**

$$fs = \epsilon_s \times E_s$$

$$fs1 = 0.00257 \times 200000 = 514.53772 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fy = 300 \text{ Mpa}$

$$fs7 = 0.00150 \times 200000 = 299.60299 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fy = 299.603 \text{ Mpa}$

**Untuk daerah tarik**

$$fs = \epsilon_s \times E_s$$

$$fs8 = 0.00262 \times 200000 = 524.50374 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fs = 300 \text{ Mpa}$

$$fs15 = 0.01049 \times 200000 = 2098.809 \text{ Mpa} > fy = 300 \text{ Mpa}$$

maka digunakan  $fy = 300 \text{ Mpa}$

Besarnya gaya-gaya yang bekerja :

Tabel 4.5. Tabel nilai Ts dan Cs

N	kN	N	kN	N	kN
Cs1	79.67143	Ts12	79.671429	Ts23	79.67143
Cs2	79.67143	Ts13	79.671429	Ts24	79.67143
Cs3	76.9193	Ts14	79.671429	Ts25	79.67143
Cs4	47.0557	Ts15	79.671429	Ts26	79.67143
Cs5	17.19209	Ts16	79.671429	Ts27	79.67143
Cs6	19.83878	Ts17	79.671429	Ts28	79.67143
Cc7	79.566	Ts18	79.671429	Ts29	79.67143
Ts8	79.67143	Ts19	79.671429	Ts30	79.67143
Ts9	79.67143	Ts20	79.671429	Ts31	79.67143
Ts10	79.67143	Ts21	79.671429	Ts32	79.67143
Ts11	79.67143	Ts22	79.671429	Ts33	79.67143

Keterangan tabel :

Untuk daerah tekan :

$$Cs = As \times fs$$

$$Cs1 = (2 \times 0,25 \times 22/7 \times 13) \times 300 = 120685.7143 \text{ N} \\ = 120.6857143 \text{ kN}$$

$$Cs7 = (2 \times 0,25 \times 22/7 \times 13) \times 299.6 = 120526.0044 \text{ N} \\ = 120.5260044 \text{ kN}$$

Untuk daerah tarik :

$$Ts = As \times fs$$

$$Ts8 = (2 \times 0,25 \times 22/7 \times 13) \times 524.5 = 211000.3627 \text{ N} \\ = 211.0003627 \text{ kN}$$

$$Ts15 = (2 \times 0,25 \times 22/7 \times 13) \times 300 = 120685.7143 \text{ N} \\ = 120.6857143 \text{ kN}$$

$$Cc = 0,85 \cdot fc \cdot \beta \cdot c \cdot b$$

$$= 0.85 \times 30 \times 0.85 \times 533.569 \times 350$$

$$= 4047785.55 \text{ N}$$

$$= 4047.78555 \text{ kN}$$

**Kontrol  $\sum H = 0$**

$$\begin{aligned}
 (Cs1 + Cs2 + \dots + Cs5) + Cc &= (Ts6 + Ts7 + \dots + Ts33) + Pn \\
 300.5099469 + 4047.78555 &= 2170.86192 + 2177.433385 \\
 4348.295 \text{ kN} &= 4348.295 \text{ kN}
 \end{aligned}$$

sehingga besarnya momen yang terjadi terhadap titik berat penampang :

Tabel 4.6. Tabel nilai Mn

Mn	kNm	N	kNm	N	kNm
Mn1	217.025	Mn12	79.671	Mn23	95.606
Mn2	209.058	Mn13	63.737	Mn24	111.540
Mn3	194.144	Mn14	47.803	Mn25	127.474
Mn4	114.063	Mn15	31.869	Mn26	143.409
Mn5	39.95	Mn16	15.934	Mn27	159.343
Mn6	43.65	Mn17	0.000	Mn28	175.277
Mn7	159.13	Mn18	15.934	Mn29	185.156
Mn8	143.409	Mn19	31.869	Mn30	193.124
Mn9	127.474	Mn20	47.803	Mn31	201.091
Mn10	111.540	Mn21	63.737	Mn32	209.058
Mn11	95.606	Mn22	79.671	Mn33	217.025

Keterangan tabel :

$$\begin{aligned}
 Mn1 &= Nd1 \times y1 \\
 &= 79.67143 \times 272.4 = 21702.497 \text{ kNcm} = 217.02 \text{ kNm}
 \end{aligned}$$

$$\begin{aligned}
 Mn2 &= Nd2 \times y1 \\
 &= 79.67143 \times 262.4 = 20905.783 \text{ kNcm} = 209.06 \text{ kNm}
 \end{aligned}$$

$$\begin{aligned}
 \text{Jika } c &= 533.5687 \text{ mm, maka } a = \beta.c \\
 &= 0.85 \times 533.5687 \\
 &= 453.5334 \text{ mm}
 \end{aligned}$$

$$\begin{aligned}
 yc &= h/2 - a/2 \\
 &= 5600 / 2 - 453.5334 / 2 \\
 &= 2573.233 \text{ mm}
 \end{aligned}$$

$$\begin{aligned}
 Mn &= (Cc \times yc) + (Mn1 + Mn2 + \dots + Mn33) \\
 &= (4047.78555 \times 2573.233) + 3751.18 \\
 &= 14167.0776 \text{ kNm} > Mn = 4236.615385 \text{ kNm}
 \end{aligned}$$

### Kontrol terhadap sumbu X

$$M_u = 17476.200 \text{ kgcm} = 1747620 \text{ Nmm}$$

$$P_u = 141533.17 \text{ kg} = 1415331.7 \text{ N}$$

Kuat Nominal Penampang :

untuk mengetahui nilai c dapat diselesaikan dengan menggunakan persamaan

Jika diketahui data sebagai berikut :

$$A_s' \text{ 33 D 16} = 6637.714286 \text{ mm}^2 \quad f_y = 300 \text{ Mpa}$$

$$A_s \text{ 33 D 16} = 6637.714286 \text{ mm}^2 \quad \beta = 0.85$$

$$d' = 76 \text{ mm} \quad P_u = 141533.17 \text{ kg}$$

$$b = 5600 \text{ mm} \quad = 1415331.7 \text{ N}$$

Maka  $C_c + C_s = T_s + P_u$

$$\text{Dimana : } C_c \text{ (Beton tertekan)} = 0,85 \cdot f_c \cdot a \cdot b \quad ; \quad a = \beta \cdot c$$

$$C_s \text{ (Baja tertekan)} = A_s' (f_s' - 0,85 \cdot f_c)$$

$$T_s \text{ (Baja tertarik)} = A_s \cdot F_y$$

Momen Nominal yang disumbangkan oleh beton :

$$M_{nd1} = C_c \times \left[ d - \frac{a}{2} \right]$$

$$M_{nd2} = C_s \cdot (d - d')$$

$$M_n = M_{nd1} + M_{nd2} > M_n = \frac{M_u}{\Phi}$$

untuk mendapatkan nilai c, maka :

$$f_s' = e_s' \cdot E_s = \frac{0,003 (c - d')}{c} \cdot E_s = \frac{600 (c - d')}{c} ; \quad E_s : 200000 \text{ Mpa}$$

Maka :

$$N_{d1} + N_{d2} = N_t + P_u$$

$$(0,85 \cdot f_c \cdot \beta \cdot c \cdot b) + \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f_c \cdot A_s' = A_s \cdot F_y + P_u$$

apabila persamaan tersebut dikalikan c, maka :

$$(0,85 \cdot f_c \cdot \beta \cdot c^2 \cdot b) + ((600 (c - d')) \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \cdot c) = (A_s \cdot f_y + P_u) \cdot c$$

Setelah dilakukan pengelompokan, maka didapatkan persamaan kuadrat :

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot c \cdot A_s' - 600 \cdot d' \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \cdot c) - (A_s \cdot f_y \cdot c) - P_u \cdot c = 0$$

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot A_s' - 0,85 \cdot f_c \cdot A_s' - A_s \cdot f_y - P_u) \cdot c - 600 \cdot d' \cdot A_s' = 0$$

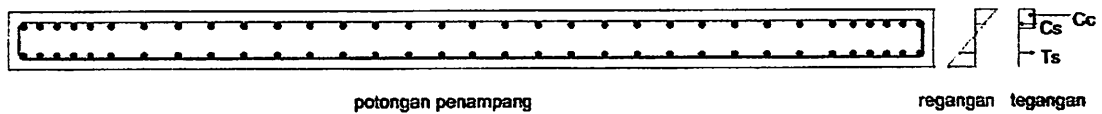
$$121380 \quad c^2 \quad -1584593.414 \quad c \quad - 302679771.4 = 0$$

dari persamaan di atas, di dapatkan nilai c = 56.8887 mm

$$a = \beta \times c = 0.85 \times 56.8887 = 48.3554 \text{ mm}$$

$$\epsilon_y = \frac{f_y}{E_s} = \frac{300}{200000} = 0.00150$$

$$\epsilon_s = 0,003 \cdot \frac{c - d'}{c} = 0.003 \cdot \frac{56.8887 - 76}{56.8887} = -0.001008$$



Gambar 4.2. Diagram tegangan dan regangan arah y

Karena  $\epsilon_s > \epsilon_y$ , maka dapat disimpulkan bahwa tulangan leleh meluluh, dengan demikian maka yang digunakan adalah  $f_y = 300 \text{ Mpa}$

Gaya-gaya yang timbul :

$$\begin{aligned} C_c &= 0,85 \cdot f_c \cdot a \cdot b \\ &= 0.85 \times 30 \times 48.355 \times 5600 \\ &= 6905152.401 \quad \text{N} \end{aligned}$$

$$\begin{aligned} C_s &= \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \\ &= -1337930.415 - 169261.7143 = -1507192.129 \text{ N} \end{aligned}$$

$$\begin{aligned} T_s &= A_s \cdot F_y \\ &= (66 \times 3.14 \times 8^2) \times 300 \\ &= 3982628.571 \quad \text{N} \end{aligned}$$

**Kontrol :**

$$\begin{aligned} Cc + Cs &= Ts + Pu \\ 6905152.40 + -1507192.13 &= 3982628.571 + 1415331.7 \\ 5397960.271 \text{ N} &= 5397960.271 \text{ N} \dots\dots \text{Ok} \end{aligned}$$

$$e = \frac{Mu}{Pu} = \frac{1747620}{1415331.7} = 1.234777685 \text{ mm}$$

sehingga momen nominal yang disumbangkan oleh beton dan baja adalah sebesar :

$$\begin{aligned} Mnd1 &= Cc \times \left( d - \frac{a}{2} \right) \\ &= 6905152.401 \times \left( 274 - \frac{48.3554}{2} \right) \\ &= 1725061024 \text{ Nmm} \end{aligned}$$

$$\begin{aligned} Mnd2 &= Cs \cdot (d - d') \\ &= -1507192.129 \times (274 - 76) \\ &= -298424041.6 \text{ Nmm} \end{aligned}$$

$$\begin{aligned} Mnd &= Mnd1 + Mnd2 \\ &= 1725061024 + -298424041.6 \\ &= 1426636982 \text{ Nmm} \end{aligned}$$

$$Mn = \frac{Mu}{\Phi} = \frac{1747620}{0.65} = 2688646 \text{ Nmm}$$

$$Mnd = 1426636982 \text{ Nmm} > Mn = 2688646 \text{ Nmm} \dots\dots \text{Ok}$$



**b. Penulangan Horizontal**

Berdasarkan SNI03-2847-2002 pasal 13.1

$$\Phi V_n \geq V_u$$

$V_u = 239697.29 \text{ kg}$  Dimana :

$\Phi = 0.6$   $V_c = V$  yang disumbangkan oleh beton

$V_n = V_c + V_s$   $V_s = V$  yang disumbangkan tulangan

Berdasarkan SNI03-2847-2002 pasal 13.3.1.(2)

$$V_c = \left[ 1 + \frac{V_u}{14.A_g} \right] \left[ \frac{\sqrt{f_c}}{6} \right] b_w . d$$

$$= \left[ 1 + \frac{239697.9}{14 \times 1960000} \right] \left[ \frac{\sqrt{30}}{6} \right] 350 \times 5524$$

$$= 1919118 \text{ N} = 191911.8 \text{ kg}$$

$$V_s = \frac{A_v . f_y . d}{s}$$

$$= \frac{132.7857 \times 300 \times 5524}{300}$$

$$= 733508.286 \text{ N} = 73350.83 \text{ kg}$$

$V_n = 191912 + 73350.82857 = 265263 \text{ kg}$

$V_n \geq V_u$

$265263 \text{ kg} \geq 239697 \text{ kg} \dots\dots\dots \text{Ok}$

Direncanakan tulangan D 13 - 300

$A_v = 1/4 \times 22/7 \times 13^2$

$= 132.785714 \text{ mm}^2 \geq 119.8143095 \text{ mm}^2 \dots\dots\dots \text{OK}$

Syarat :

$$A_v \geq \frac{75 \sqrt{f_c} \times b_w \times s}{1200 \times f_y}$$

$$\geq \frac{75 \times \sqrt{30} \times 350 \times 300}{1200 \times 300}$$

$$\geq 119.814309 \text{ mm}^2$$

### c. Panjang Penyaluran

Berdasarkan buku T. Paulay-M.J.N.Priestley hal 150, panjang sambungan lewatan ls sama dengan ld, sedangkan letak penyaluran dinyatakan dalam Ld.

$$L_d = m_{db} \times l_{db}$$

Dimana :

$$l_{db} = \frac{1.38 \times A_b \times f_y}{c \times \sqrt{f_c}}$$

$$m_{db} = \text{Faktor modifikasi} = 1.3$$

$$A_b = \text{Luas tulangan}$$

$$c = 3 \times \text{diameter tulangan}$$

Untuk tulangan D 13

$$A_b = 3.14 \times 7^2$$

$$= 132.7857143 \text{ mm}^2$$

$$c = 3 \times 13 = 39 \text{ mm}$$

$$l_{db} = \frac{1.38 \times 132.78571 \times 300}{39 \times \sqrt{5.477}}$$

$$= 257.3513559 \text{ mm}$$

Jadi untuk :

$$L_d = m_{db} \times l_{db}$$

$$= 1.3 \times 257.3514$$

$$= 334.5567627 \text{ mm}$$



## 5 Penulangan pada segmen 4 ( ada bukaan )

### a. Penulangan Vertikal

$$M_u = 21840.50 \text{ kgm} = 2184050 \text{ kgcm}$$

$$P_u = 67322.22 \text{ kg}$$

$$M_n = \frac{M_u}{\Phi} = \frac{2184050}{0.65} = 3360076.923 \text{ kgcm}$$

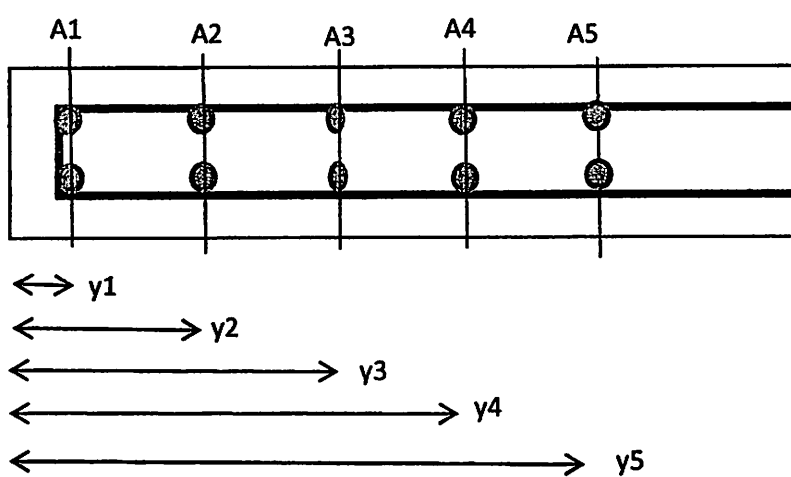
$$P_n = \frac{P_u}{\Phi} = \frac{67322.2}{0.65} = 103572.6462 \text{ kg}$$

$$l_w = 2.1 \text{ m}$$

Pendekatan pertama di misalkan  $d = 162.4 \text{ cm}$

$$A_v = \frac{M_n}{f_y \times d} = \frac{3360076.923}{3000 \times 162.4} = 6.89671 \text{ cm}^2$$

Dicoba tulangan 10 D 13  $A_s = 13.279 \text{ cm}^2$



$$y_1 = 7.6 \quad y_3 = 27.6 \quad y_5 = 47.6$$

$$y_2 = 17.6 \quad y_4 = 37.6$$

$$A = \left( \frac{1}{4} \times 3.14 \times 1.3^2 \right) \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A_1 \times y_1) + (A_2 \times y_2) + (A_3 \times y_3) + \dots + (A_5 \times y_5)}{A_1 + A_2 + A_3 + \dots + A_5}$$

$$= 27.6 \text{ cm}$$

$$d = 210 - 27.6 = 182.4 \text{ cm}$$

Untuk rasio penulangan pada dinding geser berpedoman pada SNI03-2847-2002 pasal 23.6.2.(1)

$$V_u < \frac{1}{12} \times A_{cv} \times \sqrt{f_c}$$

$$< 894613.511 \text{ N}$$

Dimana :

$$A_{cv} = \text{Luas bruto penampang}$$

$$= 1960000 \text{ mm}^2$$

$$V_u = 19788.126 \text{ kg}$$

$$= 197881.26 \text{ N}$$

Karena  $V_u = 197881.26 \text{ N} < 894613.5106 \text{ N}$   
maka rasio penulangan untuk dinding geser adalah :

$$\rho > \rho_{min} = 0.0025$$

Jika dalam perhitungan dicoba menggunakan  $\rho_{min} = 0.00250$

Sehingga luas penampang yang diperlukan :

$$A_s = \rho \times b \times d$$

$$= 0.0025 \times 35 \times 182.4$$

$$= 15.96 \text{ cm}^2$$

Dipasang tulangan untuk bagian tengah 16 D 13 = 21.246 cm<sup>2</sup>

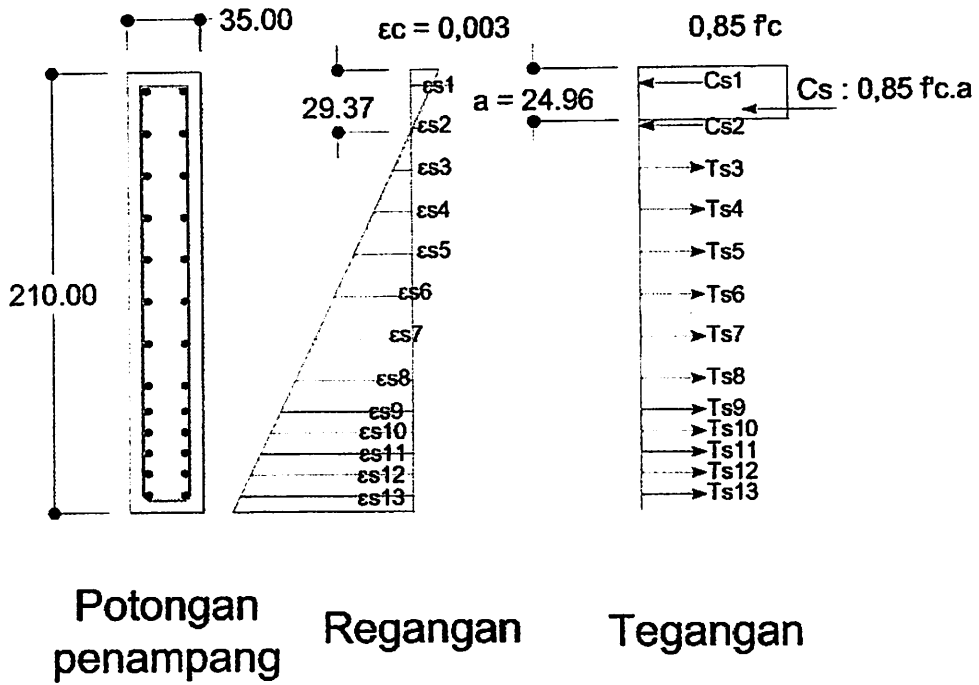
Cek  $\rho$  terpasang

$$\rho = \frac{A_s}{b.d} = \frac{21.246}{35 \times 182.4} = 0.00333$$

$$\rho > \rho_{min}$$

$$0.00333 > 0.00250 \text{ ..... OK}$$

dicoba nilai sampai memenuhi  $C_s + C_c = T_s + P_n$  sehingga  
dari hasil trial and error, didapat nilai  $c = 293.695 \text{ mm}$



Gambar 4.1. Diagram tegangan dan regangan

Tabel 4.1. Tabel jarak tulangan terhadap serat atas penampang

di	jarak cm
d1	7.6
d2	27.6
d3	47.6
d4	67.6
d5	87.6

di	jarak cm
d6	107.6
d7	127.6
d8	147.6
d9	162.4
d10	172.4

di	jarak cm
d11	182.4
d12	192.4
d13	202.4

Menghitung regangan yang terjadi :

Tabel 4.3. Tabel regangan pada

es	Nilai
es1	0.00222
es2	0.00018
es3	0.00186
es4	0.00391
es5	0.00595

es	Nilai
es6	0.00799
es7	0.01003
es8	0.01208
es9	0.01359
es10	0.01461

es	Nilai
es11	0.01563
es12	0.01665
es13	0.01767

Untuk daerah tekan :

$$\frac{\epsilon_{s1}}{\epsilon_s} = \frac{c - d1}{c} \qquad \epsilon_{s1} = \frac{c - d1}{c} \times \epsilon_c \qquad ; \epsilon_c = 0,003$$

$$= \frac{29.3695 - 7.6}{29.3695} \times 0.003$$

$$= 0.002223683$$

Untuk daerah tarik :

$$\frac{\epsilon_{s3}}{\epsilon_s} = \frac{d3 - c}{c} \qquad \epsilon_{s3} = \frac{d3 - c}{c} \times \epsilon_c \qquad ; \epsilon_c = 0,003$$

$$= \frac{47.6 - 29.3695}{29.3695} \times 0.003$$

$$= 0.00186$$

Mencari nilai fs :

Tabel 4.4. Tabel nilai fs

fs	Mpa	fs	Mpa	fs	Mpa
fs1	444.7367	fs6	1598.2018	fs11	3126.32
fs2	36.14898	fs7	2006.7895	fs12	3330.614
fs3	372.4387	fs8	2415.3772	fs13	3534.907
fs4	781.0264	fs9	2717.7321		
fs5	1189.614	fs10	2922.0259		

Keterangan tabel :

**Untuk daerah tekan**

$$fs = \epsilon_s \times E_s$$

$$fs1 = 0.00222 \times 200000 = 444.73668 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $f_y = 300 \text{ Mpa}$

**Untuk daerah tarik**

$$fs = \epsilon_s \times E_s$$

$$fs3 = 0.00186 \times 200000 = 372.43872 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $fs = 300 \text{ Mpa}$

$$fs4 = 0.00391 \times 200000 = 781.02641 \text{ Mpa} > f_y = 300 \text{ Mpa}$$

maka digunakan  $f_y = 300 \text{ Mpa}$

Besarnya gaya-gaya yang bekerja :

Tabel 4.4. Tabel nilai fs

N	kN	N	kN	N	kN
Cs1	120.6857	Cs6	120.68571	Ts11	120.6857
Cs2	14.54222	Ts7	120.68571	Ts12	120.6857
Cs3	120.6857	Ts8	120.68571	Ts13	120.6857
Cs4	120.6857	Ts9	120.68571		
Cs5	120.6857	Ts10	120.68571		

Keterangan tabel :

Untuk daerah tekan :

$$C_s = A_s' \times f_s'$$

$$C_{s1} = (2 \times 0,25 \times 22/7 \times 13) \times 300 = 120685.7143 \text{ N} \\ = 120.6857143 \text{ kN}$$

$$C_{s2} = (2 \times 0,25 \times 22/7 \times 13) \times 36.15 = 14542.21794 \text{ N} \\ = 14.54221794 \text{ kN}$$

Untuk daerah tarik :

$$T_s = A_s \times f_s$$

$$T_{s3} = (2 \times 0,25 \times 22/7 \times 13) \times 372 = 149826.7753 \text{ N} \\ = 149.8267753 \text{ kN}$$

$$T_{s4} = (2 \times 0,25 \times 22/7 \times 13) \times 300 = 120685.7143 \text{ N} \\ = 120.6857143 \text{ kN}$$

$$C_c = 0,85 \cdot f_c \cdot \beta_c \cdot b \\ = 0,85 \times 30 \times 0,85 \times 293.695 \times 350 \\ = 2228040.66 \text{ N} \\ = 2228.04066 \text{ kN}$$

**Kontrol  $\sum H = 0$**

$$(C_{s1} + C_{s2}) + C_c = (T_{s3} + T_{s4} + \dots + T_{s13}) + P_n \\ 135.2279322 + 2228.040659 = 1327.542857 + 1035.726462 \\ 2363.269 \text{ kN} = 2363.269 \text{ kN}$$

Mencari titik tengah penampang tulangan

$$A = \frac{1}{4} \times 22/7 \times 1.3^2 \times 2$$

$$= 2.65571429 \text{ cm}^2$$

$$y = \frac{(A_1 \times d_1) + (A_2 \times d_2) + (A_3 \times d_3) + \dots + (A_{13} \times d_{13})}{A_1 + A_2 + A_3 + \dots + A_{13}}$$

$$= \frac{4070.678857}{34.52428571}$$

$$= 117.9076923 \text{ cm}$$

Tabel 4.1. Tabel jarak tulangan terhadap tengah penampang

y	jarak cm	y	jarak cm	y	jarak cm
y1	110.3077	y6	10.307692	y11	64.49231
y2	90.30769	y7	9.6923077	y12	74.49231
y3	70.30769	y8	29.692308	y13	84.49231
y4	50.30769	y9	44.492308		
y5	30.30769	y10	54.492308		

sehingga besarnya momen yang terjadi terhadap titik berat penampang :

Tabel 4.4. Tabel nilai fs

Mn	kNm	Mn	kNm	Mn	kNm
Mn1	133.126	Mn6	12.440	Mn11	77.833
Mn2	13.133	Mn7	11.697	Mn12	89.902
Mn3	84.851	Mn8	35.834	Mn13	101.970
Mn4	60.714	Mn9	53.696		
Mn5	36.577	Mn10	65.764		

Keterangan tabel :

$$Mn1 = Cc1 \times y1$$

$$= 120.6857 \times 110.31 = 13312.563 \text{ kNcm} = 133.13 \text{ kNm}$$

$$Mn2 = Cc2 \times y2$$

$$= 14.54222 \times 90.308 = 1313.2741 \text{ kNcm} = 13.13 \text{ kNm}$$



$$\begin{aligned} \text{Jika } c &= 293.6946 \text{ mm, maka } a = \beta \cdot c \\ &= 0.85 \times 293.6946 \\ &= 249.64041 \text{ mm} \end{aligned}$$

$$\begin{aligned} y_c &= y - a/2 \\ &= 1179.077 - 249.64041 / 2 \\ &= 1054.257 \text{ mm} \end{aligned}$$

$$\begin{aligned} M_n &= (C_c \times y_c) + (M_{n1} + M_{n2} + \dots + M_{n13}) \\ &= (2228.04066 \times 1.0542567) + 777.54 \\ &= 3126.464 \text{ kNm} > M_n = 336.01 \text{ kNm} \dots \text{Ok} \end{aligned}$$

### Kontrol terhadap sumbu X

$$M_u = 16559.400 \text{ kgcm} = 1655940 \text{ Nmm}$$

$$P_u = 67322.22 \text{ kg} = 673222.2 \text{ N}$$

Kuat Nominal Penampang :

untuk mengetahui nilai c dapat diselesaikan dengan menggunakan persamaan

Jika diketahui data sebagai berikut :

$$\begin{array}{ll} A_s' \text{ 13 D 16} &= 2614.857143 \text{ mm}^2 & f_y &= 300 \text{ Mpa} \\ & & f_c &= 30 \text{ Mpa} \\ A_s \text{ 13 D 16} &= 2614.857143 \text{ mm}^2 & \beta &= 0.85 \\ d' &= 76 \text{ mm} & P_u &= 67322.22 \text{ kg} \\ b &= 2100 \text{ mm} & &= 673222.2 \text{ N} \end{array}$$

$$\text{Maka } C_c + C_s = T_s + P_u$$

$$\text{Dimana : } C_c \text{ (Beton tertekan)} = 0,85 \cdot f_c \cdot a \cdot b \quad ; \quad a = \beta \cdot c$$

$$C_s \text{ (Baja tertekan)} = A_s' (f_{s1} - 0,85 \cdot f_c)$$

$$T_s \text{ (Baja tertarik)} = A_s1 \cdot F_{y1}$$

Momen Nominal yang disumbangkan oleh beton :

$$M_{nd1} = C_c \times \left[ d - \frac{a}{2} \right]$$

$$M_{nd2} = N_{d2} \cdot (d - d')$$

$$M_n = M_{nd1} + M_{nd2} > M_n = \frac{M_u}{\Phi}$$

untuk mendapatkan nilai c, maka :

$$f_s' = \epsilon_s' \cdot E_s = \frac{0,003 (c - d')}{c} \cdot E_s = \frac{600 (c - d')}{c} ; \quad E_s : 200000 \text{ Mpa}$$

Maka :

$$N_{d1} + N_{d2} = N_t + P_u$$

$$(0,85 \cdot f_c \cdot \beta \cdot c \cdot b) + \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f_c \cdot A_s' = A_s \cdot f_y + P_u$$

apabila persamaan tersebut dikalikan c, maka :

$$(0,85 \cdot f_c \cdot \beta \cdot c^2 \cdot b) + ((600 (c - d')) \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \cdot c) = (A_s \cdot f_y + P_u) c$$

Setelah dilakukan pengelompokan, maka didapatkan persamaan kuadrat :

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot c \cdot A_s' - 600 \cdot d' \cdot A_s' - 0,85 \cdot f_c \cdot A_s' \cdot c) - (A_s \cdot f_y \cdot c) - P_u \cdot c = 0$$

$$(0,85 \cdot f_c \cdot \beta \cdot b \cdot c^2) + (600 \cdot A_s' - 0,85 \cdot f_c \cdot A_s' - A_s \cdot f_y - P_u) \cdot c - 600 \cdot d' \cdot A_s' = 0$$

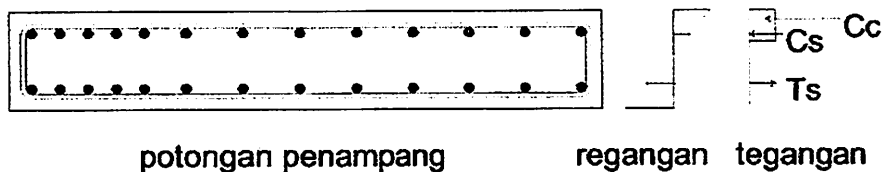
$$45518 \quad c^2 \quad -739901,057 \quad c \quad - 119237485,7 = 0$$

dari persamaan di atas, di dapatkan nilai c = 59.9510 mm

$$a = \beta \times c = 0,85 \times 59,9510 = 50,9583 \text{ mm}$$

$$\epsilon_y = \frac{f_y}{E_s} = \frac{300}{200000} = 0,00150$$

$$\epsilon_s = 0,003 \cdot \frac{c - d'}{c} = 0,003 \cdot \frac{60 - 76}{59,9510} = -0,000803$$



Gambar 4.2. Diagram tegangan dan regangan arah y

Karena  $\epsilon_s > \epsilon_y$ , maka dapat disimpulkan bahwa tulangan leleh meluluh, dengan demikian maka yang digunakan adalah  $f_y = 300 \text{ Mpa}$

Gaya-gaya yang timbul :

$$\begin{aligned} C_c &= 0,85 \cdot f_c \cdot a \cdot b \\ &= 0,85 \times 30 \times 50,958 \times 2100 \\ &= 2728817,979 \quad \text{N} \end{aligned}$$

$$C_s = \frac{600 (c - d')}{c} \cdot A_s' - 0,85 \cdot f'c \cdot A_s'$$

$$= -420002.6363 - 66678.85714 = -486681.4935 \text{ N}$$

$$T_s = A_s \cdot F_y$$

$$= (2614.857 + 2614.857) \times 300$$

$$= 1568914.286 \text{ N}$$

$$N_d = C_c + C_s$$

**Kontrol :**

$$N_d = N_t + P_u$$

$$2242136.486 = 1568914.286 + 673222.2$$

$$2242136.486 \text{ N} = 2242136.486 \text{ N} \dots\dots \text{Ok}$$

$$e = \frac{M_u}{P_u} = \frac{1655940}{673222.2} = 2.459722808 \text{ mm}$$

sehingga momen nominal yang disumbangkan oleh beton dan baja adalah sebesar :

$$M_{nd1} = C_c \times \left( d - \frac{a}{2} \right)$$

$$= 2242136.486 \times \left( 274 - \frac{50.9583}{2} \right)$$

$$= 557217644 \text{ Nmm}$$

$$M_{nd2} = C_s \cdot (d - d')$$

$$= -486681.4935 \times (274 - 76)$$

$$= -96362935.71 \text{ Nmm}$$

$$M_{nd} = M_{nd1} + M_{nd2}$$

$$= 557217644 + -96362935.71$$

$$= 460854708.3 \text{ Nmm}$$

$$M_n = \frac{M_u}{\Phi} = \frac{1655940}{0.7} = 2547600 \text{ Nmm}$$

$$M_{nd} = 460854708 \text{ Nmm} > M_n = 2547600 \text{ Nmm} \dots\dots \text{Ok}$$

**b. Penulangan Horizontal**

Berdasarkan SNI03-2847-2002 pasal 13.1

$$\Phi V_n \geq V_u$$

$V_u = 197881.26 \text{ kg}$       Dimana :

$\Phi = 0.6$        $V_c = V$  yang disumbangkan oleh beton

$V_n = V_c + V_s$        $V_s = V$  yang disumbangkan tulangan

Berdasarkan SNI03-2847-2002 pasal 13.3.1.(2)

$$V_c = 1 + \frac{V_u}{14 \cdot A_g} \cdot \frac{f_c}{6} \cdot b_w \cdot d$$

$$= 1 + \frac{197881.26}{14 \times 735000} \cdot \frac{30}{6} \cdot 350 \times 2024$$

$$= 771037 \text{ N} = 77103.68 \text{ kg}$$

$$V_s = \frac{A_v \cdot f_y \cdot d}{s}$$

$$= \frac{380.2857 \times 300 \times 2024}{150}$$

$$= 1539396.57 \text{ N} = 153939.7 \text{ kg}$$

$$V_n = 77104 + 153939.6571 = 231043 \text{ kg}$$

$$V_n \geq V_u$$

$$231043 \text{ kg} \geq 197881 \text{ kg} \dots\dots\dots \text{Ok}$$

Direncanakan tulangan D      22      -      150

$$A_v = 1/4 \times 22/7 \times 22^2$$

$$= 380.286 \text{ mm}^2 \geq 59.907 \text{ mm}^2 \dots\dots\dots \text{OK}$$

Syarat :

$$A_v \geq \frac{75 \cdot f_c \cdot b_w \cdot x}{1200 \cdot f_y}$$

$$\geq \frac{75 \times 30 \times 350 \times 150}{1200 \times 300}$$

$$\geq 59.9071547 \text{ mm}^2$$

### c. Panjang Penyaluran

Berdasarkan buku T. Paulay-M.J.N.Priestley hal 150, panjang sambungan lewatan ls sama dengan ld, sedangkan letak penyaluran dinyatakan dalam Ld.

$$Ld = mdb \times ldb$$

Dimana :

$$ldb = \frac{1.38 \times Ab \times fy}{c \times fc}$$

$$mdb = \text{Faktor modifikasi} = 1.3$$

$$Ab = \text{Luas tulangan}$$

$$c = 3 \times \text{diameter tulangan}$$

Untuk tulangan D 16

$$\begin{aligned} Ab &= 3.14 \times 8^2 \\ &= 201.1428571 \text{ mm}^2 \end{aligned}$$

$$c = 3 \times 16 = 48 \text{ mm}$$

$$\begin{aligned} ldb &= \frac{1.38 \times 201.14286 \times 300}{48 \times 30} \\ &= 316.7401304 \text{ mm} \end{aligned}$$

Jadi untuk :

$$\begin{aligned} Ld &= mdb \times ldb \\ &= 1.3 \times 316.7401 \\ &= 411.7621695 \text{ mm} \end{aligned}$$





**LAMPIRAN  
STAAD PRO**

STAAD SPACE HOTEL 15 LANTAI

START JOB INFORMATION

ENGINEER DATE 20-Jul-14

END JOB INFORMATION

INPUT WIDTH 79

UNIT METER KG

JOINT COORDINATES

1 0 0 0; 2 5.25 0 0; 3 10.5 0 0; 4 18.3 0 0; 5 22.6 0 0; 6 29.75 0 0;  
7 36.75 0 0; 8 43.75 0 0; 9 50.75 0 0; 10 57.75 0 0; 11 0 5 0; 12 5.25 5 0;  
13 10.5 5 0; 14 18.3 5 0; 15 22.6 5 0; 16 29.75 5 0; 17 36.75 5 0;  
18 43.75 5 0; 19 50.75 5 0; 20 57.75 5 0; 21 0 9 0; 22 5.25 9 0; 23 10.5 9 0;  
24 18.3 9 0; 25 22.6 9 0; 26 29.75 9 0; 27 36.75 9 0; 28 43.75 9 0;  
29 50.75 9 0; 30 57.75 9 0; 31 0 13 0; 32 5.25 13 0; 33 10.5 13 0;  
34 18.3 13 0; 35 22.6 13 0; 36 29.75 13 0; 37 36.75 13 0; 38 43.75 13 0;  
39 50.75 13 0; 40 57.75 13 0; 41 0 17 0; 42 5.25 17 0; 43 10.5 17 0;  
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ELEMENT PROPERTY

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 1668 TO 1670 1672 TO 1674 1676 TO 1678 1680 TO 1682 1684 1686 1688 1689 1691 -  
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 1972 TO 1974 1976 1977 1979 1980 1982 1983 1985 1986 1988 1989 1991 1992 -  
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 2018 TO 2019 2021 TO 2023 2025 2027 2029 2030 2032 2034 THICKNESS 0.1  
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 2062 TO 2063 2065 TO 2067 2069 TO 2071 2073 2075 2077 TO 2080 2082 TO 2084 -

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 11554 TO 11558 11560 TO 11568 11570 TO 11576 11578 11580 TO 11584 -  
 11586 TO 11594 11596 TO 11602 11604 11606 TO 11610 11612 THICKNESS 0.35  
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 DENSITY 2400  
 ALPHA 1e-005  
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CONSTANTS

MATERIAL CONCRETE MEMB 1 TO 1140 1142 TO 1149 1152 TO 1159 1162 TO 1169 1172 -  
 1173 TO 1179 1182 TO 1189 1192 TO 1199 1202 TO 1209 1212 TO 1219 1222 TO 1229 -  
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7957 TO 7963 7982 TO 7988 FIXED  
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 5052 5054 5056 5058 5060 5062 5064 5066 5068 5070 5072 TO 5074 5076 5077 -

5079 5080 5082 5083 5085 5086 5088 5089 5091 5092 5094 5096 TO 5099 5101 -  
5102 TO 5103 5105 TO 5107 5109 TO 5111 5113 TO 5115 5117 TO 5119 5121 TO 5123 -  
5125 5127 5129 5130 5132 5134 5136 5138 5140 5142 5144 TO 5147 5149 TO 5151 -  
5153 TO 5155 5157 TO 5159 5161 TO 5163 5165 TO 5167 5169 TO 5171 5173 5175 -  
5177 TO 5180 5182 TO 5184 5186 TO 5188 5190 TO 5192 5194 TO 5196 -  
5198 TO 5200 5202 TO 5204 5206 5208 5210 TO 5213 5215 TO 5217 5219 TO 5221 -  
5223 TO 5225 5227 TO 5229 5231 TO 5233 5235 TO 5237 5239 5241 PR 250  
5243 TO 5246 5248 TO 5250 5252 TO 5254 5256 TO 5258 5260 TO 5262 5264 TO 5266 -  
5268 TO 5270 5272 5274 5276 TO 5279 5281 TO 5283 5285 TO 5287 5289 TO 5291 -  
5293 TO 5295 5297 TO 5299 5301 TO 5303 5305 5307 5309 5310 5517 TO 5646 5648 -  
5649 5651 TO 5930 6137 TO 6266 6268 6269 6271 TO 6550 6757 TO 6886 6888 6889 -  
6891 TO 7170 7377 TO 7506 7508 7509 7511 TO 7790 7997 TO 8126 8128 8129 8131 -  
8132 TO 8410 8617 TO 8746 8748 8749 8751 TO 9030 9237 TO 9366 9368 9369 9371 -  
9372 TO 9650 9857 TO 9986 9988 9989 9991 TO 10270 10477 TO 10606 10608 10609 -  
10611 TO 10890 PR 250

LOAD 3 BEBAN GEMPA

JOINT LOAD

7989 FZ 150986  
7989 FX 45295.8  
8046 FX 70137.7 FZ 233792  
8051 FX 65215.7 FZ 217386  
8056 FX 60293.8 FZ 200979  
8061 FX 55371.8 FZ 184573  
8066 FX 50449.9 FZ 168166  
8071 FX 45528 FZ 151760  
8076 FX 40606 FZ 135353  
8081 FX 35684.1 FZ 118947  
8086 FX 30762.1 FZ 102540  
8091 FX 25840.2 FZ 86134  
8096 FX 20918.3 FZ 69727.5  
8101 FX 15996.3 FZ 53321.1  
8106 FX 11074.4 FZ 36914.6  
8111 FX 6496.99 FZ 21656.6

LOAD COMB 4 KOMBINASI 1

1 1.4

LOAD COMB 5 KOMBINASI 2

1 1.2 2 1.6

LOAD COMB 6 KOMBINASI 3

1 1.2 2 1.0 3 1.0

LOAD COMB 7 KOMBINASI 4

1 1.2 2 1.0 3 -1.0

LOAD COMB 8 KOMBINASI 5

1 0.9 3 1.0

LOAD COMB 9 KOMBINASI 6

1 0.9 3 -1.0

PERFORM ANALYSIS

FINISH





**LAMPIRAN  
GAMBAR PENULANGAN**



INSTITUT TEKNOLOGI NASIONAL  
Jl. Bendungan Sigura-gura 2  
Jl. Raya Karanglo Km. 2  
Malang

# UJIAN SKRIPSI

## PRODI TEKNIK SIPIL S-1

### FORM REVISI / PERBAIKAN BIDANG \_\_\_\_\_

Nama : \_\_\_\_\_  
NIM : \_\_\_\_\_  
Hari / tanggal : \_\_\_\_\_ / \_\_\_\_\_

Perbaiki materi Skripsi meliputi :

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Perbaikan Skripsi harus diselesaikan selambatnya 14 hari terhitung sejak pelaksanaan Ujian dilaksanakan. **Bila melebihi** masa 14 hari, maka **tidak dapat diikuti Yudisium.**

Tugas Akhir telah diperbaiki dan disetujui :

Malang, \_\_\_\_\_ 20\_\_\_\_  
Dosen Penguji  
  
( \_\_\_\_\_ )

Malang, \_\_\_\_\_ 20\_\_\_\_  
Dosen Penguji  
  
( \_\_\_\_\_ )



INSTITUT TEKNOLOGI NASIONAL  
 Jl. Bendungan Sigura-gura 2  
 Jl. Raya Kamuglo Km. 2  
 Malang

# UJIAN SKRIPSI PRODI TEKNIK SIPIL S-1

## FORM REVISI / PERBAIKAN BIDANG STRUKTUR.

Nama : AGUS FAISAL

NIM : 6021053

Hari / tanggal : Selasa, 19-8-2014

Perbaikan materi Skripsi meliputi :

Revisi gbr diagram teg & reg

Perbaikan Skripsi harus diselesaikan selambatnya 14 hari terhitung sejak pelaksanaan Ujian dilaksanakan. Bila melebihi masa 14 hari, maka tidak dapat diikuti Yudisium.

**Tugas Akhir telah diperbaiki dan disetujui :**

Malang, \_\_\_\_\_ 2014  
 Dosen Penguji

Malang, 19-08-2014  
 Dosen Penguji

( \_\_\_\_\_ )

( A. Agus Santosa )



**FORM REVISI / PERBAIKAN**  
**BIDANG STRUKTUR**

Nama : AGUS FAISAL

NIM : W 21 053

Hari / tanggal : Jum'at, 8 - 8 - 2014

Perbaiki materi Seminar Hasil Tugas Akhir meliputi :

- Momen untuk kontrol keparitas dindisy gran  
 mph. jumlah momen pd tumpuan kolom.

Perbaiki Seminar Hasil Skripsi harus diselesaikan selambatnya 14 hari terhitung sejak pelaksanaan Seminar. Bila melebihi 14 hari, maka tidak dapat diikuti Ujian Skripsi.

*Pengumpulan berkas untuk ujian skripsi dengan menyertakan lembar pengesahan dari dosen pembahas dan kaprodi*

Skripsi telah diperbaiki dan disetujui :

Malang, 13 - 8 - 14, 20  
 Dosen Pembahas

([Signature])

Malang, 8 - 8 - 2014  
 Dosen Pembahas

(A. Agus Santosa)



**FORM REVISI / PERBAIKAN  
BIDANG \_\_\_\_\_**

Nama : \_\_\_\_\_

NIM : \_\_\_\_\_

Hari / tanggal : \_\_\_\_\_

Perbaikan materi Seminar Hasil Tugas Akhir meliputi :

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Perbaikan Seminar Hasil Skripsi harus diselesaikan **selambatnya 14 hari** terhitung sejak pelaksanaan Seminar. **Bila melebihi 14 hari, maka tidak dapat diikuti Ujian Skripsi.**


*Pengumpulan berkas untuk ujian skripsi dengan menyertakan lembar pengesahan dari dosen pembahas dan kaprodi*

**Skripsi telah diperbaiki dan disetujui :**

Malang, \_\_\_\_\_ 20  
Dosen Pembahas

(  )

Malang, \_\_\_\_\_ 20  
Dosen Pembahas

(  )





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PROGRAM STUDI TEKNIK SIPIL S-1  
Jl. Bendungan Sigura-gura No.2 Telp. (0341) 551431 Malang

LEMBAR ASISTENSI

PROPOSAL SKRIPSI

Nama : Agus Faisal  
Nim : 10.21.053  
Program Studi : Teknik Sipil S-1  
Dosen Pembimbing : Ir. Ester Priskasari, MT.

No	Tanggal	Keterangan	Tanda Tangan
		Betulan gambar dan tata bahasa statikanya dibetulkan bayangkan pembagian penulangan Shear wall dan tul tranversal acc. Anaya ujian	



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Jl. Bendungan Sigura-gura No.2 Telp. (0341) 551431 Malang

LEMBAR ASISTENSI

PROPOSAL SKRIPSI

Nama : Agus Faisal  
Nim : 10.21.053  
Program Studi : Teknik Sipil S-1  
Dosen Pembimbing : Ir. Bambang Wedyantadji, MT.

No	Tanggal	Keterangan	Tanda Tangan
1	22-5-2014	Tentunya kan dulu apa yg akan di bahas, dan Rumusan Masalah, teori sesuai dgn apa yg akan di bahas	
2	4-6-2014	Bahasan masalah sesuaikan - buku literatur dibawa - teori sesuai dgn masalah	
3	4-7-2014	Tetapkan dulu model Bunday geser	
4	11-7-2014	Sesuaikan dan menentukan dimensi	
5	22-7-2014	perhatikan aturan nya Rehit perbedaan Salah 2 ada yg janggal	
6	23-7-2014	ele dan cari perhatian thg babasan 2 mendiskusikan dan penulangan	